

Report No A1077-11

SANDESEND SLOPE STABILISATION

WHITBY, NORTH YORKSHIRE

FACTUAL REPORT ON GROUND INVESTIGATION

Carried out for:
Balfour Beatty Living Places

Engineer:
Haskoning Ltd

December 2011

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Soil Mechanics part of Environmental Scientifics Group

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Employer:

**Balfour Beatty Living Places
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Engineer:

**Haskoning Ltd
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NE1 4EE**

Issue No	Date	Details
1	December 2011	Report as submitted

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1 INTRODUCTION

In September 2011 Soil Mechanics was commissioned by Balfour Beatty Living Places to carry out a ground investigation at Sandsend, near Whitby in North Yorkshire, under the direction of their technical advisors, Haskoning Ltd. The investigation was required to obtain geotechnical and geoenvironmental information for proposed stabilisation measures in an area of cliff slope instability on the landward side of the A174 Sandsend Road.

The scope of the investigation, which was specified by Haskoning comprised cable percussion and rotary drilled boreholes, dynamic sampling and laboratory testing. The investigation was carried out in accordance with the contract specification, Eurocode 7 and relevant related standards identified below (see also References). The fieldwork was carried out between 3 and 10 October 2011.

This report presents the factual records of the fieldwork and laboratory testing. The data is also presented separately in digital format following AGS (2005).

2 THE SITE AND GEOLOGY

2.1 The Site

The site comprises an approximately 1 km length of coastline between Sandsend and Whitby, see Site Location Plan in Enclosure F. The area of slope instability is about 200 m long and centred at National Grid reference NZ 867 123.

2.2 Published Geology

The published geological map covering the site, combined BGS Sheets 35 and 44 (1998), show Glacial Till overlying the Whitby Mudstone Formation of the Lias Group.

3 FIELDWORK

3.1 General

The fieldwork was carried out in general accordance with BS 5930+A2 (2010), BS EN 1997-2 (2007) and BS EN ISO 22475-1 (2006).

The exploratory hole locations were selected by Haskoning and were set out by Soil Mechanics and Haskoning from local features. The co-ordinates and reduced levels were surveyed by Soil Mechanics to National Grid and Ordnance Datum. The exploratory hole locations are shown on the Site Plan in Enclosure F.

3.2 Exploratory Holes

The exploratory holes are listed in the following table.

SUMMARY OF EXPLORATORY HOLES

TYPE	QUANTITY	MAXIMUM DEPTH (m)	REMARKS
Cable Percussion Boring extended by Rotary Core Drilling	6	40.7	BH1 to 3 carried out above the slip area, and BH4 to 6 at road level (assumed slip toe).
Dynamic Sampling	5	4.2	WS1 to 5 carried out at beach level spaced along approximately 900 m of coastline.

The exploratory hole records are presented in Enclosure A and should be read in conjunction with the Key which is included in that enclosure. The records provide descriptions of the materials encountered in accordance with BS EN ISO 14688-1 (2002) and 14689-1 (2003), for soils and rocks respectively, as amplified by BS 5930+A2 (2010). The records also give details of the samples taken together with observations made during boring, drilling and dynamic sampling. Standard penetration tests (SPT) were carried out in accordance with BS EN ISO 22476-3 (2005) and the results are included on the borehole records. The SPT hammer energy ratio calibration certificates are also included in Enclosure A.

Photographs of the cores recovered from the rotary drilling are presented in Enclosure E.

On completion of the fieldwork geotechnical samples were transported to the Doncaster laboratory of Soil Mechanics for temporary retention and testing. Geoenvironmental samples were transported from site directly to the ESG Scientifics laboratory.

3.3 Instrumentation

The instruments installed in the exploratory holes are shown on the logs and detailed in Enclosure B. Soil Mechanics was not required to carry out any monitoring of these.

4 LABORATORY TESTING

4.1 Geotechnical Testing

The testing was scheduled by Haskoning and was carried out in accordance with BS 1377 (1990) unless otherwise stated. The testing is summarised below and the results are presented in Enclosure C.

SUMMARY OF GEOTECHNICAL LABORATORY TESTING

TYPE	REMARKS
Moisture Content Determination	
Atterberg Limit Determination	
Particle Size Distribution Analysis	
pH and Water Soluble Sulphate Content of Soils	
Unconsolidated Undrained Triaxial Compression	
Consolidated Undrained Triaxial Compression	
Shear Strength by Direct Shear (Shearbox)	
Residual Shear Strength (Ring Shear)	
Laboratory Vane Shear Strength	
One Dimensional Oedometer Consolidation	
Dry Density / Moisture Content Relationship	
Moisture Condition Value (MCV) / Moisture Content Relationship	

4.2 Geoenvironmental Testing

The testing was scheduled by Haskoning and was carried out by ESG Scientifics. The results are presented in Enclosure D.

Prepared By	P Hepton BSc PhD
Reviewed By	L K Rodger BSc MSc CGeol CSci FGS
Approved for Issue By	

REFERENCES

- AGS : 2005 : Electronic transfer of geotechnical and geoenvironmental data (Edition 3.1 including addendum May 2005). Association of Geotechnical and Geoenvironmental Specialists.
- BGS England and Wales Sheet 35 and 44 : 1998 : Whitby and Scalby. 1:50 000 geological map (solid and drift). British Geological Survey
- BS 1377 : 1990 : Methods of test for soils for civil engineering purposes. British Standards Institution.
- BS 5930+A2 : 2010 : Code of practice for site investigations (Amendment 2). British Standards Institution.
- BS EN 1997-2 : 2007 : Eurocode 7 - Geotechnical design - Part 2 Ground investigation and testing. British Standards Institution.
- BS EN ISO 14688-1 : 2002 : Geotechnical investigation and testing - Identification and classification of soil - Part 1 Identification and description. British Standards Institution.
- BS EN ISO 14689-1 : 2003 : Geotechnical investigation and testing - Identification and classification of rock - Part 1 Identification and description. British Standards Institution.
- BS EN ISO 22475-1 : 2006 : Geotechnical investigation and testing – Sampling methods and groundwater measurements - Part 1 Technical principles for execution. British Standards Institution.
- BS EN ISO 22476-3 : 2005 : Geotechnical investigation and testing - Field testing - Part 3 Standard penetration test. British Standards Institution.

ENCLOSURE A
EXPLORATORY HOLE RECORDS

Key to Exploratory Hole Records
SPT Hammer Energy Reports
Borehole Logs
Dynamic Sampler Hole Logs

Key
Calibration certificates for DC1 and JB15
BH1 to 6
WS1 to 5

Key to Exploratory Hole Records

SAMPLES

Undisturbed

U	Driven tube sample	} nominally 100 mm diameter and full recovery unless otherwise stated
UT	Driven thin wall tube sample	
TW	Pushed thin wall tube sample	
P	Pushed piston sample	
L	Liner sample (from Windowless or similar sampler), full recovery unless otherwise stated	
CBR	CBR mould sample	
BLK	Block sample	
CS	Core sample (from rotary core) taken for laboratory testing	
AMAL	Amalgamated sample	

Disturbed

D	Small sample
B	Bulk sample

Other

W	Water sample
G	Gas sample

	Environmental chemistry samples (in more than one container where appropriate)
ES	Soil sample
EW	Water sample

Comments

Sample reference numbers are assigned to every sample taken. A sample reference of 'NR' indicates that attempt was made to take a tube sample, however, there was no recovery.

Monitoring samples taken after completion of hole construction are not shown on the exploratory hole logs.

TESTS

SPT S or SPT C	Standard Penetration Test, open shoe (S) or solid cone (C)
----------------	--

The Standard Penetration Test is defined in BS EN ISO 22476-3 (2005). The incremental blow counts are given in the Field Records column; each increment is 75 mm unless stated otherwise and any penetration under self weight in mm (SW) is noted. Where the full 300 mm test drive is achieved the total number of blows for the test drive is presented as N = ** in the Test column. Where the test drive blows reach 50 the total blow count beyond the seating drive is given (without the N = prefix).

IV	<i>in situ</i> Vane shear strength, peak (p) and remoulded (r)
HV	Hand vane shear strength, peak (p) and remoulded (r)
PP	Pocket penetrometer test, converted to shear strength
KFH, KRH, KPI	Permeability tests (KFH = falling head, KRH = rising head; KPI = packer inflow); results provided in Field Records column (one value per stage for packer tests)

DRILLING RECORDS

The mechanical indices (TCR/SCR/RQD & If) are defined in BS 5930+A2 (2010)

TCR	Total Core Recovery, %
SCR	Solid Core Recovery, %
RQD	Rock Quality Designation, %
If	Fracture spacing, mm. Minimum, typical and maximum spacings are presented. The term non-intact (NI) is used where the core is fragmented.

Flush returns, estimated percentage with colour where relevant, are given in the Records column

CRF	Core recovered (length in m) in the following run
AZCL	Assessed zone of core loss
NR	Not recovered

GROUNDWATER



Groundwater strike



Groundwater level after standing period

Notes:
See report text for full references of standards

Project SANDSEND SLOPE STABILISATION
Project No. A1077-11
Carried out for Balfour Beatty Living Places

Key

Sheet 1 of 2

Key to Exploratory Hole Records

INSTALLATION

Standpipe/ piezometer

Details of standpipe/piezometer installations are given on the Record. Legend column shows installed instrument depths including slotted pipe section or tip depth, response zone filter material type and layers of backfill.

SP
SPIE
PPIE
EPIE



The type of instrument installed is indicated by a code in the Legend column at the depth of the response zone:
Standpipe
Standpipe piezometer
Pneumatic piezometer
Electronic piezometer

Inclinometer or Slip Indicator

The installation of vertical profiling instruments is indicated on the Record. The base of tubing is shown in the Legend column.

ICE
ICM
SLIP



The type of instrument installed is indicated by a code in the Legend column at the base of the tubing:
Biaxial inclinometer
Inclinometer tubing for use with probe
Slip indicator

Settlement Points or Pressure Cells

The installation of single point instruments is indicated on the Record. The location of the measuring device is shown in the Legend column.

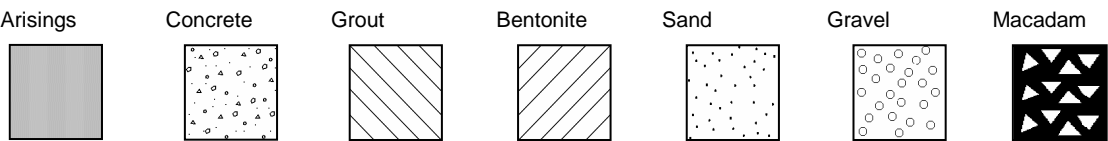
ESET
ETM
EPCE
PPCE



The type of instrument installed is indicated by a code in the Legend column:
Electronic settlement cell/gauge
Magnetic extensometer settlement point
Electronic embedment pressure cell
Electronic push in pressure cell

INSTALLATION LEGENDS

A legend describing the installation is shown in the rightmost column. Legends additional to BS5930 are used to describe the backfill materials as indicated below.



NOTES

- 1 Soils and rocks are described in accordance with BS EN ISO 14688-1 (2002) and 14689-1 (2003) respectively as amplified by BS 5930+A2 (2010).
- 2 For fine soils consistency determined in the field by the logger is reported for those strata where undisturbed samples are available. The consistency is qualified and given (in brackets) when, in the opinion of the logger, the sample is disturbed but the assessed consistency is reasonably representative of the in situ conditions; in these circumstances it will normally underestimate consistency in situ. No consistency is given where the samples available are too disturbed to allow a reasonable assessment.
- 3 Evidence of the occurrence of very coarse particles (cobbles and boulders) is presented on the logs, however, because of their size in relation to the exploratory hole these records may not be fully representative of their size and frequency in the ground mass.
- 4 The declination of bedding and joints is given with respect to the normal to the core axis. Thus in a vertical borehole this will be the dip.
- 5 The assessment of SCR, RQD and Fracture Spacing excludes artificial fractures
- 6 Strata legends are in accordance with BS 5930+A2 (2010).
- 7 Water level observations of discernible entries during the advancing of the exploratory hole are given at the foot of the log and in the Legend column. The term "none observed" is used where no discrete entries are identified although this does not necessarily indicate that the hole has not been advanced below groundwater level. Under certain conditions groundwater cannot be observed, for instance, drilling with water flush or overwater, or boring at a rate much faster than water can make its way into the borehole (ref BS5930+A2:2010, Clause 47.2.7). In addition, where appropriate, water levels in the hole at the time of recovering individual samples or carrying out in situ tests and at shift changes are given in the Records column.
- 8 The borehole logs present the results of Standard Penetration Tests recorded in the field without correction or interpretation. However, in certain ground conditions (eg high hydraulic head or where very coarse particles are present) some judgement may be necessary in considering whether the results are representative of in situ mass conditions.

Updated March 2011

Notes: See report text for full references of standards	Project	SANDESEND SLOPE STABILISATION	Key
	Project No.	A1077-11	
	Carried out for	Balfour Beatty Living Places	Sheet 2 of 2

Hammer Energy Report



Soil Mechanics

Date of test: 25/11/2010

Hammer ID: DC1

Instrumented rod:

Type: BW
Cross-sectional area (A_a): 11.30 cm²
Young's modulus (E_a): 207000 MPa
Length: 0.60 m

Hammer mass (m): 63.5 kg

Fall height (h): 0.76 m

Test type: SPT

Manufacturer: Archway

Model: Automatic Trip Hammer

Test rod type: BW

Rig: Dando 3000

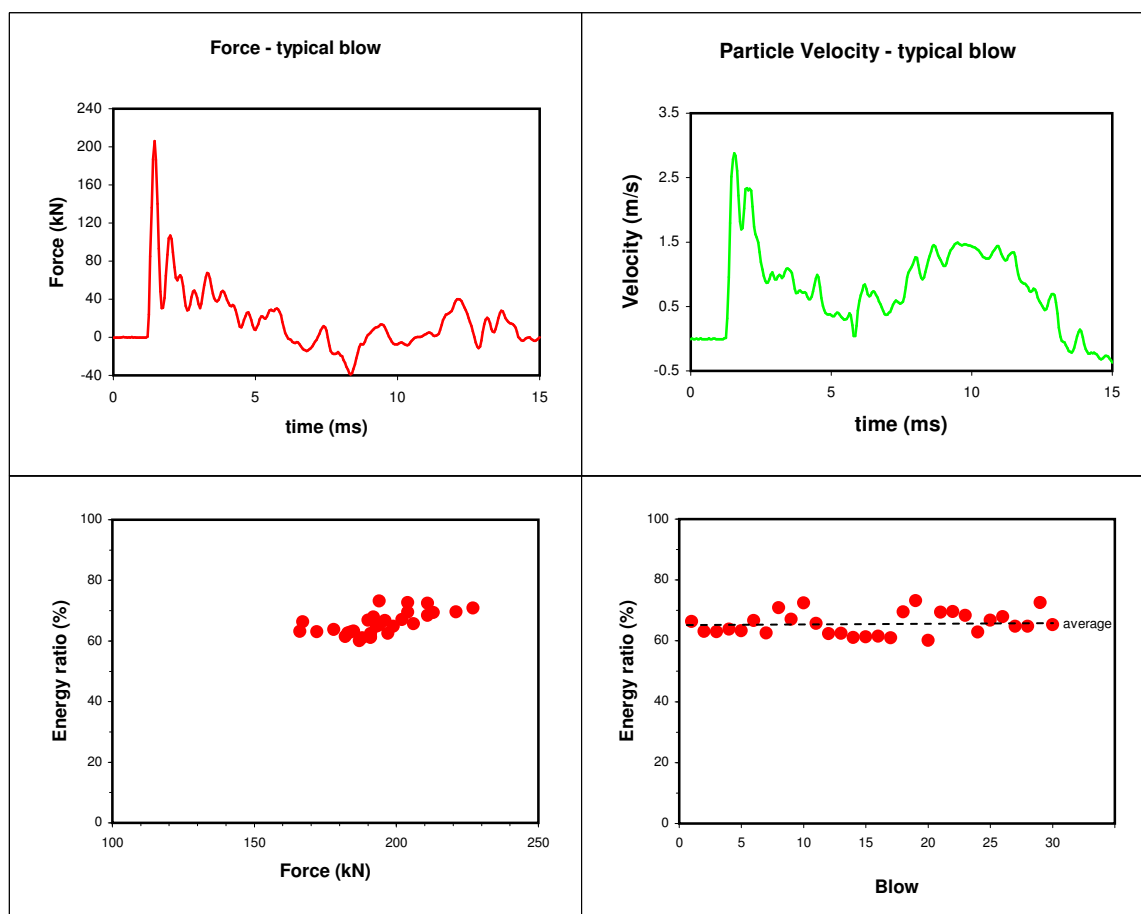
Rig ID: CT3

Type: Cable Percussion

Foreman: D Clay

Remarks:

Data obtained from test carried out in BH1, located in SM Doncaster yard. Test carried out at depth of 9.20 mbgl, with a total blow count of 30. Energy determined from every blow.



Theoretical energy (E_{theor}) = $m \times g \times h$ =

0.473 kN-m (473 J)

Measured energy (E_{meas}) average of 30 blows =

0.311 kN-m

Energy ratio = $\frac{E_{\text{meas}}}{E_{\text{theor}}}$ =

66 %

Test carried out by: Rob Cooke

Test carried out in accordance with BS EN ISO 22476-3:2005

Signed for issue:

Equipment used: SPT Analyzer Serial No. 4032T



SPT Hammer Energy Test Report

in accordance with BSEN ISO 22476-3:2005

J.B. Site Investigations
Windmill Way West
Ramparts Business Park
BERWICK-upon-TWEED
TD15 1TB

SPT Hammer Ref: JB.15
Test Date: 24/06/2011
Report Date:
File Name: JB15.spt
Test Operator: AH

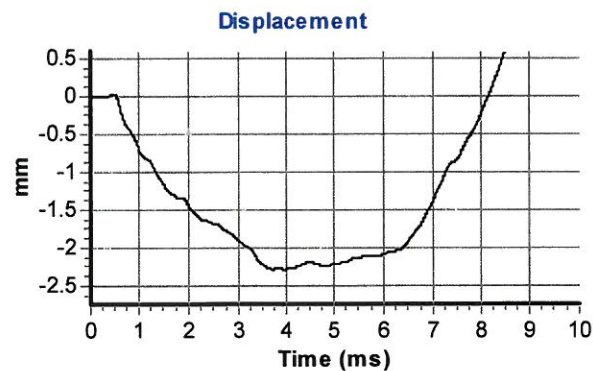
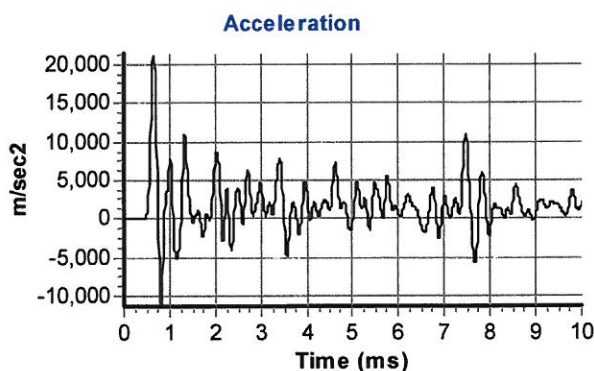
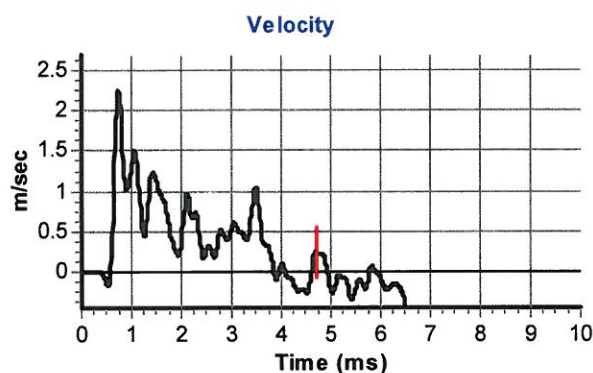
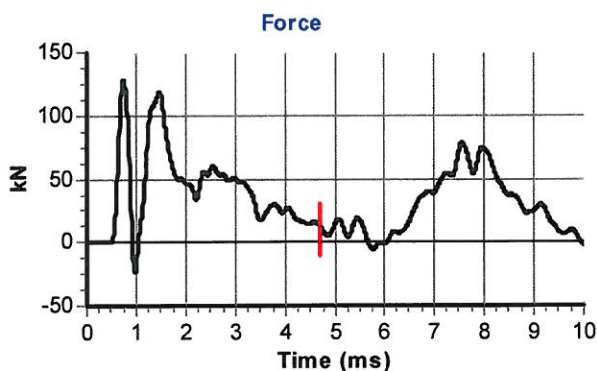
Instrumented Rod Data

Diameter d_r (mm): 54
Wall Thickness t_r (mm): 6.5
Assumed Modulus E_a (GPa): 208
Accelerometer No.1: 6454
Accelerometer No.2: 6456

SPT Hammer Information

Hammer Mass m (kg): 63.5
Falling Height h (mm): 760
SPT String Length L (m): 14.0

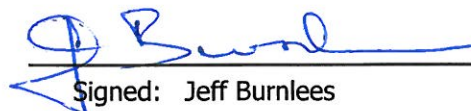
Comments / Location



Calculations

Area of Rod A (mm^2): 970
Theoretical Energy E_{theor} (J): 473
Measured Energy E_{meas} (J): 277

Energy Ratio E_r (%): **59**


Signed: Jeff Burnlees
Title: Proprietor

The recommended calibration interval is 12 months

Φ

Drilled DC/CL Logged CH/JMH Checked PH		Start 05/10/2011 End 06/10/2011		Equipment, Methods and Remarks Dando 3000 and Beretta T51. Cable percussion boring followed by rotary core drilling (PWF size) using air mist flush. SPT: hammer ID DC1, rods BW.			Depth from 0.00m to 24.60m to 24.60m Diameter 200mm Casing Depth 24.60m				Ground Level +44.48 mOD Coordinates E 486765.86 National Grid N 512240.04 Chainage			
Samples and Tests							Strata							
Depth		Type & No	Records	Date Casing	Time Water	Description					Depth, Level/ (Thickness)	Legend	Backfill/ Instrument	
0.10 0.10-0.50		D 1 B 2	0.00-1.20 m Hand excavated inspection pit.			Orangish and light brown sandy slightly gravelly CLAY with frequent rootlets. Gravel is subangular fine of coal and mudstone. (TOPSOIL)					(0.50)			
0.50 0.50-1.00		D 3 B 4				Firm orangish yellow and brown slightly gravelly sandy CLAY. Gravel is angular to subangular fine to medium of various lithologies.					0.50 +43.98			
1.00 1.20-1.65		ES 4A U 5	22 blows								(1.10)			
1.65-1.85		D 6				Firm, becoming stiff below 6.00m, dark brown slightly sandy slightly gravelly CLAY. Gravel is angular to subrounded fine to medium, locally coarse, of various lithologies predominantly sandstone and quartzite. (GLACIAL TILL)					1.60 +42.88			
2.00-2.45 2.00-2.45 2.00		D 7 B 8 ES 8A												
3.00-3.45		U 9	28 blows 400 mm rec											
3.45-3.65		D 10												
4.00-4.45 4.00-4.45		D 11 B 12												
4.45-4.90		SPT S	N=6 (1,1/1,2,2,1)	3.10	dry									
5.00-5.45		U 13	24 blows	3.40	dry									
5.45-5.65		D 14												
5.65-6.10 5.65-6.10 5.65-6.10		SPT S D 15 B 16	N=12 (1,2/2,3,3,4)	3.10	dry						(8.40)			
6.50-6.95		U 17	28 blows	6.10	dry									
6.95-7.15		D 18												
7.15-7.60 7.15-7.60 7.15-7.60		SPT S D 19 B 20	N=12 (5,3/2,3,3,4)	6.10	dry	7.00-7.15 m 1 No. sandstone cobble								
8.00-8.45		U 21	27 blows 400 mm rec	7.60	dry									
8.45-8.65		D 22												
8.65-9.10 8.65-9.10 8.65-9.10		SPT S D 23 B 24	N=15 (2,2/3,3,4,5)	7.60	dry									
9.50-9.95		U 25	27 blows	9.10	dry									
9.95-10.15		D 26												
Depth		Type & No	Records	Date Casing	Time Water									
Groundwater Entries						Depth Related Remarks *					Chiselling			
No. Struck (m)		Post strike behaviour		Depth sealed (m)		From to (m)					Depths (m)	Time	Tools used	
Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.						Project SANDSEND SLOPE STABILISATION					Borehole			
Scale 1:50						Project No. A1077-11					BH1			
(c) ESG www.esg.co.uk 426.4813/12/2011 15.24.22						Carried out for Balfour Beatty Living Places					Sheet 1 of 4			

Borehole Log



Soil Mechanics

Drilled DC/CL Logged CH/JMH Checked PH	Start 05/10/2011 End 06/10/2011	Equipment, Methods and Remarks Dando 3000 and Beretta T51. Cable percussion boring followed by rotary core drilling (PWF size) using air mist flush. SPT: hammer ID DC1, rods BW.	Depth from 0.00m to 24.60m Diameter 200mm Casing Depth 24.60m	Ground Level +44.48 mOD Coordinates E 486765.86 National Grid N 512240.04 Chainage												
Samples and Tests			Strata													
Depth	Type & No	Records	Date Casing	Time Water	Description (Continued from Sheet 1)	Depth, Level (Thickness)	Legend	Backfill/ Instruments								
10.15-10.60 10.15-10.60 10.15-10.60	SPT S D 27 B 28	N=17 (2,2/3,4,4,6)	9.10	dry	Stiff reddish brown, locally greyish brown, slightly sandy slightly gravelly, locally grading to gravelly, CLAY. Gravel is angular to subrounded fine to medium of various lithologies predominantly mudstone. (GLACIAL TILL)	10.00 +34.48										
11.00-11.45	U 29	33 blows	10.60	dry												
11.45-11.65	D 30															
11.65-12.10 11.65-12.10 11.65-12.10	SPT S D 31 B 32	N=19 (2,3/4,4,5,6)	10.60	dry	11.40 m 1 No. cobble of mudstone	(4.25)										
12.50-12.95	U 33	34 blows	12.10	dry												
12.95-13.15	D 34															
13.15-13.60 13.15-13.60 13.15-13.60	SPT S D 35 B 36	N=19 (2,3/3,5,5,6)	12.10	dry												
14.00-14.45	U 37	32 blows	13.60	dry												
14.45-14.65	D 38				Light brown silty fine to medium SAND. (GLACIAL TILL)	14.25 +30.23										
14.65-15.10 14.65-15.10 14.65-15.10	SPT S D 39 B 40	N=24 (2,4/5,6,6,7)	13.60	dry	Stiff dark brown thinly laminated fissured slightly sandy slightly gravelly CLAY. Fissures are very closely spaced, randomly orientated, smooth, polished. Gravel is angular to subrounded fine to medium of various lithologies with occasional 15mm pockets of light brown fine sand. (GLACIAL TILL)	14.45 +30.03										
15.50-15.95	U 41	41 blows	15.10	dry		(1.55)										
15.95-16.15	D 42															
16.15-16.60 16.15-16.60 16.15-16.60	SPT S D 43 B 44	N=24 (4,5/4,6,6,8)	15.10	15.70	Dark brown sandy very clayey angular to subrounded fine to medium GRAVEL of various lithologies predominantly mudstone. (Possibly weathered MUDSTONE)	16.00 +28.48 16.20 +28.28										
17.00-17.45	U 45	45 blows	16.60	dry	Stiff dark brown slightly gravelly sandy CLAY. Gravel is angular to subangular fine to medium, locally coarse, of various lithologies predominantly mudstone. (Possibly weathered MUDSTONE)	(1.90)										
17.45-17.65	D 46															
17.65-18.10 17.65-18.10 17.65-18.10	SPT S D 47 B 48	N=33 (4,5/6,7,10,10)	16.60	dry												
18.10	D 49															
18.50-18.95 18.50-18.95 18.70	SPT C B 50 D 51	N=26 (3,4/5,6,6,9)	18.10	dry	Medium dense dark brown sandy clayey GRAVEL. Gravel is angular to rounded fine to coarse of various lithologies predominantly mudstone. (Possibly weathered MUDSTONE)	18.10 +26.38 18.70 +25.78										
19.50-19.95	U 52	60 blows	19.40	dry	Stiff to very stiff dark brown, becoming greyish brown below 20.50m, slightly sandy slightly gravelly CLAY. Gravel is angular to rounded fine to medium of various lithologies predominantly mudstone and sandstone. (GLACIAL TILL)											
19.95-20.15	D 53															
Depth	Type & No	Records	Date Casing	Time Water	Stratum continues to 23.90 m											
Groundwater Entries <table border="1"> <thead> <tr> <th>No.</th> <th>Struck (m)</th> <th>Post strike behaviour</th> <th>Depth sealed (m)</th> </tr> </thead> <tbody> <tr> <td>1</td> <td>16.00</td> <td>Rose to 15.70 m after 20 minutes.</td> <td>-</td> </tr> </tbody> </table>					No.	Struck (m)	Post strike behaviour	Depth sealed (m)	1	16.00	Rose to 15.70 m after 20 minutes.	-	Depth Related Remarks * From to (m)		Chiselling Depths (m) Time Tools used	
No.	Struck (m)	Post strike behaviour	Depth sealed (m)													
1	16.00	Rose to 15.70 m after 20 minutes.	-													
Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.					Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places		Borehole BH1 Sheet 2 of 4									

Borehole Log



Soil Mechanics

Drilled DC/CL Logged CH/JMH Checked PH	Start 05/10/2011 End 06/10/2011	Equipment, Methods and Remarks Dando 3000 and Beretta T51. Cable percussion boring followed by rotary core drilling (PWF size) using air mist flush. SPT: hammer ID DC1, rods BW.	Depth from 0.00m to 24.60m Diameter 200mm Casing Depth 24.60m	Ground Level +44.48 mOD Coordinates E 486765.86 National Grid N 512240.04 Chainage				
Samples and Tests			Strata					
Depth	Type & No	Records	Date Casing	Time Water	Description (Continued from Sheet 2)	Depth, Level (Thickness)	Legend	Backfill/ Instruments
20.15-20.60 20.15-20.60 20.15-20.60	SPT S D 54 B 55	N=38 (4,6/8,9,10,11)	19.40	dry	Stiff to very stiff dark brown, becoming greyish brown below 20.50m, slightly sandy slightly gravelly CLAY. Gravel is angular to rounded fine to medium of various lithologies predominantly mudstone and sandstone. (GLACIAL TILL)			
21.00-21.45	U 56	70 blows	19.40	dry		(5.20)		
21.45-21.65	D 57							
21.65-22.10 21.65-22.10 21.65-22.10	SPT S D 58 B 59	N=29 (11,7/6,7,8,8)	19.40	dry				
22.50-22.95	U 60	60 blows 400 mm rec	19.40	dry				
22.95-23.15	D 61							
23.15-23.60 23.15-23.60 23.15-23.60	SPT S D 62 B 63	N=33 (4,6/7,8,9,9)	19.40	dry				
23.90	D 64							
24.60-24.86 24.60-24.86	SPT S D 65	50 (7,10/18,32 for 35mm)	05/10/2011 19.40 06/10/2011 19.40	dry 1030 22.30	Light grey, locally dark grey, thinly laminated silty MUDSTONE. Recovered as angular to subangular fine to coarse gravel.	23.90 +20.58 (0.96)		
24.60-25.60	55 0 0 NA NA NA				Extremely weak thinly laminated grey MUDSTONE. Weathering is complete loss of structure to gravelly clay.	24.86 +19.62 (0.69)		
25.60-26.60	100 52 43 NI 90 300				Weak thinly laminated grey SILTSTONE with rare brown sandstone bands. Weathering is an orangish brown discolouration on fracture surfaces penetrating up to 5mm and localised reduction in strength to very weak. Fractures are subhorizontal, very closely to medium spaced, planar, smooth.	25.55 +18.93 (1.30)		
26.60-28.10	100 81 29				Weak thinly inter-laminated grey SILTSTONE and light brown fine grained SANDSTONE. Weathering is an orangish brown discolouration on fracture surfaces penetrating up to 2mm. Fractures are subhorizontal, very closely to closely spaced, planar, smooth.	26.85 +17.63 (0.55)		
28.10-29.60	100 77 73 NI 120 360				Weak thinly laminated grey SILTSTONE with rare brown sandstone bands. Weathering is an orangish brown discolouration on fracture surfaces penetrating up to 5mm and localised reduction in strength to very weak. Fractures are subhorizontal, very closely to medium spaced, planar, smooth.	27.40 +17.08 (0.50)		
		Flush: 24.60-34.10 air mist, 100 %			Medium strong thinly laminated brown and grey medium grained SANDSTONE. Weathering is an orangish brown	27.90 +16.58 (2.00)		
Depth	ICR ROD	If	Records/Samples	Date Casing	Time Water	Stratum continues to 30.50 m		
Groundwater Entries No. Struck Post strike behaviour Depth sealed (m)						Depth Related Remarks * From to (m)		
Chiselling Depths (m) Time Tools used								
Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.						Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places		
Scale 1:50 (c) ESG www.esg.co.uk 426.4813/12/2011 15:24:24 AGS						Borehole BH1 Sheet 3 of 4		

Borehole Log



Soil Mechanics

Drilled DC/CL Logged CH/JMH Checked PH		Start 05/10/2011 End 06/10/2011		Equipment, Methods and Remarks Dando 3000 and Beretta T51. Cable percussion boring followed by rotary core drilling (PWF size) using air mist flush. SPT: hammer ID DC1, rods BW.			Depth from 0.00m to 24.60m to 24.60m 34.10m Diameter 200mm 121mm Casing Depth 24.60m		Ground Level +44.48 mOD Coordinates E 486765.86 N 512240.04 Chainage			
Samples and Tests						Strata						
Depth		TCR SCR RGD	If	Records/Samples	Date Casing	Time Water	Description (Continued from Sheet 3)		Depth, Level/ (Thickness)	Legend	Backfill/ Instruments	
29.60-31.10		92 80 70	NI 50 240				27.90m - 29.90m : discolouration on fracture surfaces penetrating up to 30mm. Fractures are: 1) subhorizontal, closely to medium spaced, planar, smooth. 2) 50 degrees, medium spaced, planar, rough.		29.60-29.72 m AZCL			
			NI 70 180				30.32-30.38 m NI 30.38-30.50 m recovered as gravelly clay		30.50 +13.98			
							29.90m - 30.50m : Weak thinly laminated grey SILTSTONE. Weathering is an orangish brown discolouration on fracture surfaces penetrating up to 5mm. Fractures are subhorizontal, closely spaced, planar, smooth.		31.10-31.30 m 1no. vertical, planar, rough fracture 31.30-31.38 m NI			31.50 +12.98
31.10-32.60		100 35 20					Weak thinly laminated grey fine grained SANDSTONE. Weathering is an orangish brown discolouration on fracture surfaces penetrating up to 10mm. Fractures are subhorizontal, closely spaced, planar, smooth.		32.60-33.00 m AZCL			(2.60)
32.60-34.10		73 35 7	NI NI 100		06/10/2011 24.60		Extremely weak thinly laminated grey MUDSTONE. Weathering is loss of structure to gravel and localised orangish brown discolouration on fracture surfaces. Fractures are: 1) recovered as non-intact 2) subhorizontal, very closely to closely spaced, planar, rough.		33.12-33.44 m 1no. 75 degree planar, rough fracture			
EXPLORATORY HOLE ENDS AT 34.10 m												
GROUNDWATER ENTRIES No. Struck Post strike behaviour Depth sealed (m)												
Depth Related Remarks * From to (m)												
Chiselling Depths (m) Time Tools used												
Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.						Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places			Borehole BH1 Sheet 4 of 4			

Borehole Log



Soil Mechanics

Drilled DC/CL Logged CH/JMH Checked PH		Start 03/10/2011 End 05/10/2011	Equipment, Methods and Remarks Dando 3000 and Beretta T51. Cable percussion boring followed by rotary core drilling (PWF size) using air mist flush. SPT: hammer ID DC1, rods BW.		Depth from 0.00m to 17.00m Diameter 200mm Casing Depth 17.00m	Ground Level +45.56 mOD Coordinates E 486709.85 National Grid N 512274.24 Chainage		
Samples and Tests					Strata			
Depth	Type & No	Records	Date Casing	Time Water	Description	Depth, Level/ (Thickness)	Legend	Backfill/ Instruments
0.10	D 1	0.00-1.20 m Hand excavated inspection pit.			Orangish brown slightly sandy CLAY. Sand is fine to coarse. (TOPSOIL)	(0.50)		
0.50	D 2					0.50 +45.06		
0.50-1.00	D 3				Stiff orangish brown, becoming dark brown below 3.00m, slightly sandy slightly gravelly CLAY. Gravel is angular to rounded fine to coarse of various lithologies. (GLACIAL TILL)			
1.00	ES 3A							
1.20-1.65	U 4	38 blows		dry				
1.65-1.85	D 5							
2.00-2.45	SPT S	N=7 (1,1/1,2,2,2)	1.60	dry				
2.00-2.45	D 6							
2.00	ES 6A							
2.00-2.45	B 7							
3.00-3.45	U 8	50 blows		dry				
3.45-3.65	D 9							
4.00-4.45	SPT S	N=11 (1,2/3,2,3,3)	3.10	dry				
4.00-4.45	D 10							
4.00-4.45	B 11							
5.00-5.45	U 12	30 blows 400 mm rec	4.60	dry				
5.45-5.65	D 13				5.45-5.60 m pocket of dark brown very sandy slightly gravelly clay. Gravel is subangular to subrounded fine to medium of various lithologies			
6.00-6.45	SPT S	N=11 (1,2/2,2,3,4)	5.80	dry				
6.00-6.45	D 14							
6.00-6.45	B 15							
7.00-7.45	U 16	38 blows	5.80	dry				
7.45-7.65	D 17							
8.00-8.45	U 18	29 blows	7.60	dry		(15.20)		
8.45-8.65	D 19							
9.00-9.45	U 20	35 blows	7.60	dry				
9.45-9.65	D 21							
Depth	Type & No	Records	Date Casing	Time Water	Stratum continues to 15.70 m			
Groundwater Entries					Depth Related Remarks *		Chiselling	
No.	Struck (m)	Post strike behaviour	Depth sealed (m)		From	to (m)	Depths (m)	Time Tools used
Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.					Project SANDSEND SLOPE STABILISATION		Borehole	
Project No. A1077-11					Carried out for Balfour Beatty Living Places		BH2	
Scale 1:50							Sheet 1 of 5	

Borehole Log



Soil Mechanics

Drilled DC/CL Logged CH/JMH Checked PH		Start 03/10/2011 End 05/10/2011	Equipment, Methods and Remarks Dando 3000 and Beretta T51. Cable percussion boring followed by rotary core drilling (PWF size) using air mist flush. SPT: hammer ID DC1, rods BW.		Depth from 0.00m 17.00m 26.20m	to 17.00m 26.20m 40.20m	Diameter 200mm 150mm 121mm	Casing Depth 17.00m 26.20m 26.20m	Ground Level +45.56 mOD Coordinates E 486709.85 National Grid N 512274.24 Chainage
Samples and Tests					Strata				
Depth	Type & No	Records	Date Casing	Time Water	Description (Continued from Sheet 1)	Depth, Level/ (Thickness)	Legend	Backfill/ Instruments	
10.00-10.45	U 22	35 blows	7.60	dry	Stiff orangish brown, becoming dark brown below 3.00m, slightly sandy slightly gravelly CLAY. Gravel is angular to rounded fine to coarse of various lithologies. (GLACIAL TILL)				
10.45-10.65	D 23								
11.00-11.45	U 24	30 blows	7.60	dry					
11.45-11.65	D 25								
12.00-12.45	U 26	36 blows	7.60	dry	12.00 m very sandy				
12.45-12.65	D 27								
13.00-13.65 13.00-13.45	B 28 U NR	55 blows No recovery	7.60	dry					
14.00-14.45	U 29	35 blows	7.60	dry					
14.45-14.65	D 30								
15.00-15.45	U 31	40 blows 400 mm rec	7.60	dry					
15.45-15.65	D 32								
15.70	D 33								
15.90	W 36								
16.00-16.45	U 34	40 blows 400 mm rec			Dark brown sandy very clayey GRAVEL. Gravel is angular to rounded fine to coarse of various lithologies predominantly mudstone and sandstone. (GLACIAL TILL)	15.70 +29.86 15.90 +29.66			
16.45-16.65	D 35		03/10/2011 7.60	dry					
17.00-17.45	U 37	30 blows 400 mm rec	04/10/2011 7.60	0800 13.70	Stiff dark brown, locally orange and brown, sandy slightly gravelly CLAY. Gravel is subangular to rounded fine to medium of various lithologies. (GLACIAL TILL)	(2.00)			
17.45-17.65	D 38								
17.90 18.00-18.45 18.00-18.45	W 39 SPT S D 40 B 41	N=37 (3,5/9,9,10,9)	18.00	14.50	Orange brown very sandy clayey angular to subrounded fine to medium GRAVEL of various lithologies, predominantly quartzite and flint. (GLACIAL TILL)	17.90 +27.66 (0.60)			
18.50	D 42				Dark brown thinly laminated fissured slightly sandy slightly gravelly CLAY. Gravel is angular to subrounded fine to medium of various lithologies. Fissures are very closely spaced, randomly orientated, smooth, polished. (GLACIAL TILL)	18.50 +27.06 (0.50)			
19.00-19.45	U 43	55 blows	18.80	dry		19.00 +26.56			
19.45-19.65	D 44				Stiff, locally very stiff, dark brown				
Depth	Type & No	Records	Date Casing	Time Water	Stratum continues to 25.40 m				
Groundwater Entries					Depth Related Remarks *		Chiselling		
No.	Struck (m)	Post strike behaviour	Depth sealed (m)		From	to (m)	Depths (m)	Time Tools used	
1	15.90	Rose to 14.90 m after 20 minutes.							
2	17.90	Rose to 14.30 m after 20 minutes. Medium inflow.							
Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.					Project SANDSEND SLOPE STABILISATION		Borehole		
Project No. A1077-11					Balfour Beatty Living Places		BH2		
Scale 1:50							Sheet 2 of 5		

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Drilled DC/CL Logged CH/JMH Checked PH		Start 03/10/2011 End 05/10/2011		Equipment, Methods and Remarks Dando 3000 and Beretta T51. Cable percussion boring followed by rotary core drilling (PWF size) using air mist flush. SPT: hammer ID DC1, rods BW.		Depth from to Diameter Casing Depth 0.00m 17.00m 200mm 17.00m 17.00m 26.20m 150mm 26.20m 26.20m 40.20m 121mm 26.20m				Ground Level +45.56 mOD Coordinates E 486709.85 National Grid N 512274.24 Chainage													
Samples and Tests						Strata																	
Depth		Type & No		Records		Date Casing		Time Water		Description (Continued from Sheet 2)			Depth, Level/ (Thickness)		Legend		Backfill/ Instruments						
20.00-20.45		U 45		43 blows 400 mm rec		19.60		dry		slightly sandy slightly gravelly CLAY. Gravel is angular to subrounded fine to medium, occasionally coarse, of various lithologies predominantly mudstone. (GLACIAL TILL)			(6.40)										
20.45-20.65		D 46																					
21.00-21.45		U 47		50 blows 400 mm rec		19.60 dry																	
21.45-21.65		D 48																					
22.00-22.45		U 49		60 blows 350 mm rec		19.60 dry																	
22.45-22.65		D 50								Very weak light grey and green, locally weathered orange, thinly laminated MUDSTONE. Recovered as angular to subangular fine to coarse gravel. (Weathered MUDSTONE)			25.40 +20.16 (0.90)										
23.00-23.45		U 51		70 blows 400 mm rec		19.60 dry																	
23.45-23.65		D 52																					
24.00-24.45		U 53		65 blows		19.60 dry																	
24.45-24.65		D 54																					
25.00-25.45		U 55		65 blows 400 mm rec		19.60 dry				Weak locally very weak thinly laminated brown fine grained SANDSTONE. Weathering is localised loss of structure to gravelly clay. Fractures are subhorizontal, closely to medium spaced, planar, rough to smooth.			26.30 +19.26										
25.45-25.65		D 56																					
26.10-26.29 26.10-26.30		SPT S D 57		50 (16,9 for 40mm/50)		04/10/2011 19.60 dry																	
26.20-27.20		66 64 54				05/10/2011 19.60 0800 dry																	
27.20-28.50		85 30 23		NI 110 240																			
28.50-30.20		85 69 59						26.20-26.54 m AZCL 26.97-27.20 m 1no 75 degree, planar, rough fracture 27.20-27.60 m AZCL 27.60-27.67 m NI 27.96-28.10 m recovered as gravelly clay 28.30-28.47 m recovered as gravelly clay 28.53-28.70 m 1no subvertical, planar, rough fracture 28.70-28.75 m AZCL 28.75-29.10 m NI 29.20-29.39 m 1no. vertical, planar, smooth fracture 29.69-29.95 m 1no 70 degree			(3.65)												
Depth		ICR SCR RGO		If		Records/Samples		Date Casing		Time Water		Stratum continues to 32.00 m											
Groundwater Entries No. Struck (m) Post strike behaviour						Depth sealed (m)						Depth Related Remarks * From to (m)						Chiselling Depths (m) Time Tools used 25.90 -26.10 30 mins					
Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.						Project Project No. Carried out for						SANDSEND SLOPE STABILISATION A1077-11 Balfour Beatty Living Places						Borehole BH2 Sheet 3 of 5					
(c) ESG www.esg.co.uk 426.4813/12/2011 15:24:32						AGS																	
Scale 1:50																							

Borehole Log



Soil Mechanics

Drilled DC/CL Logged CH/JMH Checked PH	Start 03/10/2011 End 05/10/2011	Equipment, Methods and Remarks Dando 3000 and Beretta T51. Cable percussion boring followed by rotary core drilling (PWF size) using air mist flush. SPT: hammer ID DC1, rods BW.	Depth from 0.00m to 17.00m Diameter 200mm Casing Depth 17.00m 17.00m 26.20m 26.20m 40.20m 150mm 121mm	Ground Level +45.56 mOD Coordinates E 486709.85 National Grid N 512274.24 Chainage				
Samples and Tests			Strata					
Depth TCR SCR RGD	If	Records/Samples	Date Casing	Time Water	Description (Continued from Sheet 3)	Depth, Level/ (Thickness)	Legend	Backfill/ Instruments
30.20-31.70	73 36 36	NI 120 270			Very weak thinly laminated grey fine grained SANDSTONE. Fractures are subhorizontal, closely to medium spaced, planar, smooth. planar, smooth fracture 30.92-31.16 m NI 31.07-31.52 m 1no subvertical, planar, rough fracture 31.52-31.64 m NI	(2.05)		
31.70-33.20	100 81 75	100 150 220			Extremely weak thinly laminated grey MUDSTONE. Fractures are subhorizontal, closely to medium spaced, planar, smooth. 32.64-32.81 m 1no. 65 degree, planar, smooth fracture	32.00 +13.56 (1.10)		
33.20-34.70	67 42 22				Very weak to extremely weak thinly laminated grey MUDSTONE. Weathering is localised loss of structure to gravel. Fractures are subhorizontal, very closely to closely spaced, planar rough to smooth. 33.10-33.20 m NI 33.20-33.70 m AZCL 33.88-34.00 m NI 34.07-34.12 m NI 34.43-34.46 m NI 34.70-34.88 m AZCL 34.88-34.92 m NI 35.14-35.16 m NI 35.38-35.67 m NI 35.67-35.98 m 1no. subvertical, undulating, smooth fracture 35.77-35.80 m NI 35.98-36.03 m NI 36.20-36.50 m AZCL 36.50-36.65 m NI 36.65-37.00 m multiple randomly orientated fractures 37.24-37.44 m NI 37.44-37.58 m recovered as gravelly clay 37.70-37.75 m AZCL	33.10 +12.46		
34.70-36.20	73 26 0							
36.20-37.70	80 21 7	NI 70 150				(7.60)		
37.70-39.20	97 67 7							
39.20-40.70	91 45 17							
Depth	TCR SCR RGD	If	Records/Samples	Date Casing	Time Water	Stratum continues to 40.70 m		
Groundwater Entries No. Struck Post strike behaviour Depth sealed (m)						Depth Related Remarks * From to (m)		
Chiselling Depths (m) Time Tools used								
Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.						Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places		
Borehole BH2 Sheet 4 of 5								



Borehole Log



Soil Mechanics

Drilled DC/CL Logged CH/JMH Checked PH		Start 03/10/2011 End 05/10/2011		Equipment, Methods and Remarks Dando 3000 and Beretta T51. Cable percussion boring followed by rotary core drilling (PWF size) using air mist flush. SPT: hammer ID DC1, rods BW.		Depth from 0.00m to 17.00m 17.00m to 26.20m 26.20m to 40.20m		Diameter 200mm 150mm 121mm		Casing Depth 17.00m 26.20m 26.20m		Ground Level +45.56 mOD Coordinates E 486709.85 National Grid N 512274.24 Chainage											
Samples and Tests						Strata																	
Depth		TCR SCR RGD		If		Records/Samples		Date Casing		Time Water		Description (Continued from Sheet 4)		Depth, Level/ (Thickness)		Legend		Backfill/ Instruments					
								05/10/2011 26.20		dry		Very weak to extremely weak thinly laminated grey MUDSTONE. Weathering is localised loss of structure to gravel. Fractures are subhorizontal, very closely to closely spaced, planar rough to smooth. EXPLORATORY HOLE ENDS AT 40.70 m		39.96-40.01 m NI 40.23-40.32 m NI 40.48-40.55 m NI 40.70 +4.86									
Depth		TCR SCR RGD		If		Records/Samples		Date Casing		Time Water													
Groundwater Entries No. Struck Post strike behaviour (m)						Depth sealed (m)						Depth Related Remarks * From to (m)						Chiselling Depths (m) Time Tools used					
Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.												Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places						Borehole BH2 Sheet 5 of 5					
Scale 1:50 (c) ESG www.esg.co.uk 426.4813/12/2011 15:24:36												AGS											

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Drilled DC/CL Logged CH Checked PH		Start 06/10/2011 End 10/10/2011	Equipment, Methods and Remarks Dando 3000 & Beretta T51. Cable percussion boring followed by rotary core drilling (PWF size) using air mist flush. SPT: hammer ID DC1, rods BW.		Depth from 0.00m to 21.10m Diameter 200mm Casing Depth 21.10m	Ground Level +45.95 mOD Coordinates E 486620.13 National Grid N 512295.06 Chainage		
Samples and Tests					Strata			
Depth	Type & No	Records	Date Casing	Time Water	Description	Depth, Level/ (Thickness)	Legend	Backfill/ Instruments
0.10 0.10-0.50 0.30 0.50-1.00	D 1 B 2 ES 2A B 3	0.00-1.20 m Hand excavated inspection pit.			Orangish brown very sandy CLAY. (TOPSOIL)	(0.50)		
					Light brownish and orange slightly gravelly sandy CLAY.	0.50 +45.45		
						(0.90)		
1.20-1.65	U 4	31 blows		dry		1.40 +44.55		
1.65-1.85	D 5				Stiff, becoming firm below 4.50m, reddish brown, locally dark brown, slightly sandy slightly gravelly CLAY. Gravel is angular to subrounded fine to medium, occasionally coarse, of various lithologies predominantly mudstone. (GLACIAL TILL)			
1.85-2.30 1.85-2.30 1.85-2.30 2.00	SPT S D 6 B 7 ES 7A	N=11 (1,1/2,2,3,4)		dry				
2.50-2.95	U 8	38 blows	1.60	dry				
2.95-3.15	D 9							
3.15-3.60 3.15-3.60 3.15-3.60	SPT S D 10 B 11	N=18 (2,3/4,4,5,5)	1.60	dry				
4.00-4.45	U 12	28 blows	3.10	dry				
4.45-4.65	D 13							
4.65-5.10 4.65-5.10 4.65-5.10	SPT S D 14 B 15	N=10 (1,2/2,2,3,3)	3.10	dry				SPIE
5.50-5.95	U 16	28 blows	4.50	dry				
5.95-6.15	D 17							
6.15-6.60 6.15-6.60 6.15-6.60	SPT S D 18 B 19	N=10 (1,2/2,2,3,3)	4.50	dry		(9.40)		
7.00-7.45	U 20	Blows not recorded						
7.45-7.65	D 21							
7.65-8.10 7.65-8.10 7.65-8.10	SPT S D 22 B 23	N=10 (1,2/2,2,3,3)	6.10	dry				
8.50-8.95	U 24	28 blows	7.60	dry				
8.95-9.15	D 25							
9.15-9.60 9.15-9.60 9.15-9.60	SPT S D 26 B 27	N=15 (1,2/3,3,4,5)	7.60	dry				
					9.15 m 1 No. quartzite cobble			
Depth	Type & No	Records	Date Casing	Time Water	Stratum continues to 10.80 m			
Groundwater Entries No. Struck (m) Post strike behaviour Depth sealed (m)					Depth Related Remarks * From to (m)		Chiselling Depths (m)	Time Tools used
Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.					Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places		Borehole BH3 Sheet 1 of 5	

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Drilled DC/CL Logged CH Checked PH		Start 06/10/2011 End 10/10/2011	Equipment, Methods and Remarks Dando 3000 & Beretta T51. Cable percussion boring followed by rotary core drilling (PWF size) using air mist flush. SPT: hammer ID DC1, rods BW.		Depth from 0.00m to 21.10m to 21.10m Diameter 200mm Casing Depth 21.10m		Ground Level +45.95 mOD Coordinates E 486620.13 National Grid N 512295.06 Chainage			
Samples and Tests					Strata					
Depth	Type & No	Records	Date Casing	Time Water	Description (Continued from Sheet 1)		Depth, Level/ (Thickness)	Legend	Backfill/ Instruments	
10.00-10.45	U 28	34 blows	9.10	dry	Stiff, becoming firm below 4.50m, reddish brown, locally dark brown, slightly sandy slightly gravelly CLAY. Gravel is angular to subrounded fine to medium, occasionally coarse, of various lithologies predominantly mudstone. (GLACIAL TILL)		10.80 +35.15			
10.45-10.65	D 29									
10.65-10.93	SPT S	50 (2,18/25,25 for 50mm)	9.10	dry	10.60 m 1 No. sandstone cobble					
10.65-10.90	D 30 B 31									
11.20	D 32									
11.50-11.95	SPT S	N=26 (3,4/4,6,8,8)	10.60	dry	Stiff to very stiff orangish light grey slightly sandy slightly gravelly CLAY. Gravel is angular to subangular fine to medium of mudstone and siltstone. Below 12.50m, becoming dark grey with bands up to 15mm thick of weathered black vitreous coal.		(3.10)			
11.50-11.95	D 33 B 34									
12.50-12.95	SPT S	N=26 (3,5/5,6,7,8)	10.60	dry						
12.50-12.95	D 35 B 36									
13.50-13.95	SPT S	N=31 (3,4/5,8,8,10)	10.60	dry						
13.50-13.95	D 37 B 38									
14.50-14.95	U 39	45 blows	13.60	dry	Stiff to very stiff dark brown slightly sandy slightly gravelly CLAY. Gravel is subangular to subrounded fine to medium of various lithologies predominantly mudstone. (GLACIAL TILL)		13.90 +32.05			
14.95-15.15	D 40						(1.00)			
15.15-15.60	SPT S	N=26 (3,5/7,6,6,7)	13.60	14.40	Dark brownish grey silty slightly gravelly CLAY. Gravel is subangular to subrounded fine of mudstone. (GLACIAL TILL)		14.90 +31.05 (0.30)			
15.15-15.60	D 41 B 42						15.20 +30.75			
16.00-16.45	U 43	45 blows	15.80	dry	Very stiff dark brown slightly sandy slightly gravelly CLAY. Gravel is angular to subrounded fine to medium, rarely coarse, of various lithologies predominantly mudstone. (GLACIAL TILL)					
16.45-16.55	D 44									
16.65-17.10	SPT S	N=28 (3,4/6,6,7,9)	15.80	dry	16.50 m 1 No. cobble of limestone					
16.65-17.10	D 45 B 46									
17.50-17.95	U 47	60 blows	16.60	dry			(4.90)			
17.95-18.15	D 48									
18.15-18.60	SPT S	N=35 (4,5/7,9,9,10)	16.60	dry						
18.15-18.60	D 49 B 50									
19.00-19.45	U 51	62 blows	18.10	dry						
19.45-19.65	D 52									
19.65-20.10	SPT S	N=35 (4,5/7,8,9,11)	18.10	dry						
19.65-20.10	D 53 B 54									
19.65-20.10			06/10/2011							
Depth	Type & No	Records	Date Casing	Time Water	Stratum continues to 20.10 m					
Groundwater Entries					Depth Related Remarks *		Chiselling			
No.	Struck (m)	Post strike behaviour	Depth sealed (m)		From to (m)		Depths (m)	Time	Tools used	
1	14.90	Rose to 14.40 m after 20 minutes. Slow inflow.	-				10.90 - 11.10	30 mins		
Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.					Project SANDSEND SLOPE STABILISATION		Borehole			
Scale 1:50					Project No. A1077-11		BH3			
(c) ESG www.esg.co.uk 426.4813/12/2011 15:24:41					Carried out for Balfour Beatty Living Places		Sheet 2 of 5			

Borehole Log



Soil Mechanics

Drilled Logged Checked	DC/CL CH PH	Start 06/10/2011 End 10/10/2011	Equipment, Methods and Remarks Dando 3000 & Beretta T51. Cable percussion boring followed by rotary core drilling (PWF size) using air mist flush. SPT: hammer ID DC1, rods BW.	Depth from 0.00m 21.10m	to 21.10m 40.10m	Diameter 200mm 121mm	Casing Depth 21.10m 21.10m	Ground Level +45.95 mOD Coordinates E 486620.13 National Grid N 512295.06 Chainage		
Samples and Tests				Strata						
Depth	Type & No	Records	Date Casing	Time Water	Description (Continued from Sheet 2)			Depth, Level/ (Thickness)	Legend	Backfill/ Instruments
20.10	D 55		19.60	dry	Very stiff dark brown slightly sandy slightly gravelly CLAY. Gravel is angular to subrounded fine to medium, rarely coarse, of various lithologies predominantly mudstone. (GLACIAL TILL)			20.10 +25.85		
20.50-21.00	B 56		07/10/2011 19.60	0800 dry				(1.00)		
21.20-21.62 21.20-21.60		SPT S 50 (8,8/ 12,13,15,10 for 45mm) D 57	07/10/2011 19.60	dry	Very weak greyish yellow thinly laminated MUDSTONE. Generally recovered as angular to subangular fine to medium gravelly clay. (Weathered MUDSTONE)			21.10 +24.85		
21.10-22.10	38 20 0		19.60 10/10/2011 19.60	dry 0800 7.90	Medium strong to weak thinly laminated brown fine to medium grained SANDSTONE with occasional grey siltstone bands. Weathering is loss of structure to gravel and orangish brown discolouration on fracture surfaces. Fractures are: 1 No. recovered as NI. 2 No. subhorizontal, very closely to closely spaced, planar, smooth.					
22.10-23.60	95 33 11				21.10-21.62 m AZCL 21.62-21.90 m recovered as slightly gravelly sandy clay 22.10-22.17 m 22.17-22.20 m NI 22.24-22.40 m 1 No. vertical planar smooth fracture 22.75-22.98 m NI 23.00-23.50 m 1 No. subvertical undulating rough fracture 23.04-23.27 m NI			(2.50)		
23.60-25.10	73 53 39				Weak to very weak thinly laminated grey SILTSTONE with occasional brown fine grained sandstone. Weathering is localised loss of structure to gravel. Fractures are subhorizontal, closely to medium spaced, planar, smooth. 24.05-24.19 m 1 No. ** incipient fracture 24.41-24.70 m recovered as gravel 24.70-25.10 m AZCL			23.60 +22.35		
25.10-26.60	100 57 57	500 500 500			Very weak thinly laminated grey MUDSTONE. 25.50-25.60 m NI			25.65 +20.30		
26.60-28.10	87 23 23				Weak to very weak thinly laminated brown medium grained SANDSTONE. Weathering is loss of structure to gravel. Fractures are: 1 No. recovered as NI. 2 No. subvertical, undulating, rough. 26.60-26.90 m AZCL			26.15 +19.80		
28.10-29.60	100 71 59	NI 100 150			Weak thinly laminated brown fine grained SANDSTONE with occasional siltstone bands. Fractures are: 1 No. subhorizontal, very closely to closely spaced, planar, smooth. 2 No. subvertical, undulating, rough. 28.10-28.14 m NI 28.24-28.27 m NI 28.43-28.48 m NI 29.53-29.60 m NI 29.60-29.90 m AZCL 29.90-30.00 m NI			28.10 +17.85		
Depth	TCR ROD	If	Records/Samples	Date Casing	Time Water	Stratum continues to 30.10 m				
Groundwater Entries						Depth Related Remarks *			Chiselling	
No.	Struck (m)	Post strike behaviour		Depth sealed (m)		From to (m)			Depths (m)	Time Tools used
Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.						Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places			Borehole BH3 Sheet 3 of 5	



Borehole Log



Soil Mechanics

Drilled DC/CL Logged CH Checked PH		Start 06/10/2011 End 10/10/2011		Equipment, Methods and Remarks Dando 3000 & Beretta T51. Cable percussion boring followed by rotary core drilling (PWF size) using air mist flush. SPT: hammer ID DC1, rods BW.			Depth from 0.00m to 21.10m Diameter 200mm Casing Depth 21.10m		Ground Level +45.95 mOD Coordinates E 486620.13 National Grid N 512295.06 Chainage		
Samples and Tests						Strata					
Depth	TCR SCR RGD	If	Records/Samples	Date Casing	Time Water	Description (Continued from Sheet 3)			Depth, Level/ (Thickness)	Legend	Backfill/ Instruments
29.60-31.10	80 53 35		Flush: 21.10-40.10 air mist, 100 %			Weak thinly laminated brown fine grained SANDSTONE with occasional siltstone bands. Fractures are: 1 No. subhorizontal, very closely to closely spaced, planar, smooth. 2 No. subvertical, undulating, rough.			30.10 +15.85		
31.10-32.60	87 39 39	NI 200 400		Very weak thinly laminated grey MUDSTONE. Weathering is loss of structure to gravel and occasional orange to brown discolouration on fracture surfaces.			30.67-30.71 m 1 No. 45deg. stepped smooth fracture 31.05-31.10 m NI 31.10-31.30 m AZCL 31.30-31.42 m NI			(2.55)	
32.60-34.10	100 63 27					Very weak, locally extremely weak, thinly laminated dark grey MUDSTONE. Weathering is localised loss of structure to gravelly clay. Fractures are: 1 No. subhorizontal, very closely to medium spaced, planar, rough to smooth. 2 No. subvertical, planar, smooth.			32.65 +13.30		
34.10-35.60	77 43 37					32.99-33.10 m NI 33.24-33.31 m NI 33.31-33.38 m recovered as gravelly clay 33.38-33.53 m 1 No. vertical rough fracture 34.02-34.10 m NI 34.10-34.45 m AZCL 34.45-34.54 m NI 34.77-34.79 m NI					
35.60-37.10	93 73 23	NI 80 240				35.10-35.60 m 1 No. 75deg. incipient fracture 35.35-35.42 m NI 35.60-35.70 m AZCL 35.84-35.90 m NI 36.00-36.06 m NI 36.55-36.59 m NI 36.76-36.84 m NI			(7.45)		
37.10-38.60	100 43 17					37.24-37.28 m NI 38.05-38.39 m NI 38.50-38.60 m NI 38.65-38.69 m NI					
38.60-40.10	100 64 20					39.06-39.11 m NI 39.63-39.70 m NI					
10/10/2011											
Depth	TCR SCR RGD	If	Records/Samples	Date Casing	Time Water	Stratum continues to 40.10 m					
Groundwater Entries						Depth Related Remarks *			Chiselling		
No.	Struck (m)	Post strike behaviour	Depth sealed (m)			From	to (m)		Depths (m)	Time	Tools used
Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.						Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places			Borehole BH3 Sheet 4 of 5		
Scale 1:50 (c) ESG www.esg.co.uk 426.4813/12/2011 15:24:44											

Borehole Log



Soil Mechanics

Drilled DC/CL Logged CH Checked PH		Start 06/10/2011 End 10/10/2011		Equipment, Methods and Remarks Dando 3000 & Beretta T51. Cable percussion boring followed by rotary core drilling (PWF size) using air mist flush. SPT: hammer ID DC1, rods BW.		Depth from 0.00m to 21.10m Diameter 200mm to 121mm Casing Depth 21.10m		Ground Level +45.95 mOD Coordinates E 486620.13 National Grid N 512295.06 Chainage		
Samples and Tests					Strata					
Depth	TCR SCR RGD	If	Records/Samples	Date Casing	Time Water	Description (Continued from Sheet 4)		Depth, Level/ (Thickness)	Legend	Backfill/ Instruments
				21.10		Very weak, locally extremely weak, thinly laminated dark grey MUDSTONE. Weathering is localised loss of structure to gravelly clay. Fractures are: 1 No. subhorizontal, very closely to medium spaced, planar, rough to smooth. 2 No. subvertical, planar, smooth.		40.10 +5.85		
						EXPLORATORY HOLE ENDS AT 40.10 m				
Depth	TCR SCR RGD	If	Records/Samples	Date Casing	Time Water					
Groundwater Entries					Depth Related Remarks *			Chiselling		
No.	Struck (m)	Post strike behaviour		Depth sealed (m)	From to (m)			Depths (m)	Time	Tools used
Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.					Project SANDSEND SLOPE STABILISATION			Borehole		
Scale 1:50					Project No. A1077-11			BH3		
(c) ESG www.esg.co.uk 426.4813/12/2011 15:24:46					Carried out for Balfour Beatty Living Places			Sheet 5 of 5		

Borehole Log



Soil Mechanics

Drilled JB/PS Logged CH Checked PH	Start 04/10/2011 End 06/10/2011	Equipment, Methods and Remarks Dando 2000 and Geotech 6. Cable percussion boring followed by rotary core drilling (PWF size) using mud flush. SPT: hammer ID JB15, rods NWY.		Depth from 0.00m to 5.00m Diameter 150mm Casing Depth 5.00m	Ground Level +14.59 mOD Coordinates E 486819.85 National Grid N 512318.46 Chainage			
Samples and Tests			Strata					
Depth	Type & No	Records	Date Casing	Time Water	Description	Depth, Level/ (Thickness)	Legend	Backfill/ Instruments
0.10	D 1	0.00-1.20 m Hand excavated inspection pit.			MACADAM. (MADE GROUND) (Foreman's description)	0.10 +14.49		
0.10-0.15	B 2					0.15 +14.44		
0.20	D 3					(0.50)		
0.20-0.30	B 4				Dark grey sandy clayey GRAVEL. Gravel is angular to subangular fine to medium of ash, macadam and sandstone. Slight hydrocarbon odour. (MADE GROUND)	0.65 +13.94		
0.35	D 5							
0.35-0.65	B 6							
0.70	D 7							
0.80-1.10	B 8							
1.00	ES 8A							
1.20	D 9							
1.20-1.65	U NR	70 blows No recovery	1.20	dry	Creamish yellow, becoming light grey, very sandy silty angular to subangular fine to medium GRAVEL of limestone. (MADE GROUND)			
1.70	D 11							
2.00-2.45	SPT S	N=4 (1,1/1,1,1,1)	2.00	dry	Firm orangish brown slightly sandy slightly gravelly CLAY. Gravel is angular to subrounded fine to medium of various lithologies including coal, ash and clinker. (MADE GROUND)	(2.65)		
2.00	D 12							
2.00-2.45	D 13							
2.00-2.50	B 14							
2.00	ES 14A							
3.00	D 15							
3.00-3.45	U 16	35 blows						
3.50	D 17				Orangish dark grey sandy slightly gravelly CLAY. Gravel is angular to subangular fine to coarse of mudstone and sandstone. (Weathered MUDSTONE)	3.30 +11.29		
4.00-4.45	SPT S	N=9 (1,2/2,2,3,2)	4.00	dry		(1.20)		
4.00	D 18							
4.00-4.45	D 19							
4.00-4.50	B 20							
4.50	D 21							
4.50-4.70	B 22							
4.70-5.00	B 23				Dark grey thinly laminated MUDSTONE. Recovered as angular to subangular fine to coarse gravel.	4.50 +10.09		
5.00-5.40	SPT S	50 (7,10/10,13,16,11 for 29mm)	04/10/2011	4.50	dry		(0.50)	
5.00-5.40	D 24		05/10/2011	0800	Extremely weak thinly laminated grey MUDSTONE. Recovered as gravelly clay.	5.00 +9.59		
	NA					(0.75)		
	NA							
	NA							
5.00-8.07	93	Flush: 5.00-8.07 mud, 95 %			Extremely weak to very weak thinly laminated grey MUDSTONE. Weathering is an orangish brown discolouration on fracture surfaces and localised loss of structure to gravelly clay. Fractures are very closely to medium spaced, planar, smooth.	5.75 +8.84		
	44							
	44							
8.07-10.69	NI	Flush: 8.07-10.69 mud, 90 %			7.17-7.41 m 1no. 75 degree, undulating, smooth fracture 7.22-7.30 m NI 7.55-8.15 m recovered as gravelly clay 8.44-8.60 m 1no. 65 degree, planar, smooth fracture 8.95-9.25 m 1no. vertical, planar, smooth fracture	(6.25)		
	100							
	62							
	42							
Depth	ICR ROD	If	Records/Samples	Date Casing	Time Water	Stratum continues to 12.00 m		
Groundwater Entries No. Struck Post strike behaviour None observed (see Key Sheet)						Depth Related Remarks * From to (m)		Chiselling Depths (m) 4.70 -5.00 Time 60 mins Tools used
Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.						Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places		Borehole BH4 Sheet 1 of 2



Borehole Log



Soil Mechanics

Drilled JB/PS Logged CH Checked PH		Start 04/10/2011 End 06/10/2011		Equipment, Methods and Remarks Dando 2000 and Geotech 6. Cable percussion boring followed by rotary core drilling (PWF size) using mud flush. SPT: hammer ID JB15, rods NWY.		Depth from 0.00m to 5.00m 5.00m to 12.00m		Diameter 150mm 146mm		Casing Depth 5.00m 5.30m		Ground Level +14.59 mOD Coordinates E 486819.85 National Grid N 512318.46 Chainage							
Samples and Tests						Strata													
Depth		TCR SCR RGD		If		Records/Samples		Date Casing		Time Water		Description (Continued from Sheet 1)		Depth, Level/ (Thickness)		Legend		Backfill/ Instruments	
10.69-12.00		100 36 0				Flush: 10.69-12.00 mud, 85 %		05/10/2011 5.30		dry		Extremely weak to very weak thinly laminated grey MUDSTONE. Weathering is an orangish brown discolouration on fracture surfaces and localised loss of structure to gravelly clay. Fractures are very closely to medium spaced, planar, smooth.		12.00 +2.59					
												10.00-10.17 m 1no. 65 degree, planar, smooth fracture 10.30-10.52 m 1no. vertical, planar, rough fracture 10.42-10.52 m NI 10.64-10.80 m recovered as gravelly clay 11.04-11.06 m NI 11.27-11.38 m recovered as gravelly clay 11.38-11.57 m 1no. 75 degree, planar, smooth fracture 11.87-12.00 m 1no. 75 degree, planar, smooth fracture							
EXPLOATORY HOLE ENDS AT 12.00 m																			
Depth		TCR SCR RGD		If		Records/Samples		Date Casing		Time Water									
Groundwater Entries						Depth Related Remarks *						Chiselling							
No. Struck Post strike behaviour						From to (m)						Depths (m) Time Tools used							
None observed (see Key Sheet)																			
Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.						Project SANDSEND SLOPE STABILISATION						Borehole							
Scale 1:50						Project No. A1077-11						BH4							
(c) ESG www.esg.co.uk 426.4813/12/2011 15:24:52						Carried out for Balfour Beatty Living Places						Sheet 2 of 2							

Φ

Drilled JB/PS Logged CH Checked PH		Start 05/10/2011 End 07/10/2011		Equipment, Methods and Remarks Dando 2000 and Geotech 6. Cable percussion boring followed by rotary core drilling (PWF size) using mud flush. SPT: hammer ID JB15, rods NWY.		Depth from 0.00m to 4.60m Diameter 150mm Casing Depth 4.60m		Ground Level +12.66 mOD Coordinates E 486705.88 National Grid N 512374.44 Chainage	
Samples and Tests						Strata			
Depth	Type & No	Records	Date Casing	Time Water	Description	Depth, Level (Thickness)	Legend	Backfill/Instrument	
0.10 0.10-0.20 0.30 0.40-0.80	D 1 B 2 D 3 B 4	0.00-1.20 m Hand excavated inspection pit.			MACADAM (Foreman's description) (MADE GROUND)	0.10 +12.56 0.20 +12.46			
					Black very sandy silty angular to subangular fine to medium GRAVEL of ash and macadam. (MADE GROUND)	(0.60)			
0.90 0.90-1.10 1.20-1.65 1.20 1.20-1.65 1.20-1.70	D 5 B 6 SPT S D 7 D 8 B 9	N=2 (1,1/0,1,1,0)		dry	Light grey very sandy angular to subangular fine to medium GRAVEL of limestone. (MADE GROUND)	0.80 +11.86			
2.00-2.45 2.00 2.00-2.45 2.00-2.50	SPT S D 10 D 11 B 12	N=5 (1,1/1,1,2,1)	2.00	dry	Firm greyish brown slightly sandy slightly gravelly CLAY. Gravel is angular to subangular fine to medium of various lithologies including ash, coal and sandstone. (MADE GROUND)	(2.30)			
3.00 3.00-3.45 3.50 3.50-3.90	D 13 U 14 D 15 B 16	75 blows 350 mm rec	3.00	dry	Very stiff orangish grey slightly sandy slightly gravelly CLAY. Gravel is angular to subangular fine to medium of mudstone. (Weathered MUDSTONE)	3.10 +9.56			
4.00-4.44 4.00 4.00-4.35 4.00-4.60	SPT S D 17 D 18 B 19	50 (8,11/12,17,21,- for 68mm)	3.00	dry		(1.50)			
4.60-4.98 4.60-4.98	SPT S D 20	50 (8,13/15,16,18,1 for 3mm)	05/10/2011 3.00 07/10/2011 3.00	dry dry 0800 dry	Extremely weak dark grey thinly laminated MUDSTONE. Fractures are very closely spaced, randomly orientated, locally stained orange, planar, smooth.	4.60 +8.06			
4.60-7.64	98 8 3 NI NI 10	Flush: 4.60-7.80 mud flush, 100 %							
7.64-8.10	100 0 0	Flush: 7.80-8.10 mud flush, 0 %							
8.10-9.38	100 15 10								
9.92-9.94 m NI									
Depth	ICR SCR RGO	If	Records/Samples	Date Casing	Time Water	Stratum continues to 12.00 m			
Groundwater Entries No. Struck (m) Post strike behaviour None observed (see Key Sheet)						Depth Related Remarks * From to (m)			
Groundwater Entries No. Struck (m) Post strike behaviour None observed (see Key Sheet)						Chiselling Depths (m) 4.00 -4.60 Time 60 mins Tools used			
Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column. Scale 1:50 (c) ESG www.esg.co.uk 426.4813/12/2011 15:24:54						Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places			
Borehole BH5 Sheet 1 of 2									

Φ

Drilled	JB/PS	Start	05/10/2011	Equipment, Methods and Remarks	Dando 2000 and Geotech 6. Cable percussion boring followed by rotary core drilling (PWF size) using mud flush.	Depth from	to	Diameter	Casing Depth	Ground Level	+12.66 mOD	
Logged	CH	End	07/10/2011	SPT: hammer ID JB15, rods NWY.		0.00m 4.60m	4.60m 12.00m	150mm 146mm	4.60m 4.60m	Coordinates	E 486705.88	
Checked	PH									National Grid	N 512374.44	
Samples and Tests									Strata			Chainage
Depth	T/C SCR RQD	If	Records/Samples	Date Casing	Time Water	Description <small>(Continued from Sheet 1)</small>	Depth, Level/ (Thickness)	Legend	Backfill/ Instrument			
9.38-12.00	100 95 32	NI 40 150	Flush: 8.10-12.00 mud flush, 100 %	07/10/2011 4.60	4.00	Extremely weak dark grey thinly laminated MUDSTONE. Fractures are very closely spaced, randomly orientated, locally stained orange, planar, smooth. <div style="text-align: right;">11.53-11.55 m NI ■ 11.90-12.00 m □</div>	12.00 +0.66	[Pattern]	[Diagram] SPIE			
						EXPLORATORY HOLE ENDS AT 12.00 m						
Depth	T/C SCR RQD	If	Records/Samples	Date Casing	Time Water							
Groundwater Entries				Depth Related Remarks *			Chiselling					
No.	Struck (m)	Post strike behaviour	Depth sealed (m)	From to (m)			Depths (m)	Time	Tools used			
None observed (see Key Sheet)												
Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.						Project		Borehole				
(c) ESG www.esg.co.uk 426.4813\12\2011 15:24:56 AGS						A1077-11		BH5				
Scale 1:50						SANDESEND SLOPE STABILISATION		Sheet 2 of 2				
						Carried out for						
						Balfour Beatty Living Places						

Borehole Log



Soil Mechanics

Drilled JB/PS Logged CH/JMH Checked PH		Start 03/10/2011 End 06/10/2011	Equipment, Methods and Remarks Dando 2000 and Geotech 6. Cable percussion boring followed by rotary core drilling (PWF size) using mud flush. SPT: hammer ID JB15, rods NWY.		Depth from 0.00m to 5.00m Diameter 150mm Casing Depth 5.00m	Ground Level +13.02 mOD Coordinates E 486739.83 National Grid N 512358.09 Chainage		
Samples and Tests			Strata					
Depth	Type & No	Records	Date Casing	Time Water	Description	Depth, Level/ (Thickness)	Legend	Backfill/ Instruments
0.10 0.10-0.25 0.30 0.40-0.70	D 1 B 2 D 3 B 4	0.00-1.20 m Hand excavated inspection pit.			MACADAM (Foreman's description) (MADE GROUND)	0.20 +12.82 0.25 +12.77 (0.45)		
0.75 0.80-1.10 1.00	D 5 B 6 ES 6A		03/10/2011	dry	Black very sandy angular to subrounded fine to medium GRAVEL of ash, macadam and sandstone. (MADE GROUND)	0.70 +12.32		
1.20-1.58 1.20 1.20-1.65 1.20-1.70	SPT S D 7 D 8 B 9	N=3 (1,0/1,1,0,1)	04/10/2011	0800 dry	Creamish grey very sandy silty angular to subangular fine to medium GRAVEL of limestone. (MADE GROUND)			
2.00-2.45 2.00 2.00 2.00-2.45 2.00-2.50	SPT S D 10 ES 10A D 11 B 12	N=4 (1,1/1,1,1,1)	2.00	dry	Soft to firm brownish orangish slightly sandy slightly gravelly CLAY. Gravel is angular to subangular fine to medium of coal and burnt shale. (MADE GROUND)	(2.70)		
3.00 3.00-3.45	D 13 U 14	80 blows						
3.50 3.50-3.90	D 15 B 16				Firm greyish brown slightly sandy slightly gravelly CLAY. Sand is fine to coarse. Gravel is subangular to subrounded fine to medium of various lithologies. (GLACIAL TILL)	3.40 +9.62 3.50 +9.52 (0.50)		
4.00-4.38 4.00 4.00-4.39 4.00-4.50	SPT S D 17 D 18 B 19	50 (7,11/14,17,19)	4.00	dry	Dark grey very gravelly CLAY. Gravel is angular to subangular fine to coarse of mudstone. (Weathered MUDSTONE)	4.00 +9.02		
5.00-5.36 5.00-5.36	SPT S D 20	50 (8,14/17,17,16 for 55mm)	04/10/2011 4.50 06/10/2011 4.50	dry dry	Very weak grey thinly laminated MUDSTONE. Recovered as angular to subangular fine to medium gravel.			
5.00-6.17	74 0 0	Flush: 5.00-6.17 mud, 100 %			Extremely weak thinly laminated grey MUDSTONE. Weathering is localised loss of structure to gravelly clay. Fractures are subhorizontal, extremely to very closely spaced, planar, rough.	5.36 +7.66		
6.17-9.14	NI 10 30					(2.19)		
	100 53 29				Very weak locally extremely weak thinly laminated grey MUDSTONE. Weathering is localised loss of structure to gravelly clay. Fractures are subhorizontal, very closely to medium spaced, planar, smooth.	7.55 +5.47		
	NI 70 450	Flush: 6.17-12.14 mud, 95 %				(4.59)		
Depth	ICR ROD	If	Records/Samples	Date Casing	Time Water	Stratum continues to 12.14 m		
Groundwater Entries No. Struck Post strike behaviour None observed (see Key Sheet)						Depth Related Remarks * From to (m)		Chiselling Depths (m) 4.00 -5.00 Time 60 mins Tools used
Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.						Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places		Borehole BH6 Sheet 1 of 2

Borehole Log



Soil Mechanics

Drilled JB/PS Logged CH/JMH Checked PH		Start 03/10/2011 End 06/10/2011		Equipment, Methods and Remarks Dando 2000 and Geotech 6. Cable percussion boring followed by rotary core drilling (PWF size) using mud flush. SPT: hammer ID JB15, rods NWY.		Depth from 0.00m to 5.00m Diameter 150mm to 146mm Casing Depth 5.00m to 5.00m		Ground Level +13.02 mOD Coordinates E 486739.83 National Grid N 512358.09 Chainage			
Samples and Tests						Strata					
Depth		TCR SCR RGD	If	Records/Samples	Date Casing	Time Water	Description (Continued from Sheet 1)		Depth, Level/ (Thickness)	Legend	Backfill/ Instruments
9.14-12.14		100 40 24			06/10/2011 5.00	5.02	Very weak locally extremely weak thinly laminated grey MUDSTONE. Weathering is localised loss of structure to gravelly clay. Fractures are subhorizontal, very closely to medium spaced, planar, smooth. :: 9.76m - gravelly clay 10.11-10.19 m recovered as gravelly clay 10.47-10.67 m weak thinly laminated grey SILTSTONE band 11.84-11.96 m 1no. vertical, planar, smooth fracture 12.08-12.14 m NI		12.14 +0.88		
EXPLORARY HOLE ENDS AT 12.14 m											
Depth		TCR SCR RGD	If	Records/Samples	Date Casing	Time Water					
Groundwater Entries No. Struck Post strike behaviour None observed (see Key Sheet)						Depth sealed (m)		Depth Related Remarks * From to (m)		Chiselling Depths (m) Time Tools used	
Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.						Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places		Borehole BH6 Sheet 2 of 2			



Dynamic Sampler Hole Log



Soil Mechanics

Drilled CS Logged CH Checked PH		Start 11/10/2011 End 11/10/2011		Equipment, Methods and Remarks Dando Terrier 2002. Windowless sampling.			Depth from 0.00m to 1.30m Diameter 80mm Casing Depth		Ground Level +4.43 mOD Coordinates E 487176.62 National Grid N 512177.53 Chainage		
Samples and Tests					Strata						
Depth	Type & No	Records	Date Casing	Time Water	Description	Depth, Level/ (Thickness)	Legend	Backfill/ Instruments			
0.30 0.30-1.00	D 1 B 2	0.00-1.20 m Hand excavated inspection pit.			Light brown medium to coarse SAND. (BEACH DEPOSITS)	(1.30)					
1.30	D 3		11/10/2011	dry	EXPLORATORY HOLE ENDS AT 1.30 m	1.30 +3.13					
					1.30 m 1 No. rounded sandstone cobble						
Depth	Type & No	Records	Date Casing	Time Water							
Groundwater Entries No. Struck Post strike behaviour None observed (see Key Sheet)					Depth sealed (m) Depth Related Remarks * From 1.30 to (m) Hole terminated due to refusal.		Chiselling Depths (m) Time Tools used				
Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.					Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places		Borehole WS1 Sheet 1 of 1				

Dynamic Sampler Hole Log



Soil Mechanics

Drilled CS Logged CH Checked PH	Start 11/10/2011 End 11/10/2011	Equipment, Methods and Remarks Dando Terrier 2002. Windowless sampling.		Depth from 0.00m 1.30m 2.30m 4.30m	to 1.30m 2.30m 4.30m 5.00m	Diameter 80mm 70mm 40mm 20mm	Casing Depth	Ground Level +3.86 mOD Coordinates E 487089.81 National Grid N 512215.23 Chainage
Samples and Tests				Strata				
Depth	Type & No	Records	Date Casing	Time Water	Description	Depth, Level/ (Thickness)	Legend	Backfill/ Instruments
0.30 0.30-1.00 0.60-2.10	D 1 B 2 B 5				Light brown slightly gravelly medium SAND. Gravel is fine to medium, occasionally coarse, of sandstone. (BEACH DEPOSITS)	(1.60)		
1.20	D 3				1.00-1.60 m increase in gravel content	1.60 +2.26		
1.50	D 4							
2.20	D 6				Stiff dark grey and brown slightly sandy slightly gravelly CLAY. Gravel is subangular to rounded fine to medium of various lithologies. (GLACIAL TILL)	(1.70)		
2.80	D 7				1.95-2.10 m band of firm dark brown sandy silt			
3.30	D 8				2.80-3.10 m band of firm dark brown sandy silt			
					NO RECOVERY	3.30 +0.56		
						(1.70)		
			11/10/2011					
					EXPLORATORY HOLE ENDS AT 5.00 m	5.00 -1.14		
Depth	Type & No	Records	Date Casing	Time Water				
Groundwater Entries No. Struck Post strike behaviour 1 2.00 -				Depth sealed (m) -	Depth Related Remarks * From to (m) 3.00 Hole collapsing.	Chiselling Depths (m)	Time	Tools used
Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.				Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places	Borehole WS2 Sheet 1 of 1			

Scale 1:50

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Dynamic Sampler Hole Log



Soil Mechanics

Drilled CS Logged CH Checked PH	Start 10/11/2011 End 10/11/2011	Equipment, Methods and Remarks Dando Terrier 2002. Windowless sampling.		Depth from 0.00m 1.30m 2.00m	to 1.30m 2.00m 2.50m	Diameter 80mm 60mm -	Casing Depth	Ground Level +3.98 mOD Coordinates E 486809.97 National Grid N 512352.00 Chainage
Samples and Tests				Strata				
Depth	Type & No	Records	Date Casing	Time Water	Description	Depth, Level/ (Thickness)	Legend	Backfill/ Instruments
0.30 0.30-1.00	D 1 B 2				Light brown gravelly medium to coarse SAND with occasional sand size shell fragments. (BEACH DEPOSITS)	(1.80)		
1.30-1.50	D 3							
1.80-2.00	D 4					1.80 +2.18 (0.40)		
			10/11/2011		Multicoloured sandy subrounded to rounded fine to coarse GRAVEL of sandstone. (BEACH DEPOSITS)	2.20 +1.78 (0.30)		
2.50	D 5				Dark grey and black slightly sandy gravelly CLAY. Gravel is angular fine to medium of mudstone. (Weathered MUDSTONE)	2.50 +1.48		
					EXPLORATORY HOLE ENDS AT 2.50 m			
Depth	Type & No	Records	Date Casing	Time Water				
Groundwater Entries					Depth Related Remarks *		Chiselling	
No.	Struck (m)	Post strike behaviour	Depth sealed (m)		From	to (m)	Depths (m)	Time
					2.50			
None observed (see Key Sheet)					Hole terminated due to refusal.		Tools used	
Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.					Project SANDESEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places		Borehole WS3 Sheet 1 of 1	

Dynamic Sampler Hole Log



Soil Mechanics

Drilled CS Logged CH Checked PH		Start 10/11/2011 End 10/11/2011		Equipment, Methods and Remarks Dando Terrier 2002. Windowless sampling.		Depth from 0.00m 1.00m		to 1.00m 1.90m		Diameter 80mm -		Casing Depth		Ground Level Coordinates National Grid Chainage		+2.69 mOD E 486649.56 N 512432.75					
Samples and Tests						Strata															
Depth		Type & No		Records		Date Casing		Time Water		Description				Depth, Level/ (Thickness)		Legend		Backfill/ Instruments			
0.40 0.40-0.90		D 1 B 2		0.00-1.20 m Hand excavated inspection pit.						Light brown gravelly fine to coarse SAND. Gravel is subrounded to rounded fine to medium of various lithologies including brick. (BEACH DEPOSITS)				(1.00)							
0.90 1.20-1.33		D 3 SPT S		50 (15/50 for 50mm)						Extremely weak dark grey, becoming light brown and orange, thinly laminated MUDSTONE.				1.00 +1.69				1			
1.50		D 4				10/11/2011								(0.90)							
EXPLORATORY HOLE ENDS AT 1.90 m																					
Depth		Type & No		Records		Date Casing		Time Water		Depth Related Remarks *				Chiselling Depths (m)		Time		Tools used			
1		1.00		-						From to (m)											
Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.										Project Project No. Carried out for				SANDSEND SLOPE STABILISATION A1077-11 Balfour Beatty Living Places				Borehole WS4 Sheet 1 of 1			
Scale 1:50										(c) ESG www.esg.co.uk 426.4813/12/2011 15:25:07				AGS							

Dynamic Sampler Hole Log



Soil Mechanics

Drilled CS Logged CH Checked PH	Start 10/11/2011 End 10/11/2011	Equipment, Methods and Remarks Dando Terrier 2002. Windowless sampling.	Depth from 0.00m to 1.30m Diameter 80mm Casing Depth 60mm 2.20m 4.00m	Ground Level +3.72 mOD Coordinates E 486380.62 National Grid N 512549.27 Chainage				
Samples and Tests			Strata					
Depth	Type & No	Records	Date Casing	Time Water	Description	Depth, Level (Thickness)	Legend	Backfill/ Instruments
0.40	D 1				Brown slightly sandy angular to rounded fine GRAVEL of various lithologies. (BEACH DEPOSITS)	(0.60)		
0.60-1.30	B 3					0.60 +3.12		
0.80	D 2				Light brown gravelly medium to coarse SAND. Gravel is subangular to rounded fine to medium of various lithologies. (BEACH DEPOSITS)	(0.95)		
1.70	D 4				Stiff, locally firm, orangish brown slightly sandy slightly gravelly CLAY. (GLACIAL TILL)	1.55 +2.17		
2.20	D 5				1.80-1.90 m band of firm sandy silt			
3.00-3.40	D 6					(2.65)		
			10/11/2011					
EXPLORATORY HOLE ENDS AT 4.20 m						4.20 -0.48		
4.20 m fragments of medium mudstone gravel								
Depth	Type & No	Records	Date Casing	Time Water				
Groundwater Entries No. Struck Post strike behaviour 1 1.30 -			Depth sealed (m) -		Depth Related Remarks * From to (m) 2.80 Hole collapsing.	Chiselling Depths (m) Time Tools used		
Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.			Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places			Borehole WS5 Sheet 1 of 1		

Scale 1:50

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ENCLOSURE B
INSTRUMENTATION

Installation Details

B1

Groundwater Installation Details

Hole No	Instrument ID	Installation Type	Date of Installation	Reference depth (mBGL)	Piezometer Diameter (mm)	Top of response zone (mBGL)	Base of response zone (mBGL)	Tubing Completion Details	Headworks	Remarks
BH2		SPIE	6 Oct 2011	21.00	19	20.50	21.50	None	Stop cock cover	
BH3	A	SPIE	11 Oct 2011	4.70	19	4.00	5.00	None	Stop cock cover	
BH3	B	SPIE	11 Oct 2011	15.70	19	15.00	16.00	None	Stop cock cover	
BH4		SPIE	6 Oct 2011	10.70	19	10.00	11.00	None	Stop cock cover	
BH5	A	SPIE	7 Oct 2011	4.20	19	3.50	4.50	None	Stop cock cover	
BH5	B	SPIE	7 Oct 2011	10.70	19	10.00	11.00	None	Stop cock cover	

Notes: Type: SP - Standpipe, SPIE - Standpipe
Piezometer, HPIE - Hydraulic Piezometer, PPIE -
Pneumatic Piezometer, EPIE - Electronic
Piezometer Prepared: 01/12/2011 17:30



Project SANDSEND SLOPE STABILISATION
Project No. A1077-11
Carried out for Balfour Beatty Living Places

Table

B1

ENCLOSURE C
GEOTECHNICAL LABORATORY TEST RESULTS

Index Properties – Summary of Results	INDX 1 and 2
Particle Size Distribution Analyses	PSD 1 to 50
Unconsolidated Undrained Triaxial Compression Tests – Summary of Results	UUSUM 1
Consolidated Undrained Triaxial Compression	CUM 1 to 10
Shear Strength by Direct Shear (Shearbox)	SSB 1 to 4
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Chemical Tests – Summary of Results	CHEM 1 and 2

INDEX PROPERTIES - SUMMARY OF RESULTS

Project No	Project Name
A1077-11	SANSEND BOREHOLES, NORTH YORKSHIRE

Hole No.	Sample				Soil Description	ρ	ρ_d	W	< 425 μm sieve	W_L	W_P	I_P	ρ_s	Remarks
	No.	Depth (m)		type										
		from	to			Mg/m ³	%	%	%	%	Mg/m ³			
BH1	5	1.20	1.65	U	Firm brown slightly sandy slightly gravelly CLAY.	2.11	1.78	19	76 s	40 a	18	22		
BH1	9	3.00	3.45	U	Firm to stiff greyish brown slightly sandy slightly gravelly CLAY.	2.18	1.89	15	80 s	31 a	15	16		
BH1	13	5.00	5.45	U	Firm to stiff greyish brown slightly sandy slightly gravelly CLAY.	2.26	1.98	14	81 s	31 a	14	17		
BH1	17	6.50	6.95	U	Firm brown slightly sandy slightly gravelly CLAY.	2.14	1.86	15	74 s	33 a	15	18		
BH1	21	8.00	8.45	U	Firm to stiff greyish brown slightly sandy slightly gravelly CLAY.			15	84 s	35 a	16	19		
BH1	25	9.50	9.95	U	Firm to stiff brown slightly sandy slightly gravelly CLAY.	2.24	1.95	15	83 s	32 a	15	17		
BH1	29	11.00	11.45	U	Firm to stiff brown slightly sandy slightly gravelly CLAY.			15	84 s	34 a	15	19		
BH1	33	12.50	12.95	U	Firm brown slightly sandy slightly gravelly CLAY.	2.19	1.91	15	79 s	32 a	15	17		
BH1	37	14.00	14.45	U	Firm greyish brown slightly sandy slightly gravelly CLAY.			23	97 s	37 a	17	20		
BH1	41	15.50	15.95	U	Stiff brown thinly laminated CLAY with thin laminations of sand becoming slightly sandy	2.11	1.77	19	91 s	36 a	17	19		
BH1	45	17.00	17.45	U	Stiff brown slightly sandy slightly gravelly CLAY.			17	94 s	37 a	17	20		
BH1	52	19.50	19.95	U	Stiff greyish brown slightly sandy slightly gravelly CLAY.	2.24	2.00	12	87 s	31 a	15	16		
BH1	56	21.00	21.45	U	Stiff to very stiff brown slightly sandy slightly gravelly CLAY.			13	88 s	31 a	15	16		
BH1	60	22.50	23.95	U	Stiff greyish brown slightly sandy slightly gravelly CLAY.	2.19	1.92	14	88 s	32 a	15	17		
BH2	4	1.20	1.65	U	Firm to stiff greyish brown and brownish grey slightly sandy slightly gravelly CLAY.	2.07	1.71	21	91 s	53 a	21	32		
BH2	8	3.00	3.45	U	Stiff greyish brown slightly sandy slightly gravelly CLAY.	2.16	1.89	14	82 s	34 a	16	18		
BH2	12	5.00	5.45	U	Brown slightly sandy slightly gravelly CLAY.			14	87 s	31 a	15	16		
BH2	16	7.00	7.45	U	Stiff brown slightly sandy slightly gravelly CLAY.			15	88 s	30 a	15	15		
BH2	20	9.00	9.45	U	Firm to stiff brown slightly sandy slightly gravelly CLAY.			13	80 s	28 a	14	14		
BH2	24	11.00	11.45	U	Firm to stiff brown slightly sandy slightly gravelly CLAY.	2.22	1.93	15	86 s	33 a	15	18		
BH2	31	15.00	15.45	U	Firm to stiff brown slightly sandy slightly gravelly CLAY.			19	99 n	49 a	23	26		
BH2	37	17.00	17.45	U	Firm to stiff greyish brown slightly sandy slightly gravelly CLAY.	2.14	1.83	17	86 s	35 a	16	19		
BH2	43	19.00	19.45	U	Firm brown slightly sandy slightly gravelly CLAY.			15	79 s	31 a	16	15		
BH2	47	21.00	21.45	U	Stiff greyish brown slightly sandy slightly gravelly CLAY.			14	81 s	31 a	15	16		
BH2	51	23.00	23.45	U	Firm brown slightly sandy slightly gravelly CLAY.	2.20	1.93	14	85 s	32 a	15	17		
BH2	55	25.00	25.45	U	Firm to stiff greyish brown and grey slightly sandy slightly gravelly CLAY.			16	88 s	35 a	16	19		
BH3	4	1.20	1.65	U	Firm to stiff greyish brown and grey slightly sandy slightly gravelly CLAY.	2.04	1.68	21	99 n	53 a	22	31		
BH3	8	2.50	2.95	U	Firm to very stiff brown slightly sandy slightly gravelly CLAY.	2.15	1.86	16	87 s	37 a	18	19		
BH3	16	5.50	5.95	U	Firm brown slightly sandy slightly gravelly CLAY.	2.17	1.85	17	70 s	32 a	15	17		
BH3	24	8.50	8.95	U	Firm greyish brown slightly sandy slightly gravelly CLAY.			17	86 s	34 a	15	19		
BH3	28	10.00	10.45	U	Firm to stiff greyish brown slightly sandy slightly gravelly CLAY.			14	69 s	33 a	16	17		
BH3	33	11.50	11.95	D	Grey slightly sandy CLAY.				100 n	31 a	17	14		


General notes:	All above tests carried out to BS1377 : 1990 definitive method in all cases unless annotated otherwise. See individual test reports for further details.												
Key :	ρ	bulk density, linear	W_L	Liquid limit	W_P	Plastic limit	<425um preparation	ρ_s	particle density				
	ρ_d	dry density	a	4 point cone test	NP	non - plastic	n	from natural soil	-g	gas jar			
	w	moisture content	b	1 point cone test	I_p	Plasticity Index	s	sieved specimen	-p	small pyknometer			

QA Ref						Printed:01/12/2011 09:37				Table
SLR 1 Rev 88 Aug 11										INDX 1

INDEX PROPERTIES - SUMMARY OF RESULTS

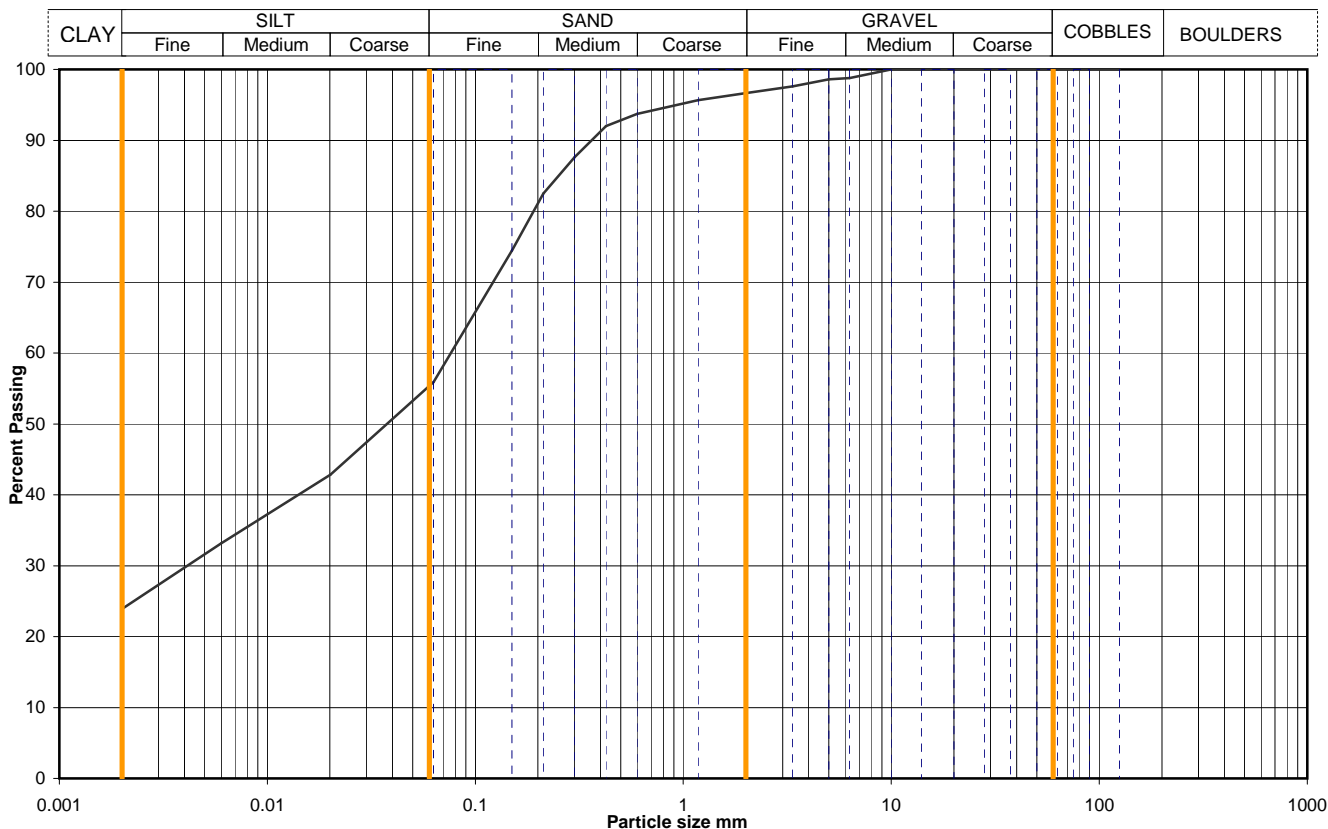
Project No	Project Name
A1077-11	SANDESEND BOREHOLES, NORTH YORKSHIRE

Hole No.	Sample				Soil Description	ρ	ρ_d	W	< 425 μm sieve	W_L	W_P	I_P	ρ_s	Remarks
	No.	Depth (m)		type										
		from	to			Mg/m^3	%	%	%	%	Mg/m^3			
BH3	37	13.50	13.95	D	Brownish grey slightly sandy slightly gravelly CLAY.				91 s	35 a	16	19		
BH3	39	14.50	14.95	U	Firm to stiff greyish brown slightly sandy slightly gravelly CLAY.	2.21	1.94	14	87 s	29 a	14	15		
BH3	47	17.50	17.95	U	Stiff greyish brown slightly sandy slightly gravelly CLAY.			12	85 s	32 a	13	19		
BH4	13	2.00	2.45	D	Brown slightly sandy slightly gravelly CLAY.			19	71 s	36 b	17	19		
BH4	16	3.00	3.45	U	Firm greyish brown slightly sandy slightly gravelly CLAY.	1.96	1.59	23	79 s	46 a	21	25		
BH4	19	4.00	4.45	D	Greyish brown slightly sandy slightly gravelly CLAY.			17	72 s	37 b	18	19		
BH4	21	4.50		D	Dark grey slightly gravelly CLAY. Gravel is mainly weak mudstone.			15	98 n	47 a	22	25		
BH4	24	5.00	5.40	D	Grey slightly sandy slightly gravelly silty CLAY.			14	98 s	44 a	25	19		
BH5	7	1.20		D	Dark brownish grey slightly gravelly sandy CLAY.				42 s	28 a	18	10		
BH5	8	1.20	1.65	D	Grey very gravelly slightly clayey SAND.			15						
BH5	11	2.00	2.45	D	Grey slightly sandy slightly gravelly CLAY.			17	74 s	35 a	19	16		
BH5	14	3.00	3.45	U	Stiff greyish brown slightly sandy slightly gravelly CLAY.	1.87	1.53	22	82 s	44 a	22	22		
BH5	18	4.00	4.35	D	Brownish grey slightly gravelly CLAY.			10	78 s	36 a	22	14		
BH6	8	1.20	1.65	D	Greyish brown slightly gravelly sandy CLAY.			17	60 s	26 b	16	10		
BH6	10	2.00		D	Greyish brown slightly sandy gravelly CLAY.				38 s	28 a	18	10		
BH6	11	2.00	2.45	D	Brown slightly sandy slightly gravelly CLAY.			21						
BH6	14	3.00	3.45	U	Firm brownish grey slightly sandy slightly gravelly CLAY.	2.08	1.72	21	90 s	49 a	26	23		
BH6	18	4.00	4.39	D	Grey slightly gravelly silty CLAY.			10	80 s	37 a	22	15		
WS1	3	1.30		D	Brown very sandy COBBLE.			2.7	29 s	25 b	NP			
WS2	2	0.30	1.00	B	Brownish grey gravelly SAND.			4.4	64 n	23 b	NP			
WS2	7	2.80		D	Grey sandy clayey SILT.			18	100 n	26 a	17	9		
WS3	2	0.30	1.00	B	Brown gravelly SAND.			11	62 s	24 b	NP			
WS3	4	1.80	2.00	D	Brownish grey sandy GRAVEL.			3.8						
WS3	5	2.50	2.50	D	Brownish grey slightly sandy gravelly CLAY.			13	30 s	39 b	22	17		
WS4	2	0.40	0.90	B	Grey and brown very gravelly SAND.			13	32 s	22 b	NP			
WS4	4	1.50		D	Dark grey slightly sandy silty CLAY.			16	100 n	39 a	25	14		
WS5	2	0.80		D	Brown gravelly SAND.			14	27 s	24 b	NP			
WS5	6	3.00	3.40	D	Brown slightly sandy slightly gravelly CLAY.			18	95 s	34 a	16	18		

General notes: All above tests carried out to BS1377 : 1990 definitive method in all cases unless annotated otherwise. See individual test reports for further details.									
Key :	ρ	bulk density, linear	W_L	Liquid limit	W_P	Plastic limit	<425um preparation	ρ_s	particle density
	ρ_d	dry density	a	4 point cone test	NP	non - plastic	n from natural soil	-g	gas jar
	w	moisture content	b	1 point cone test	I_P	Plasticity Index	s sieved specimen	-p	small pycnometer
QA Ref						Printed:01/12/2011 09:37		Table	INDX 2
SLR 1 Rev 88 Aug 11									

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH1		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		0.50		
			Samp No	4	Type	B	
			ID	ESGA1077-11201110100000000004			
			Spec Ref				



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	43
90	100	0.0060	33
75	100	0.0020	24
63	100		
50	100		
37.5	100		
28	100		
20	100		
14	100		
10	100		
6.3	99		
5.0	99		
3.35	98		
2.00	97		
1.18	96		
0.600	94	Particle density, Mg/m ³ 2.65 assumed	
0.425	92		
0.300	88	Dry mass of sample, kg 13.5	
0.212	82		
0.150	74		
0.063	56		

Soil description	Dark brown slightly gravelly sandy CLAY with occasional rootlets.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	3	3
	Silt	41	41
	Clay	32	32
		24	24

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref
SLR 2,9
Rev 88
Aug 11

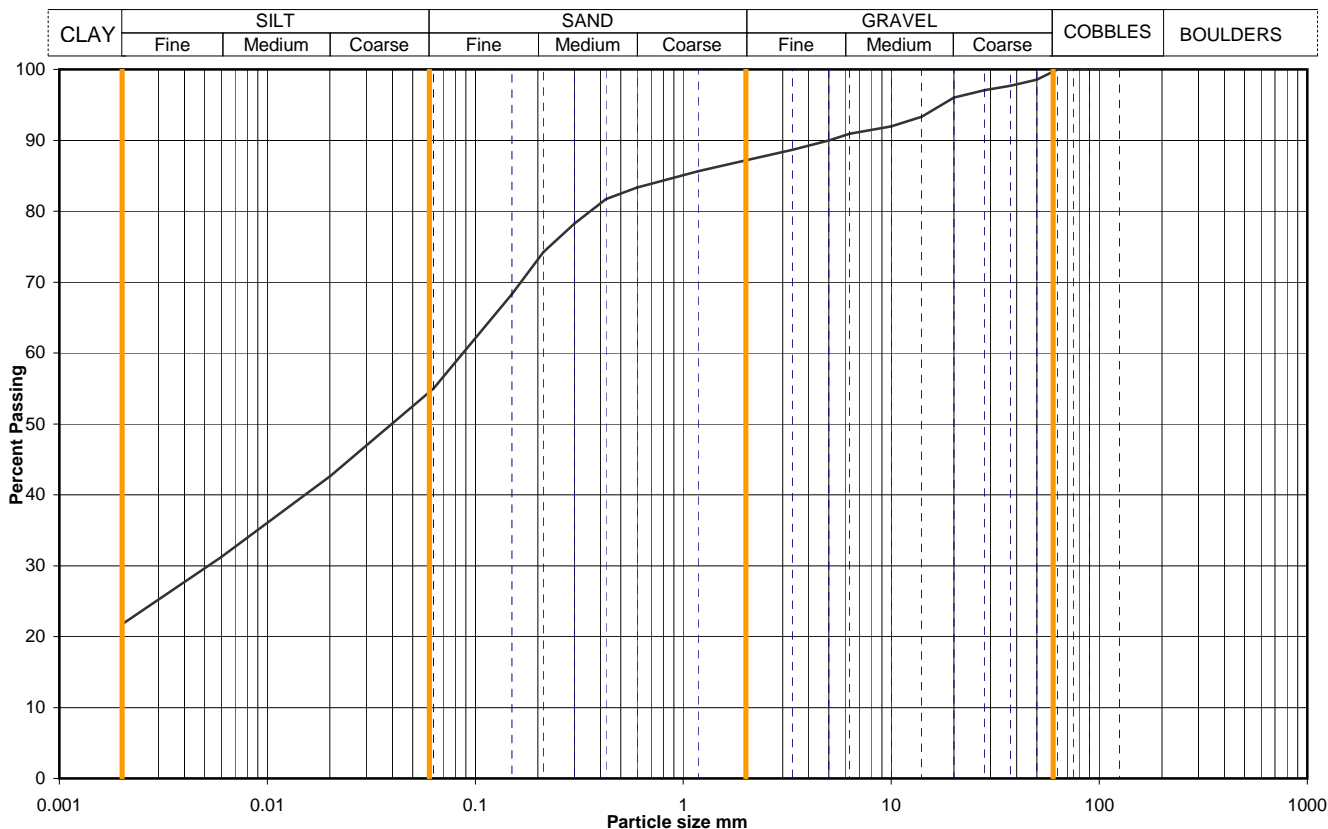


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Figure
PSD 1

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH1		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		2.00		
			Samp No	8	Type	B	
			ID	ESGA1077-11201110100000000009			
			Spec Ref				



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	43
90	100	0.0060	31
75	100	0.0020	22
63	100		
50	99		
37.5	98		
28	97		
20	96		
14	93		
10	92		
6.3	91		
5.0	90		
3.35	89		
2.00	87		
1.18	86		
0.600	83	Particle density, Mg/m ³ 2.65 assumed	
0.425	82		
0.300	78	Dry mass of sample, kg 12.5	
0.212	74		
0.150	68		
0.063	55		

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	13	13
	Silt	33	33
	Clay	32	32

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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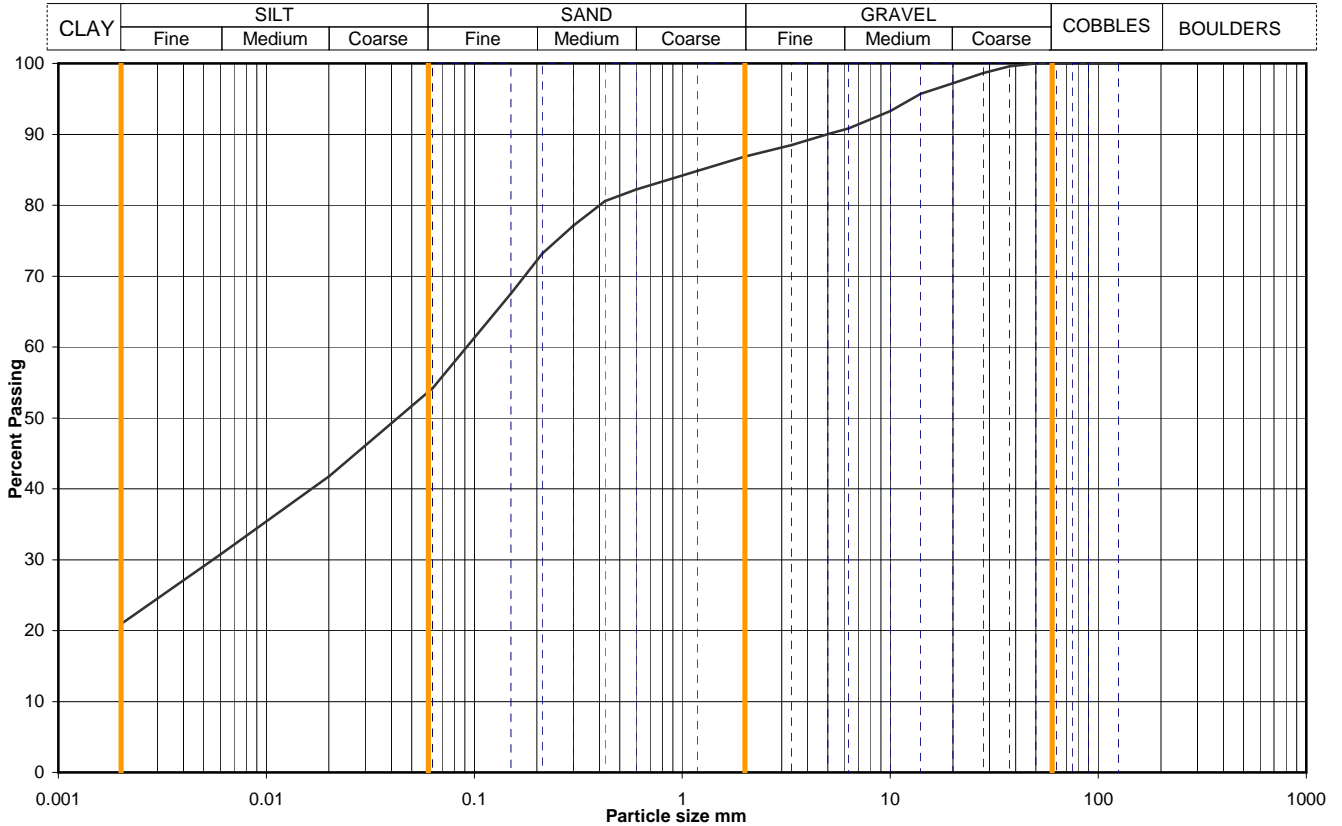
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Figure

PSD 2

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH1	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		4.00	
			Samp No	12	Type	B
			ID	ESGA1077-11201110100000000013		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	42
90	100	0.0060	31
75	100	0.0020	21
63	100		
50	100		
37.5	100		
28	99		
20	97		
14	96		
10	93		
6.3	91		
5.0	90		
3.35	88		
2.00	87		
1.18	85		
0.600	82	Particle density, Mg/m3 2.65 assumed	
0.425	81		
0.300	77	Dry mass of sample, kg 16.9	
0.212	73		
0.150	68		
0.063	54		

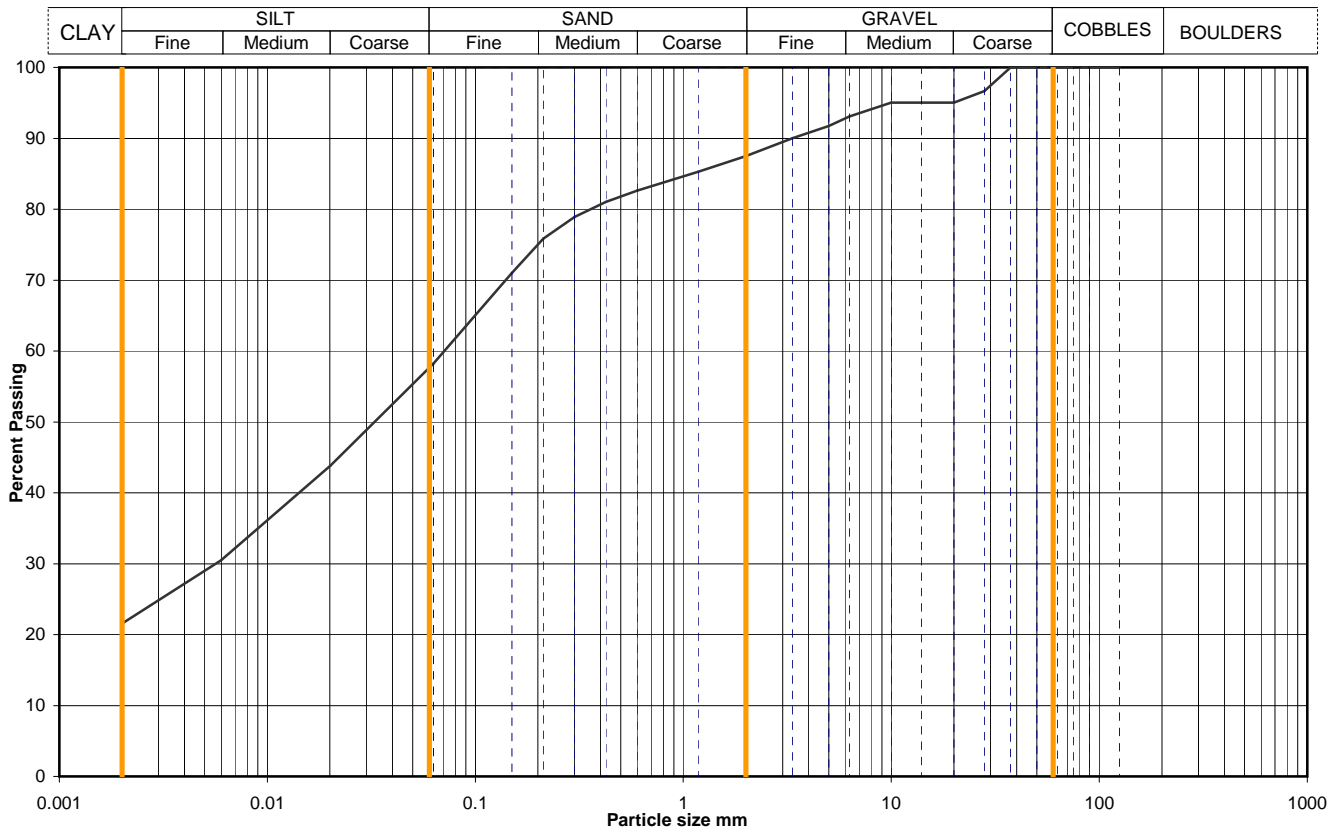
Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	13	13
	Silt	33	33
	Clay	33	33

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH1	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		9.95	
			Samp No	26	Type	D
			ID	ESGA1077-11201110100000000027		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	44
90	100	0.0060	30
75	100	0.0020	22
63	100		
50	100		
37.5	100		
28	97		
20	95		
14	95		
10	95		
6.3	93		
5.0	92		
3.35	90		
2.00	87		
1.18	85		
0.600	83	Particle density, Mg/m3	
0.425	81	2.65 assumed	
0.300	79	Dry mass of sample, kg	
0.212	76	1.7	
0.150	71		
0.063	58		

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	13	13
	Silt	30	30
	Clay	36	36
		21	21

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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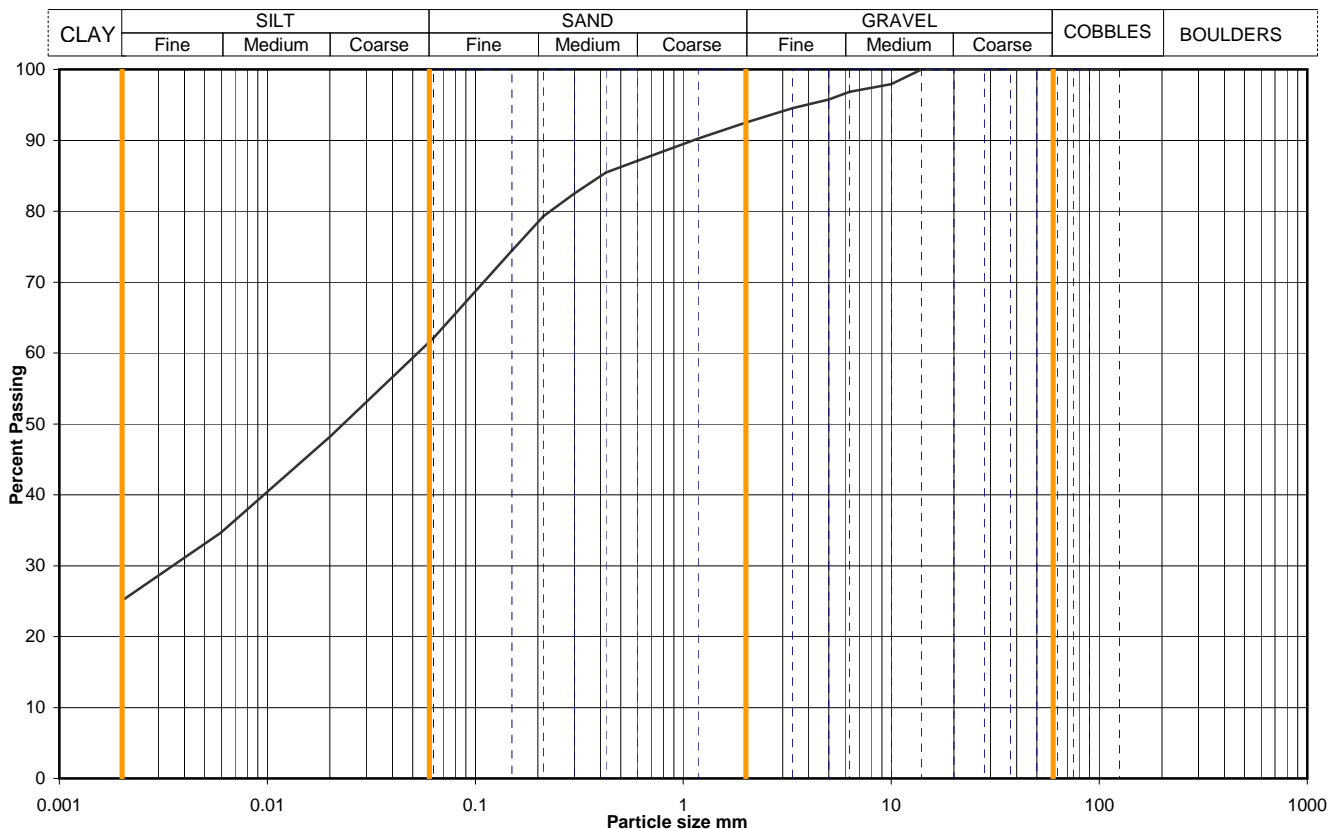


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Figure
PSD 4

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH1	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		13.15	
			Samp No	35	Type	D
			ID	ESGA1077-11201110100000000036		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	48
90	100	0.0060	35
75	100	0.0020	25
63	100		
50	100		
37.5	100		
28	100		
20	100		
14	100		
10	98		
6.3	97		
5.0	96		
3.35	95		
2.00	93		
1.18	90		
0.600	87	Particle density, Mg/m ³ 2.65 assumed	
0.425	85		
0.300	83	Dry mass of sample, kg 0.4	
0.212	79		
0.150	74		
0.063	62		

Soil description	Greyish brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	7	7
	Silt	31	31
	Clay	37	37
		25	25

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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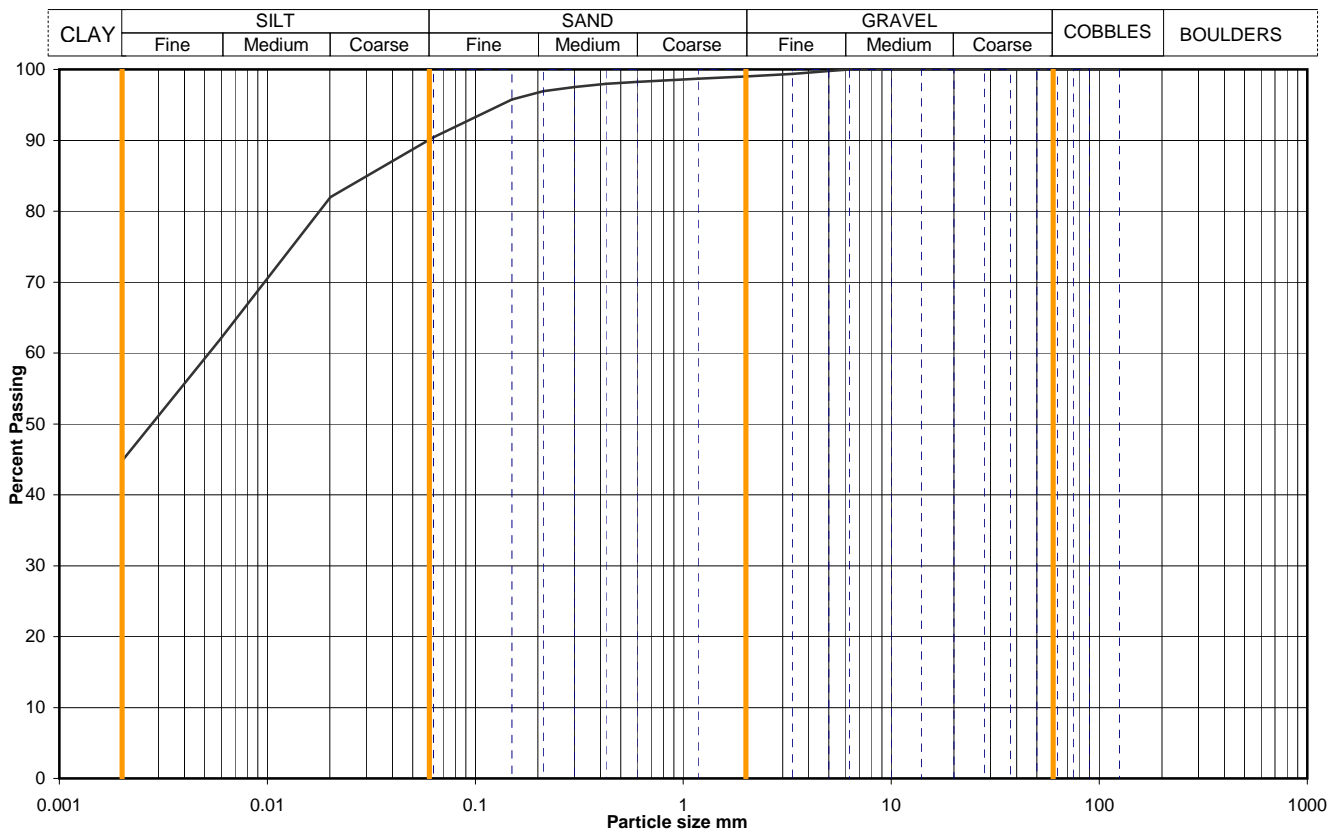
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Figure

PSD 5

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH1
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	14.45
			Samp No	38
			Type	D
			ID	ESGA1077-11201110100000000039
			Spec Ref	



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	82
90	100	0.0060	62
75	100	0.0020	45
63	100		
50	100		
37.5	100		
28	100		
20	100		
14	100		
10	100		
6.3	100		
5.0	100		
3.35	99		
2.00	99		
1.18	99		
0.600	98	Particle density, Mg/m ³ 2.65 assumed	
0.425	98		
0.300	98	Dry mass of sample, kg 1.5	
0.212	97		
0.150	96		
0.063	90		

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	1	1
	Silt	9	9
	Clay	45	45

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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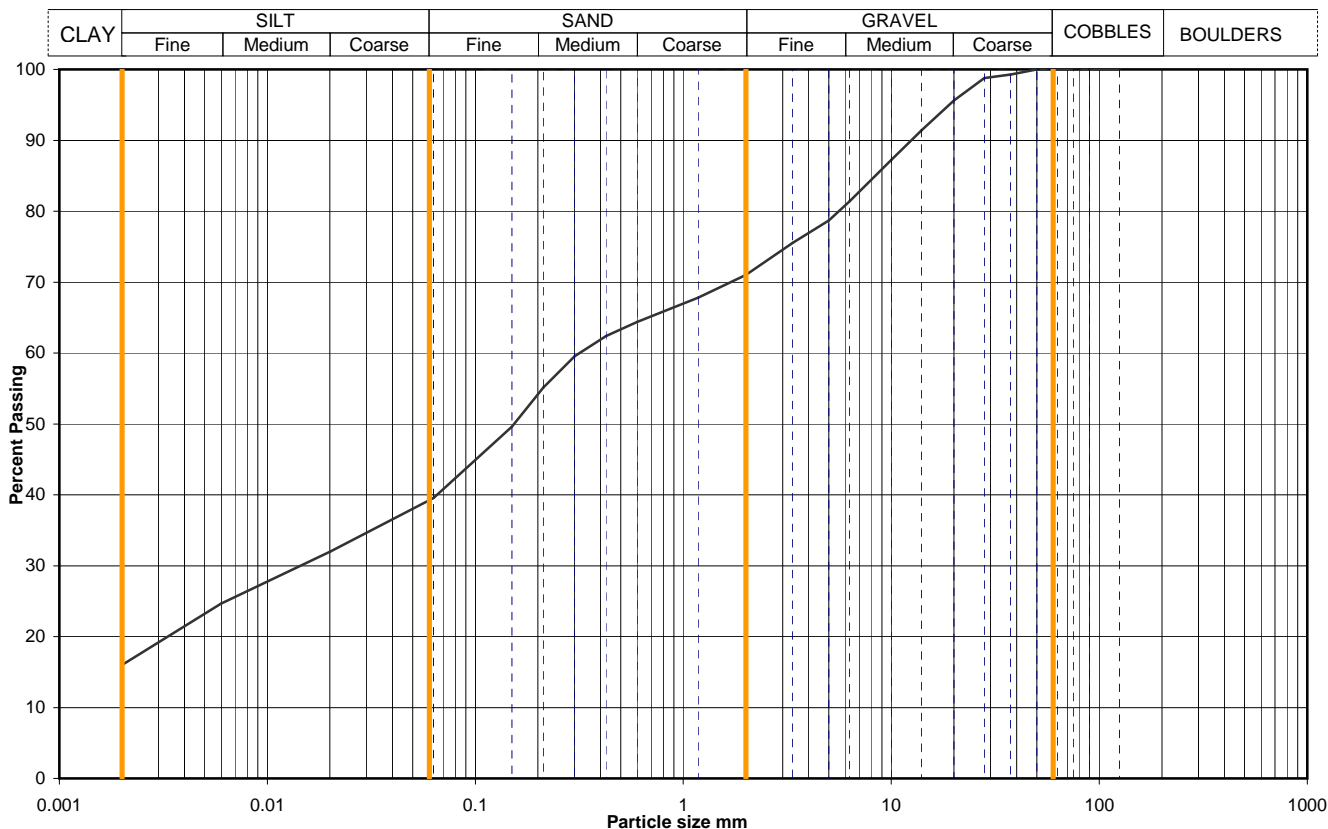
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Figure

PSD 6

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH1
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	16.15
			Samp No	44
			Type	B
			ID	ESGA1077-11201110100000000045
			Spec Ref	



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	32
90	100	0.0060	25
75	100	0.0020	16
63	100		
50	100		
37.5	99		
28	99		
20	96		
14	91		
10	87		
6.3	81		
5.0	79		
3.35	76		
2.00	71		
1.18	68		
0.600	64	Particle density, Mg/m ³ 2.65 assumed	
0.425	62		
0.300	60	Dry mass of sample, kg 12.8	
0.212	55		
0.150	50		
0.063	40		

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	29	29
	Silt	32	32
	Clay	23	23

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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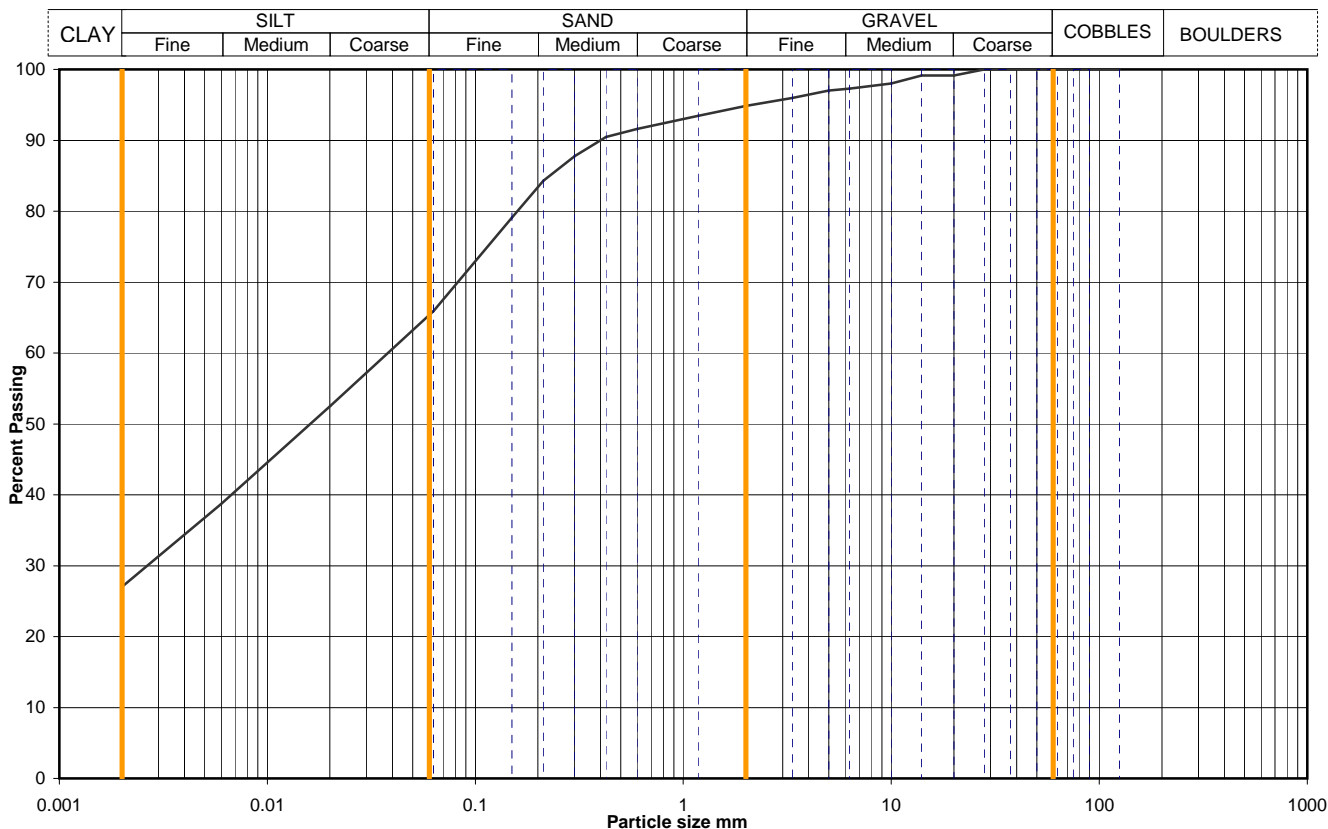
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Figure

PSD 7

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH1		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		20.15		
			Samp No	55	Type	B	
			ID	ESGA1077-11201110100000000056			
			Spec Ref				



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	53
90	100	0.0060	39
75	100	0.0020	27
63	100		
50	100		
37.5	100		
28	100		
20	99		
14	99		
10	98		
6.3	97		
5.0	97		
3.35	96		
2.00	95		
1.18	93		
0.600	92	Particle density, Mg/m3 2.65 assumed	
0.425	90		
0.300	88	Dry mass of sample, kg 6.1	
0.212	84		
0.150	79		
0.063	66		

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	5	5
	Silt	29	29
	Clay	39	39

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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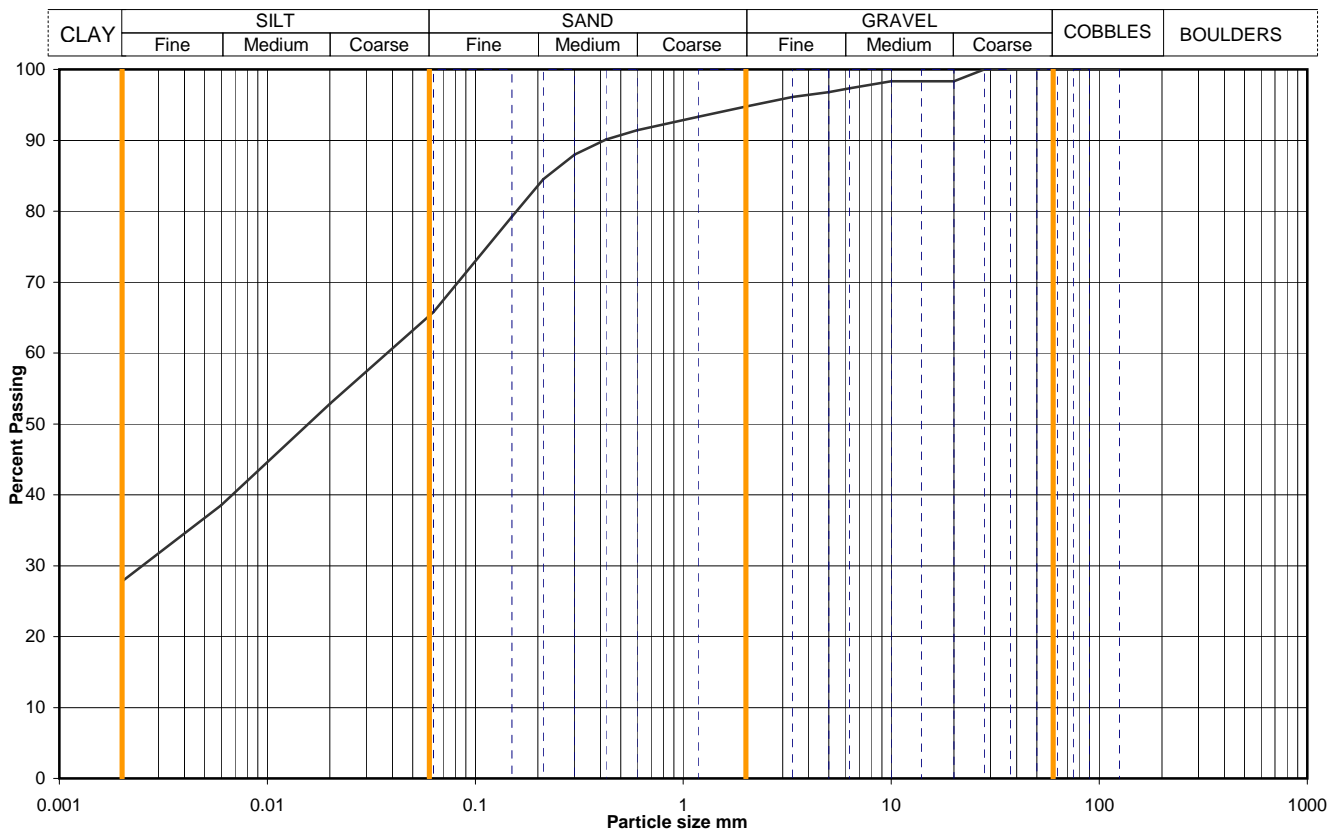


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Figure
PSD 8

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH1		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		23.15		
			Samp No	63	Type	B	
			ID	ESGA1077-11201110100000000064			
			Spec Ref				



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	53
90	100	0.0060	39
75	100	0.0020	28
63	100		
50	100		
37.5	100		
28	100		
20	98		
14	98		
10	98		
6.3	97		
5.0	97		
3.35	96		
2.00	95		
1.18	93		
0.600	91	Particle density, Mg/m ³ 2.65 assumed	
0.425	90		
0.300	88	Dry mass of sample, kg 4.5	
0.212	84		
0.150	79		
0.063	66		

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	5	5
	Silt	30	30
	Clay	37	37
		28	28

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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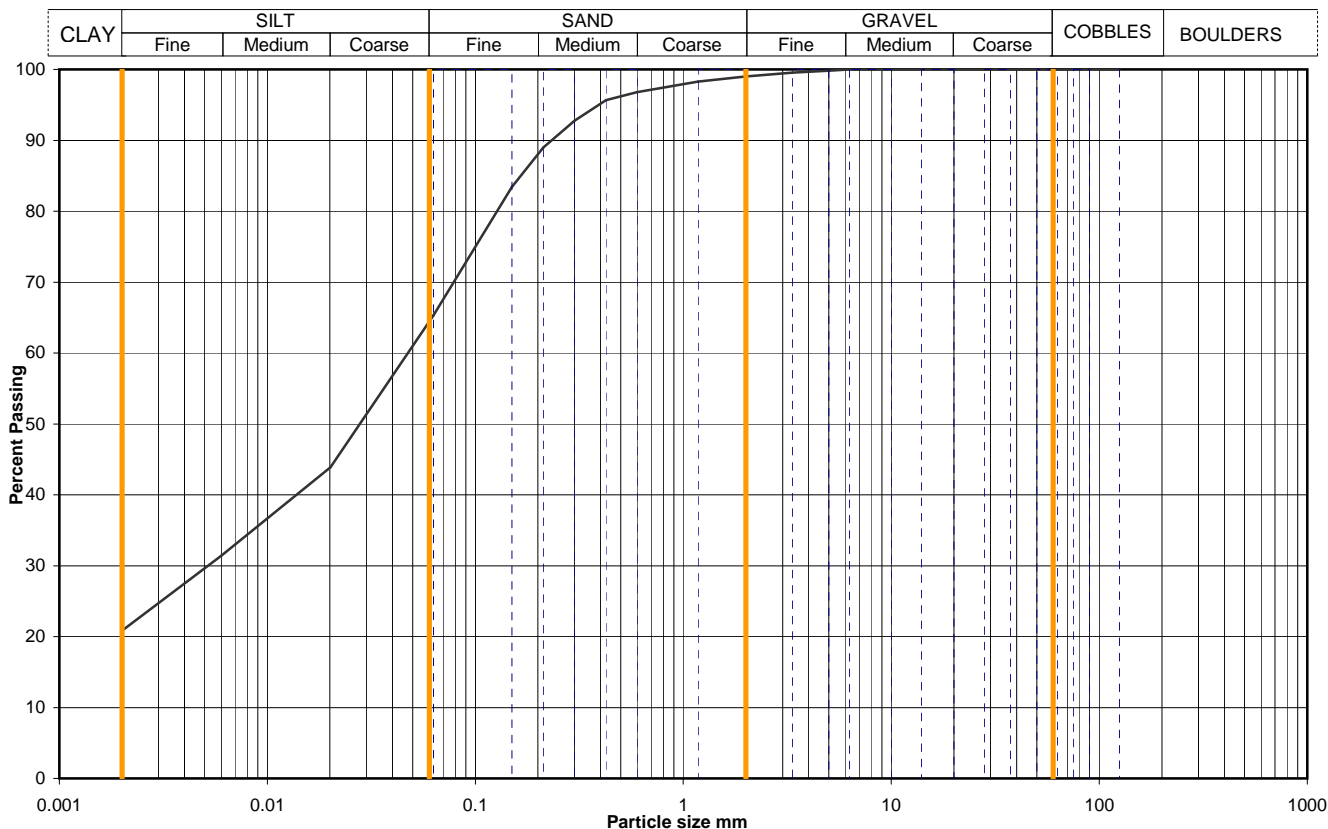
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Figure

PSD 9

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH2	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		0.50	
			Samp No	3	Type	B
			ID	ESGA1077-11201110100000000069		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	44
90	100	0.0060	31
75	100	0.0020	21
63	100		
50	100		
37.5	100		
28	100		
20	100		
14	100		
10	100		
6.3	100		
5.0	100		
3.35	100		
2.00	99		
1.18	98		
0.600	97	Particle density, Mg/m ³ 2.65 assumed	
0.425	96		
0.300	93	Dry mass of sample, kg 7.6	
0.212	89		
0.150	83		
0.063	65		

Soil description	Brown slightly gravelly sandy silty CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	1	1
	Silt	35	35
	Clay	43	43

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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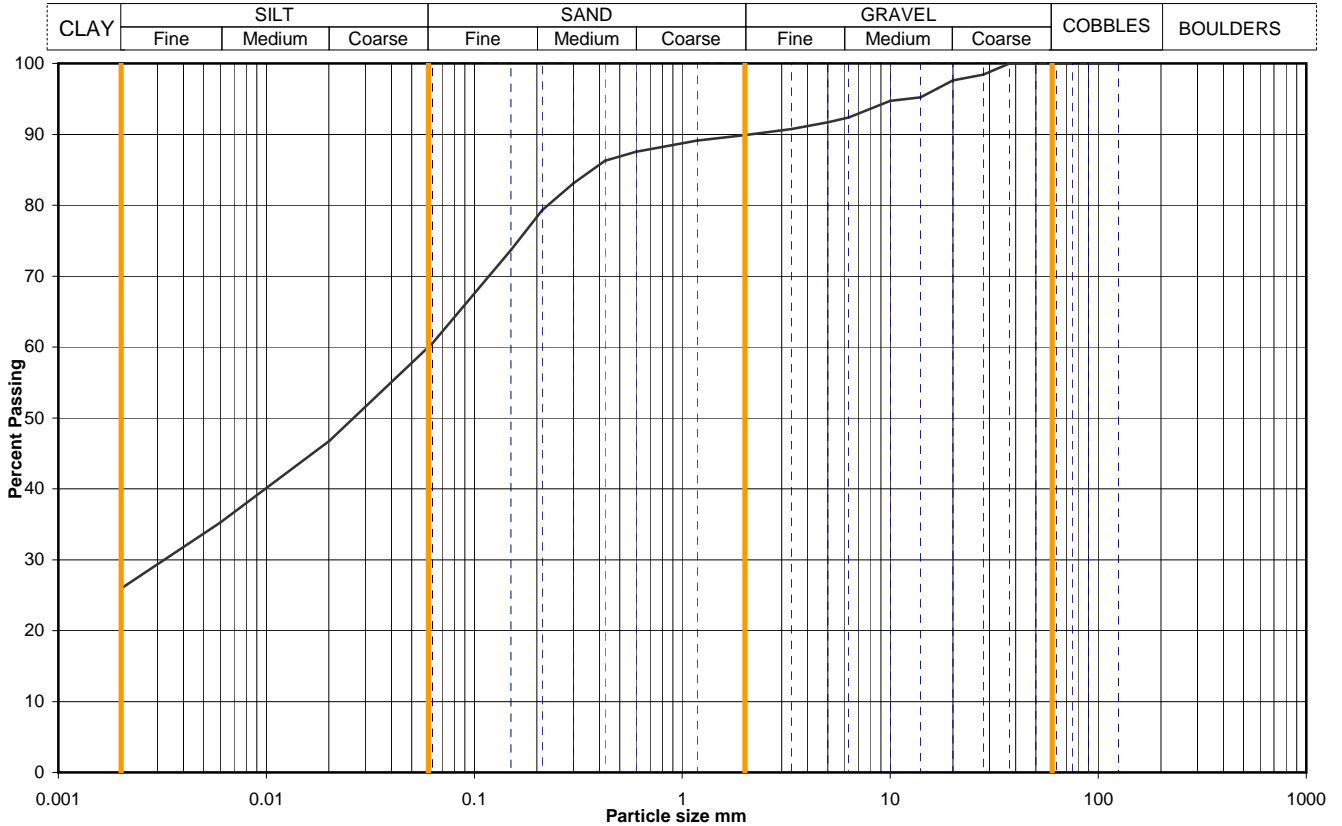
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Figure

PSD 10

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH2	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		2.00	
			Samp No	7	Type	B
			ID	ESGA1077-11201110100000000073		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	47
90	100	0.0060	35
75	100	0.0020	26
63	100		
50	100		
37.5	100		
28	98		
20	98		
14	95		
10	95		
6.3	92		
5.0	92		
3.35	91		
2.00	90		
1.18	89		
0.600	88	Particle density, Mg/m ³ 2.65 assumed	
0.425	86		
0.300	83	Dry mass of sample, kg 12.2	
0.212	79		
0.150	74		
0.063	61		

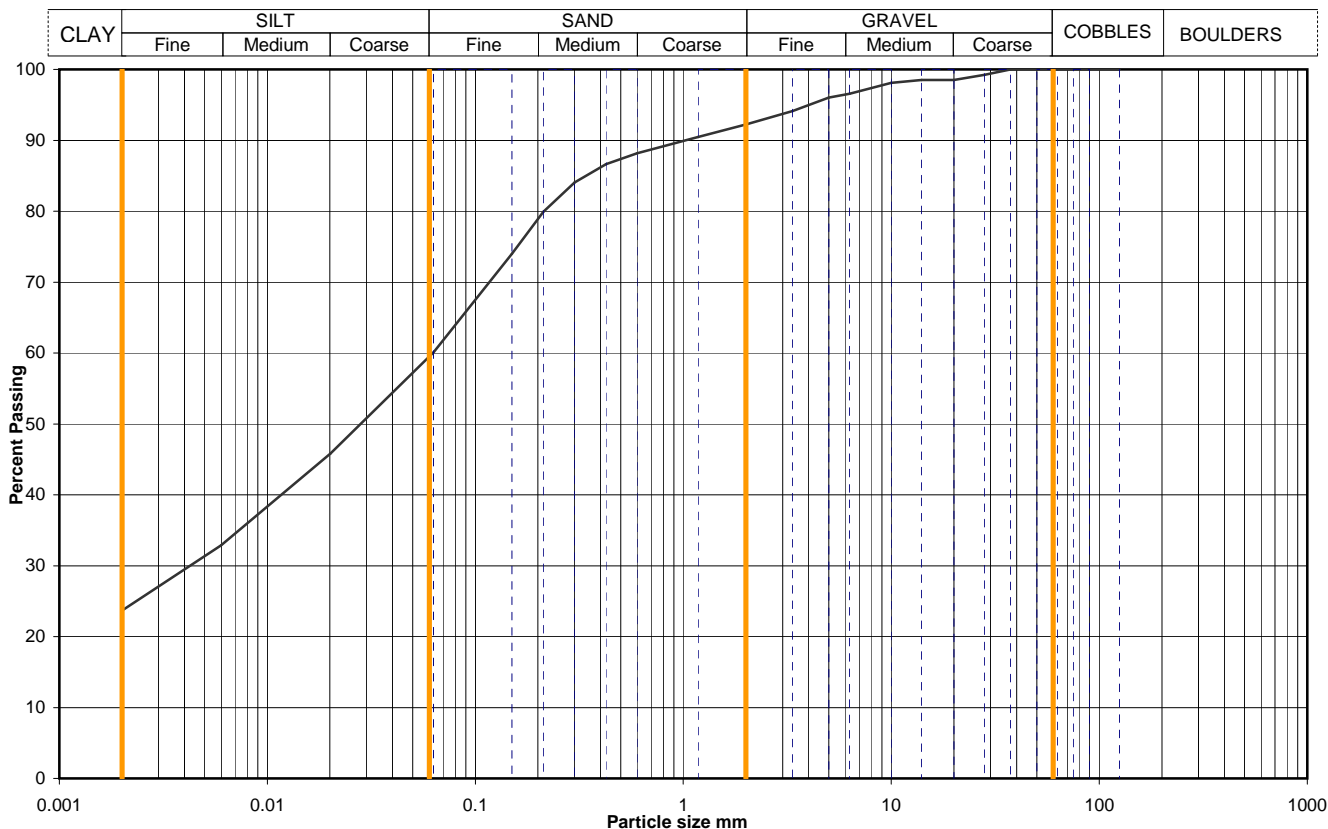
Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	10	10
	Silt	30	30
	Clay	34	34
		26	26

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH2		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		5.00		
			Samp No	12	Type	U	
			ID	ESGA1077-11201110100000000078			
			Spec Ref				



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	46
90	100	0.0060	33
75	100	0.0020	24
63	100		
50	100		
37.5	100		
28	99		
20	98		
14	98		
10	98		
6.3	97		
5.0	96		
3.35	94		
2.00	92		
1.18	90		
0.600	88	Particle density, Mg/m ³ 2.65 assumed	
0.425	87		
0.300	84	Dry mass of sample, kg 3.8	
0.212	80		
0.150	74		
0.063	60		

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	8	8
	Silt	33	33
	Clay	36	36

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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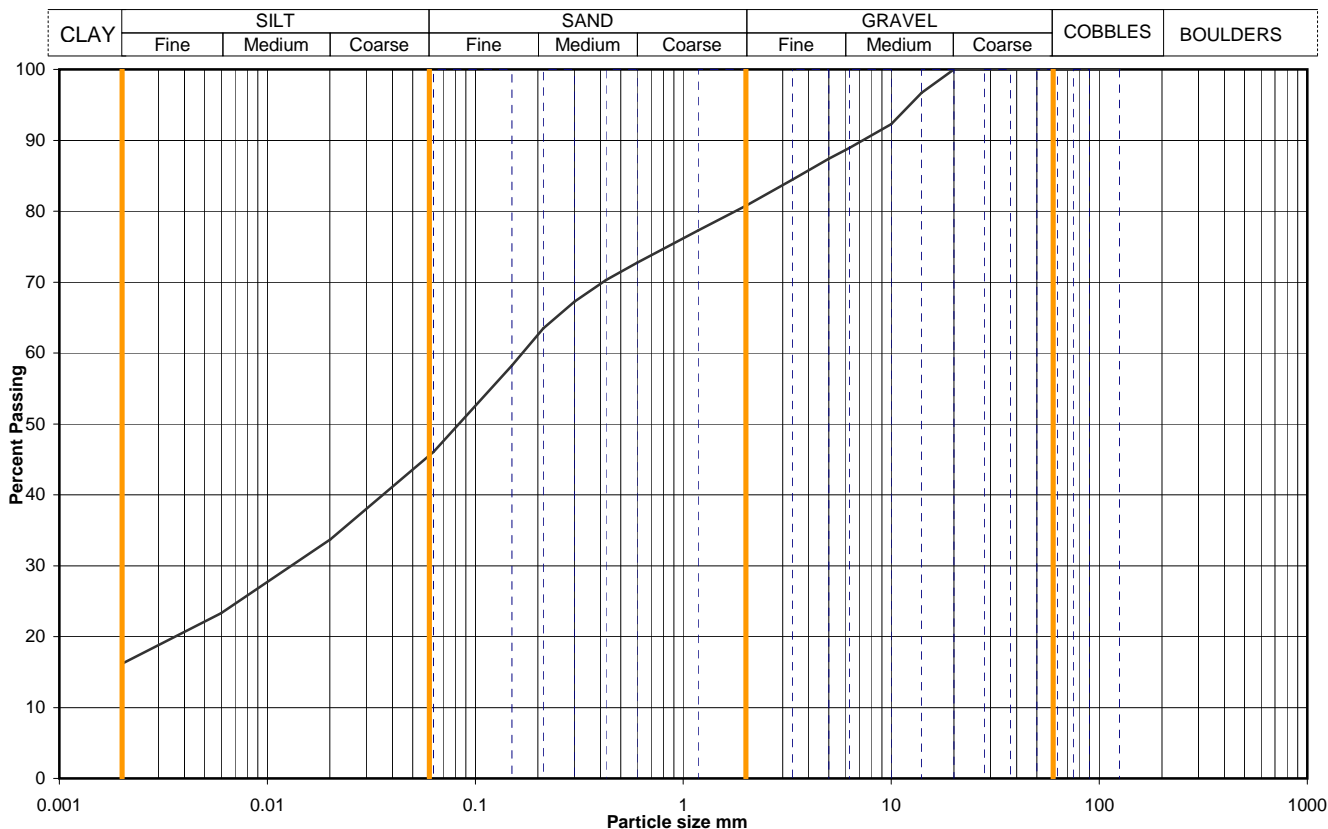
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Figure

PSD 12

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH2	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		8.45	
			Samp No	19	Type	D
			ID	ESGA1077-11201110100000000085		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	34
90	100	0.0060	23
75	100	0.0020	16
63	100		
50	100		
37.5	100		
28	100		
20	100		
14	97		
10	92		
6.3	89		
5.0	87		
3.35	84		
2.00	81		
1.18	77		
0.600	73	Particle density, Mg/m ³ 2.65 assumed	
0.425	70		
0.300	67	Dry mass of sample, kg 1.5	
0.212	63		
0.150	58		
0.063	46		

Soil description	Brown slightly gravelly sandy CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	19	19
	Silt	35	35
	Clay	30	30
		16	16

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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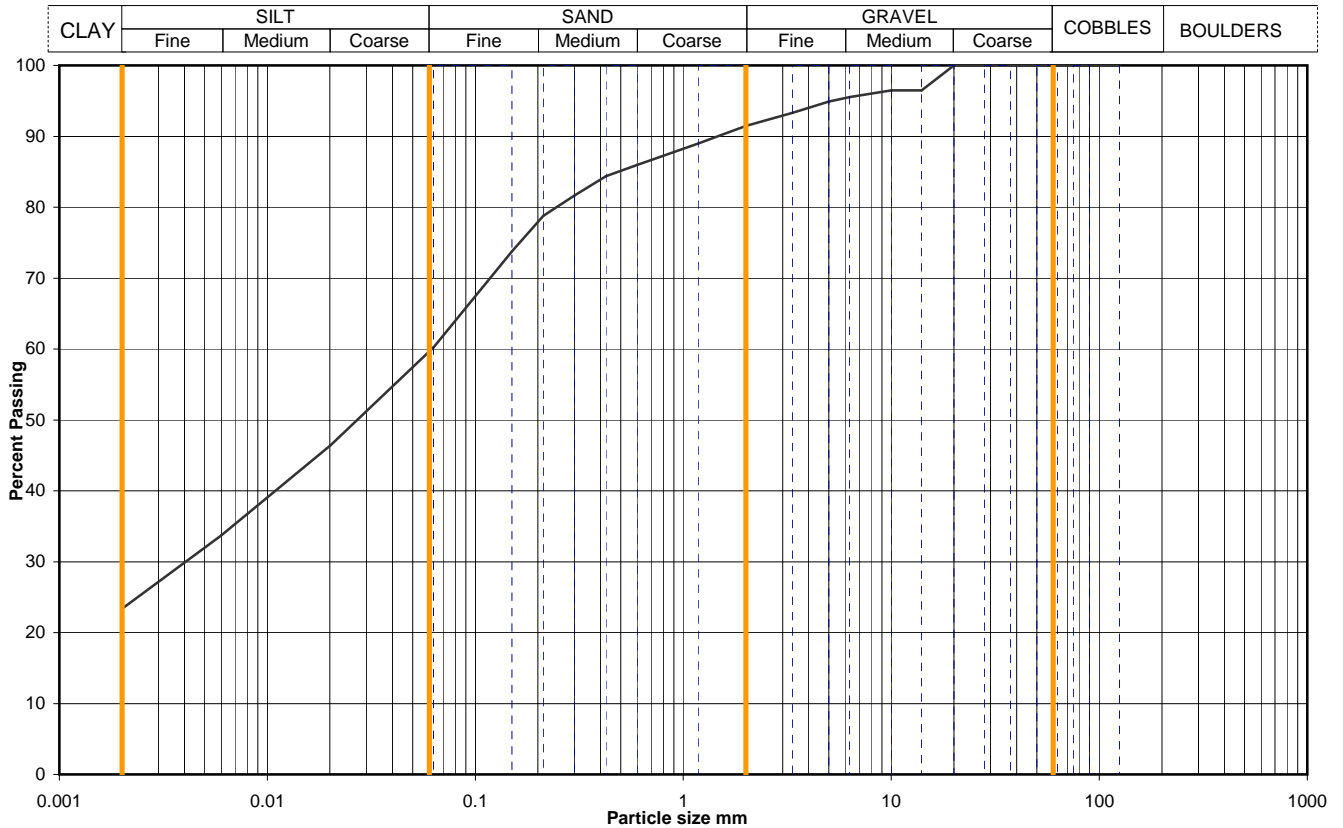
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Figure

PSD 13

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH2		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	11.45		
			Samp No	25	Type	D
			ID	ESGA1077-11201110100000000091		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	46
90	100	0.0060	34
75	100	0.0020	23
63	100		
50	100		
37.5	100		
28	100		
20	100		
14	96		
10	96		
6.3	96		
5.0	95		
3.35	93		
2.00	91		
1.18	89	Particle density, Mg/m3 2.65 assumed	
0.600	86		
0.425	84	Dry mass of sample, kg 1.1	
0.300	82		
0.212	79		
0.150	74		
0.063	60		

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions *<60mm values to aid description only	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		9	9
		32	32
		36	36
		23	23

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref

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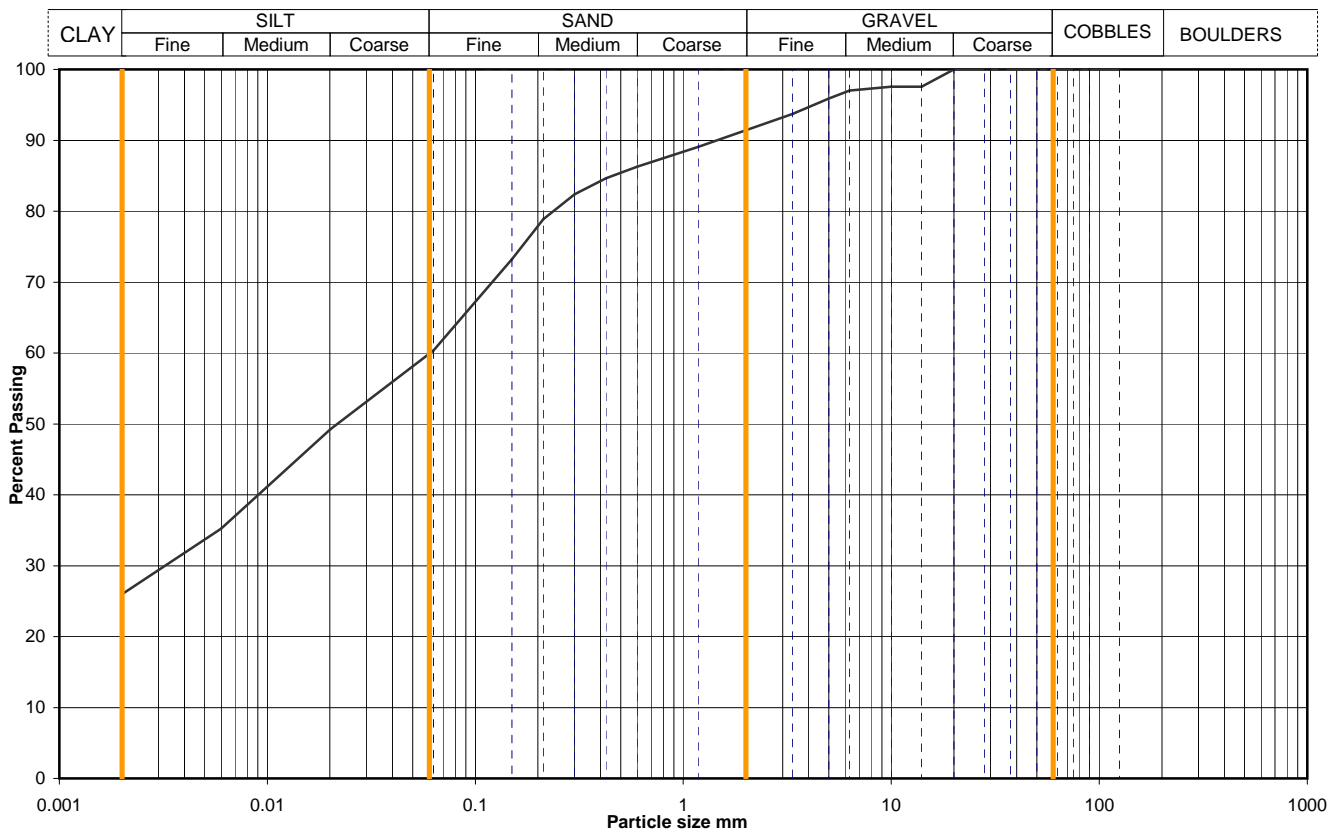
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Figure

PSD 14

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH2	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		15.45	
			Samp No	32	Type	D
			ID	ESGA1077-11201110100000000099		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	49
90	100	0.0060	35
75	100	0.0020	26
63	100		
50	100		
37.5	100		
28	100		
20	100		
14	98		
10	98		
6.3	97		
5.0	96		
3.35	94		
2.00	91		
1.18	89		
0.600	86		
0.425	85		
0.300	82		
0.212	79		
0.150	73		
0.063	60		

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions *<60mm values to aid description only	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		9	9
		32	32
		33	33
		26	26

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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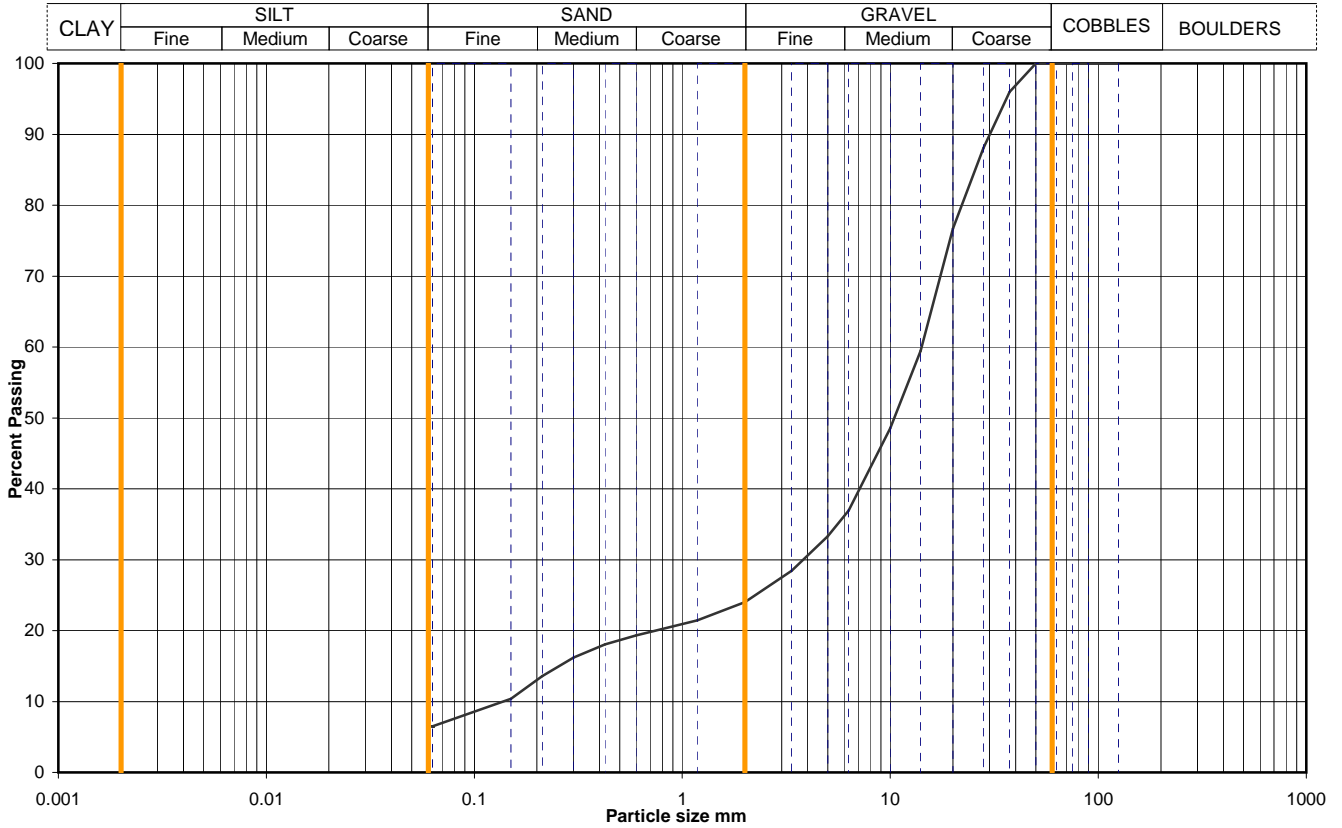
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Figure

PSD 15

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH2	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		18.00	
			Samp No	41	Type	B
			ID	ESGA1077-11201110100000000108		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	100		
37.5	96		
28	88		
20	77		
14	59		
10	49		
6.3	37		
5.0	33		
3.35	28		
2.00	24		
1.18	21		
0.600	19		
0.425	18		
0.300	16		
0.212	14		
0.150	10		
0.063	6		

Dry mass of sample, kg			
15.9			

Soil description	Brown clayey sandy GRAVEL.		
Preparation / Pretreatment	Sieve: natural material		
Remarks			
Sample Proportions <small>*<60mm values to aid description only</small>	Cobbles / boulders	Whole	*<60mm
	Gravel	0	0
	Sand	76	76
	Silt	18	18
	Clay	silt+clay =	
		6	6

Uniformity Coefficient	D_{60} / D_{10}	103
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	none

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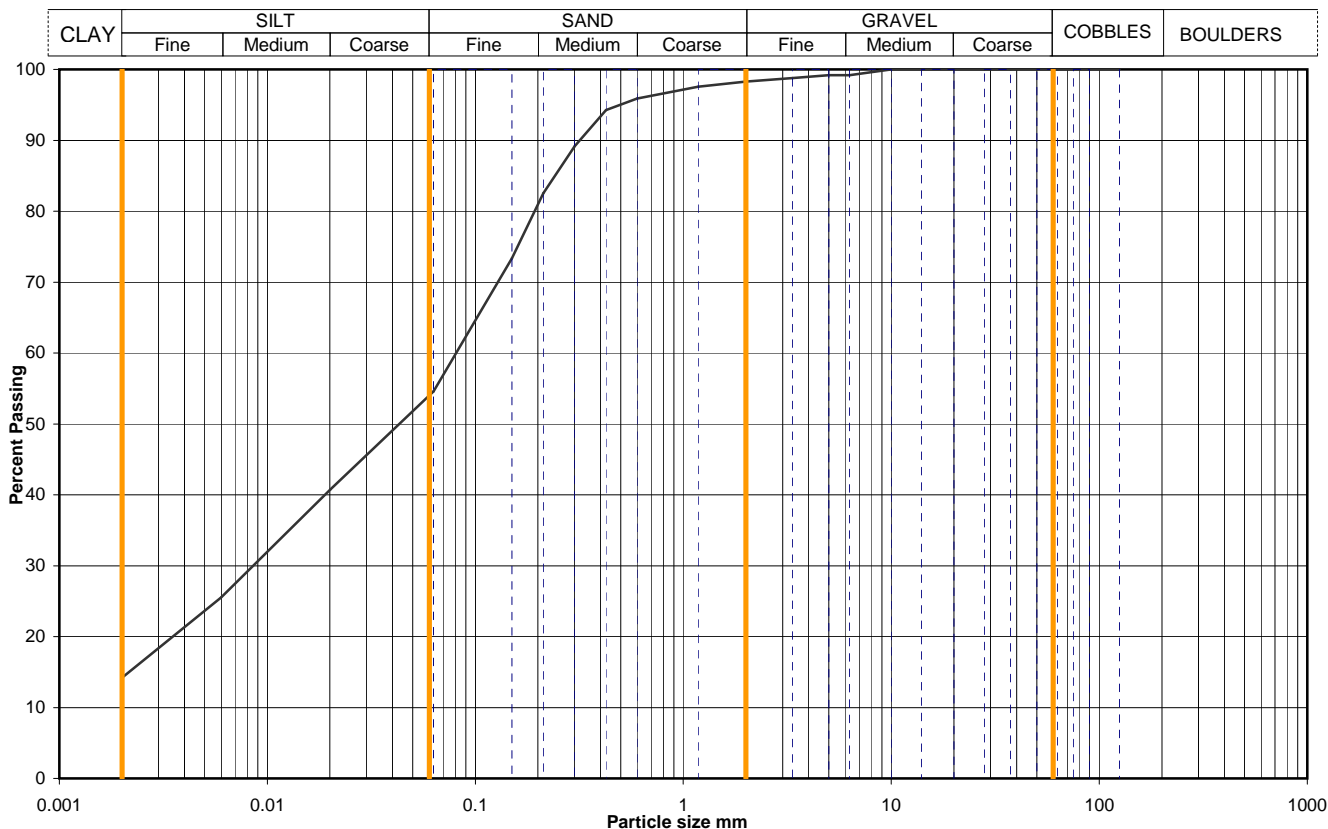


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Figure
PSD 16

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH3	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		0.50	
			Samp No	3	Type	B
			ID	ESGA1077-11201110110000000128		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	41
90	100	0.0060	25
75	100	0.0020	14
63	100		
50	100		
37.5	100		
28	100		
20	100		
14	100		
10	100		
6.3	99		
5.0	99		
3.35	99		
2.00	98		
1.18	98		
0.600	96	Particle density, Mg/m ³ 2.65 assumed	
0.425	94		
0.300	89	Dry mass of sample, kg 3.7	
0.212	83		
0.150	73		
0.063	55		

Soil description	Brown slightly gravelly sandy CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	2	2
	Silt	44	44
	Clay	40	40
		14	14

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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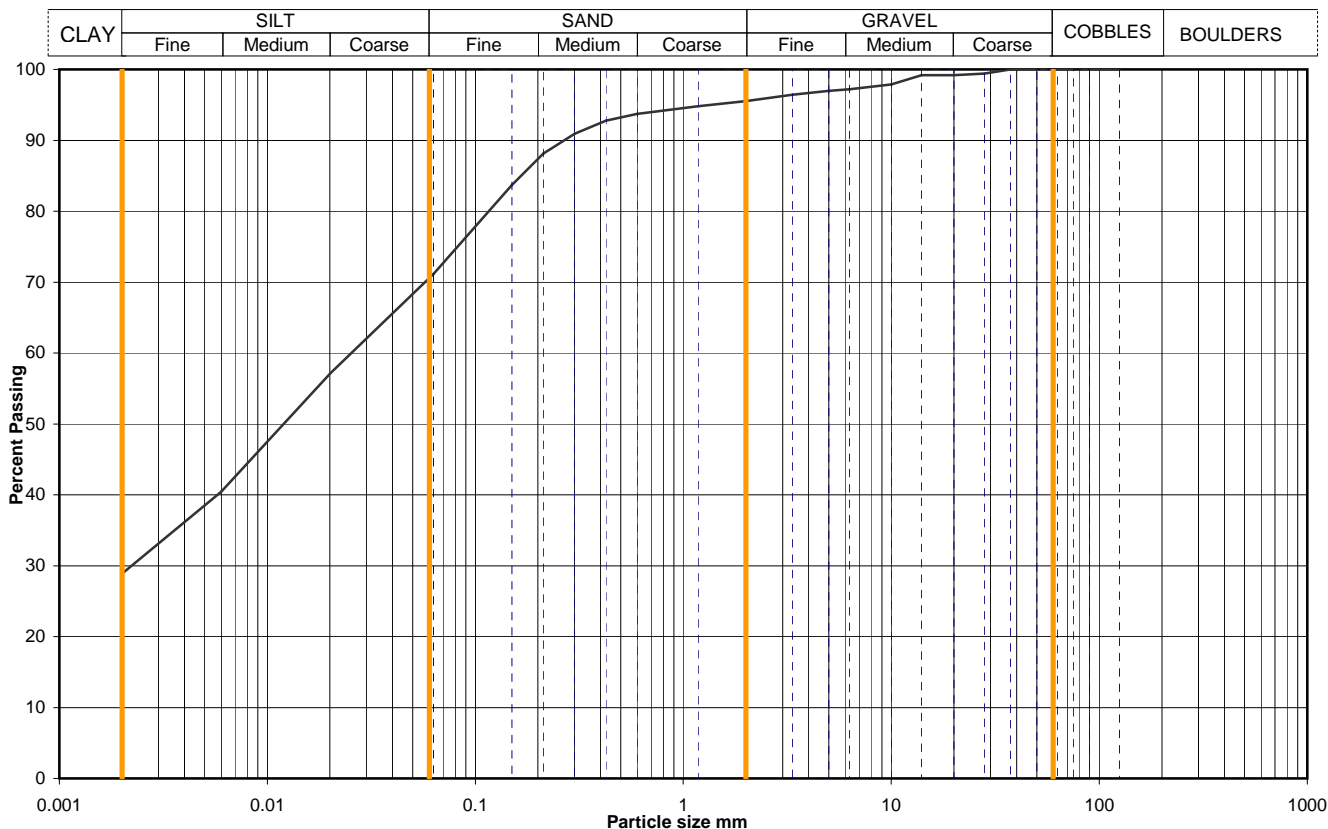
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Figure

PSD 17

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH3	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		1.85	
			Samp No	7	Type	B
			ID	ESGA1077-11201110110000000132		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	57
90	100	0.0060	40
75	100	0.0020	29
63	100		
50	100		
37.5	100		
28	99		
20	99		
14	99		
10	98		
6.3	97		
5.0	97		
3.35	96		
2.00	96		
1.18	95		
0.600	94	Particle density, Mg/m3 2.65 assumed	
0.425	93		
0.300	91	Dry mass of sample, kg 11.4	
0.212	88		
0.150	84		
0.063	71		

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * < 60mm values to aid description only	Cobbles / boulders	Whole	* < 60mm
	Gravel	0	0
	Sand	4	4
	Silt	25	25
	Clay	42	42

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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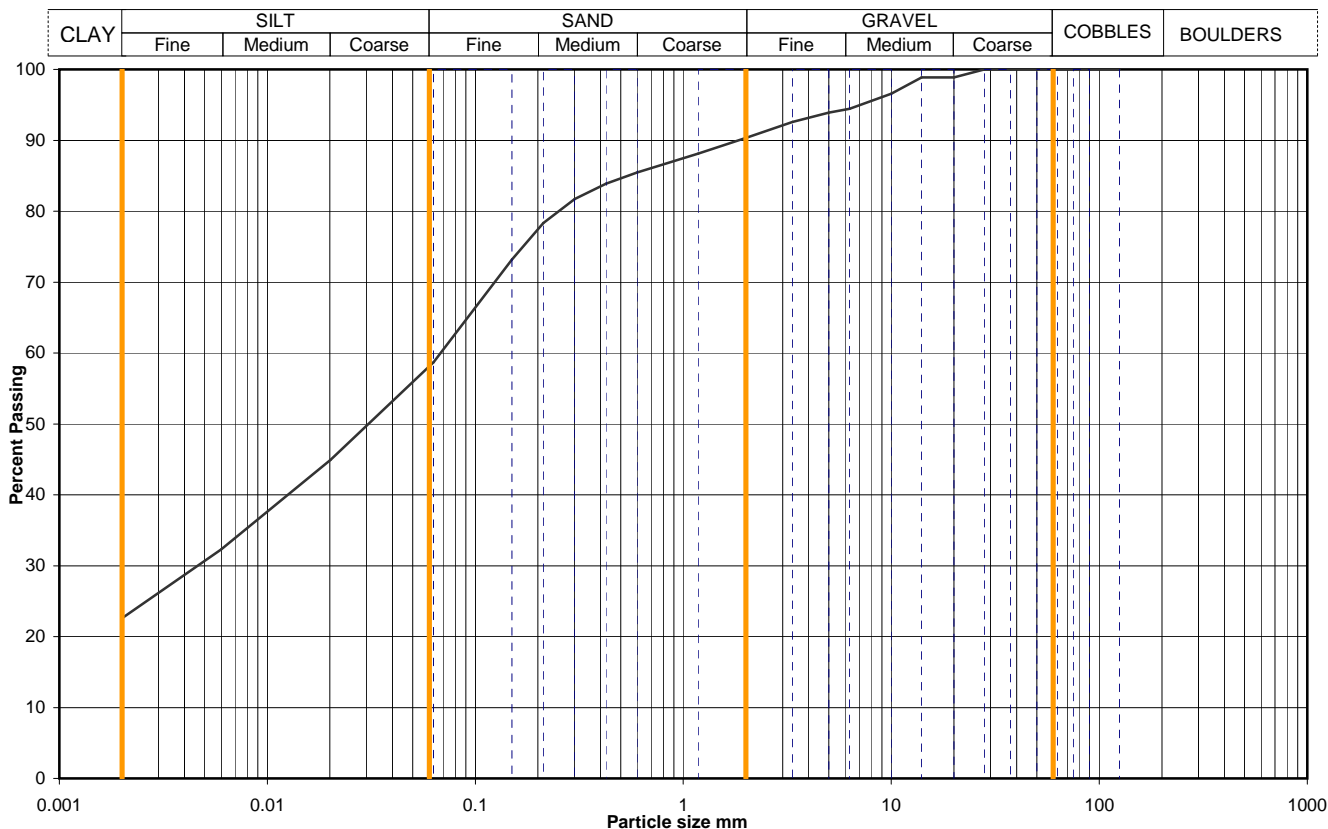


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Figure
PSD 18

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH3	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		4.65	
			Samp No	15	Type	B
			ID	ESGA1077-11201110110000000140		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	45
90	100	0.0060	32
75	100	0.0020	23
63	100		
50	100		
37.5	100		
28	100		
20	99		
14	99		
10	97		
6.3	94		
5.0	94		
3.35	93		
2.00	90		
1.18	88		
0.600	85	Particle density, Mg/m ³ 2.65 assumed	
0.425	84		
0.300	82	Dry mass of sample, kg 16.8	
0.212	78		
0.150	73		
0.063	59		

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	10	10
	Silt	32	32
	Clay	35	35
		23	23

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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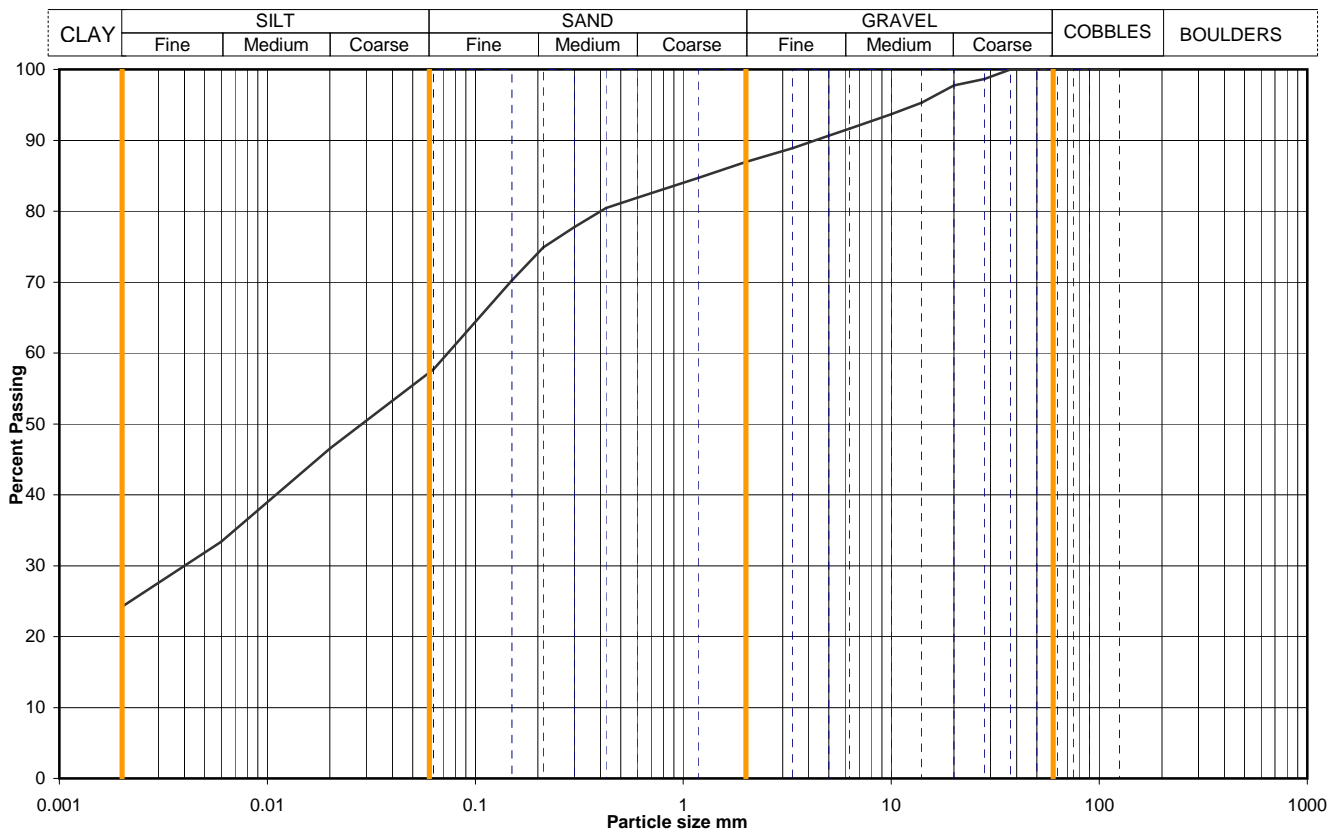


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Figure
PSD 19

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH3		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		7.65		
			Samp No	23	Type	B	
			ID	ESGA1077-11201110110000000148			
			Spec Ref				



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	47
90	100	0.0060	33
75	100	0.0020	24
63	100		
50	100		
37.5	100		
28	99		
20	98		
14	95		
10	94		
6.3	92		
5.0	91		
3.35	89		
2.00	87		
1.18	85		
0.600	82	Particle density, Mg/m ³ 2.65 assumed	
0.425	80		
0.300	78	Dry mass of sample, kg 16.4	
0.212	75		
0.150	70		
0.063	58		

Soil description	Greyish brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * < 60mm values to aid description only	Cobbles / boulders	Whole	* < 60mm
	Gravel	0	0
	Sand	13	13
	Silt	30	30
	Clay	33	33
		24	24

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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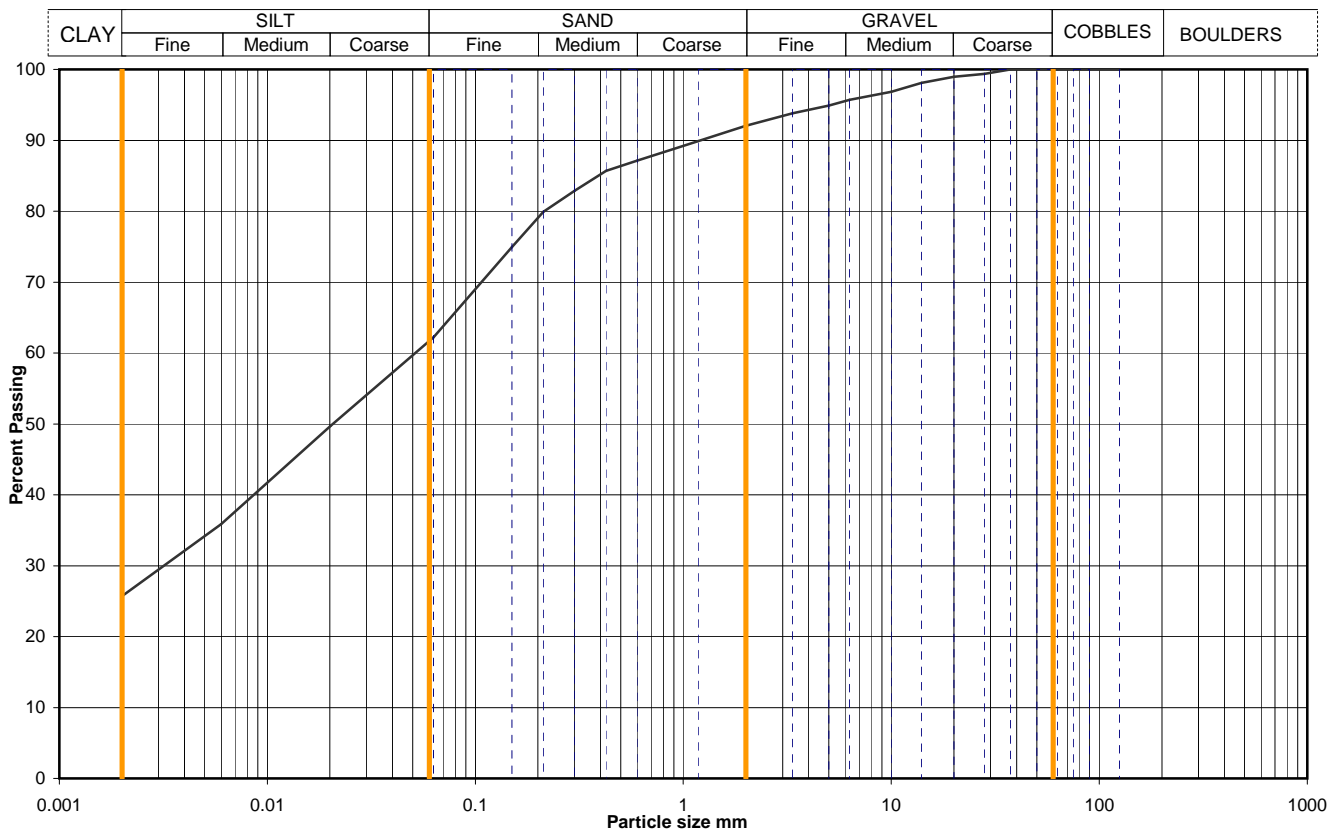
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Figure

PSD 20

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH3	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		9.15	
			Samp No	27	Type	B
			ID	ESGA1077-11201110110000000152		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	50
90	100	0.0060	36
75	100	0.0020	26
63	100		
50	100		
37.5	100		
28	99		
20	99		
14	98		
10	97		
6.3	96		
5.0	95		
3.35	94		
2.00	92		
1.18	90		
0.600	87	Particle density, Mg/m ³ 2.65 assumed	
0.425	86		
0.300	83	Dry mass of sample, kg 14.6	
0.212	80		
0.150	75		
0.063	62		

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * < 60mm values to aid description only	Cobbles / boulders	Whole	* < 60mm
	Gravel	0	0
	Sand	8	8
	Silt	30	30
	Clay	36	36

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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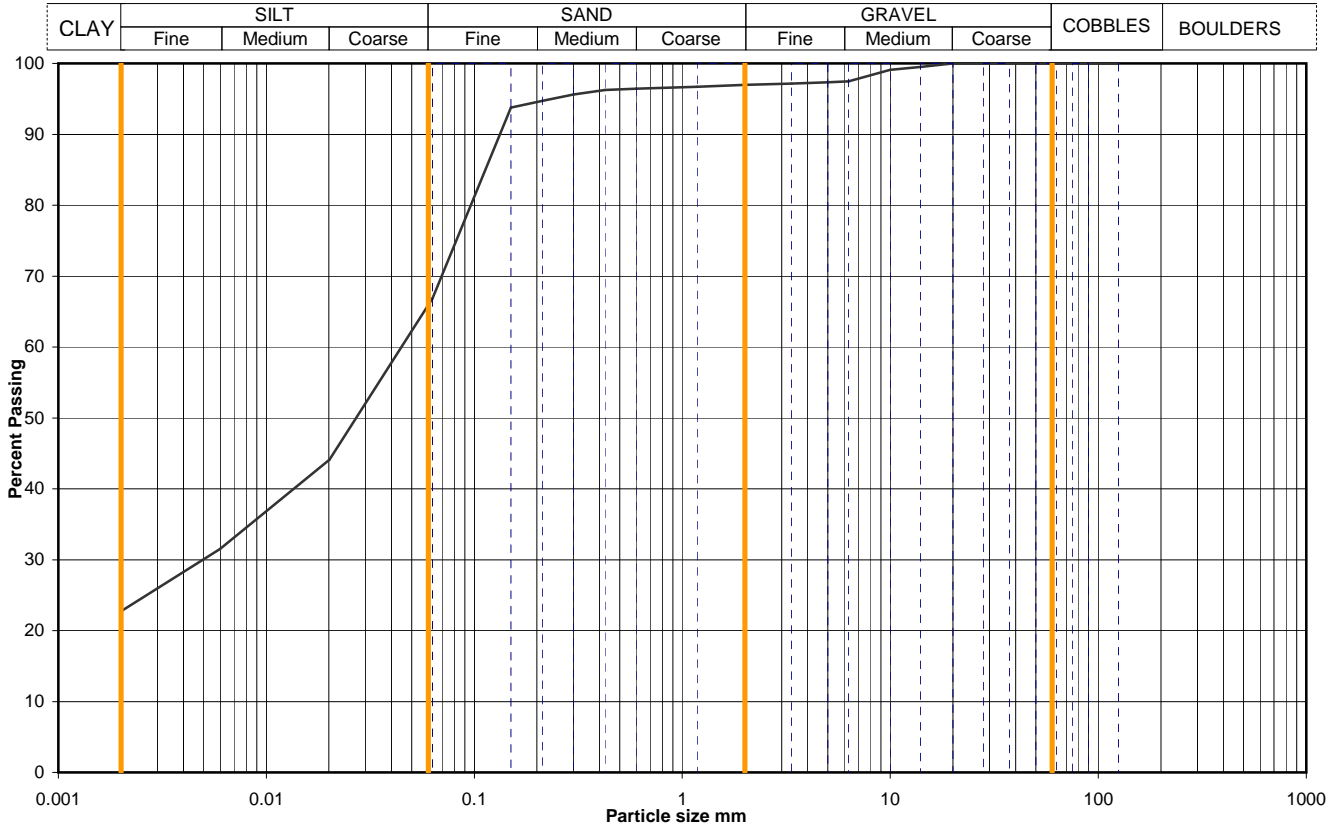
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Figure

PSD 21

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH3	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		11.50	
			Samp No	34	Type	B
			ID	ESGA1077-11201110110000000159		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	44
90	100	0.0060	32
75	100	0.0020	23
63	100		
50	100		
37.5	100		
28	100		
20	100		
14	99		
10	99		
6.3	97		
5.0	97		
3.35	97		
2.00	97		
1.18	97		
0.600	96	Particle density, Mg/m ³ 2.65 assumed	
0.425	96		
0.300	96	Dry mass of sample, kg 14.6	
0.212	95		
0.150	94		
0.063	67		

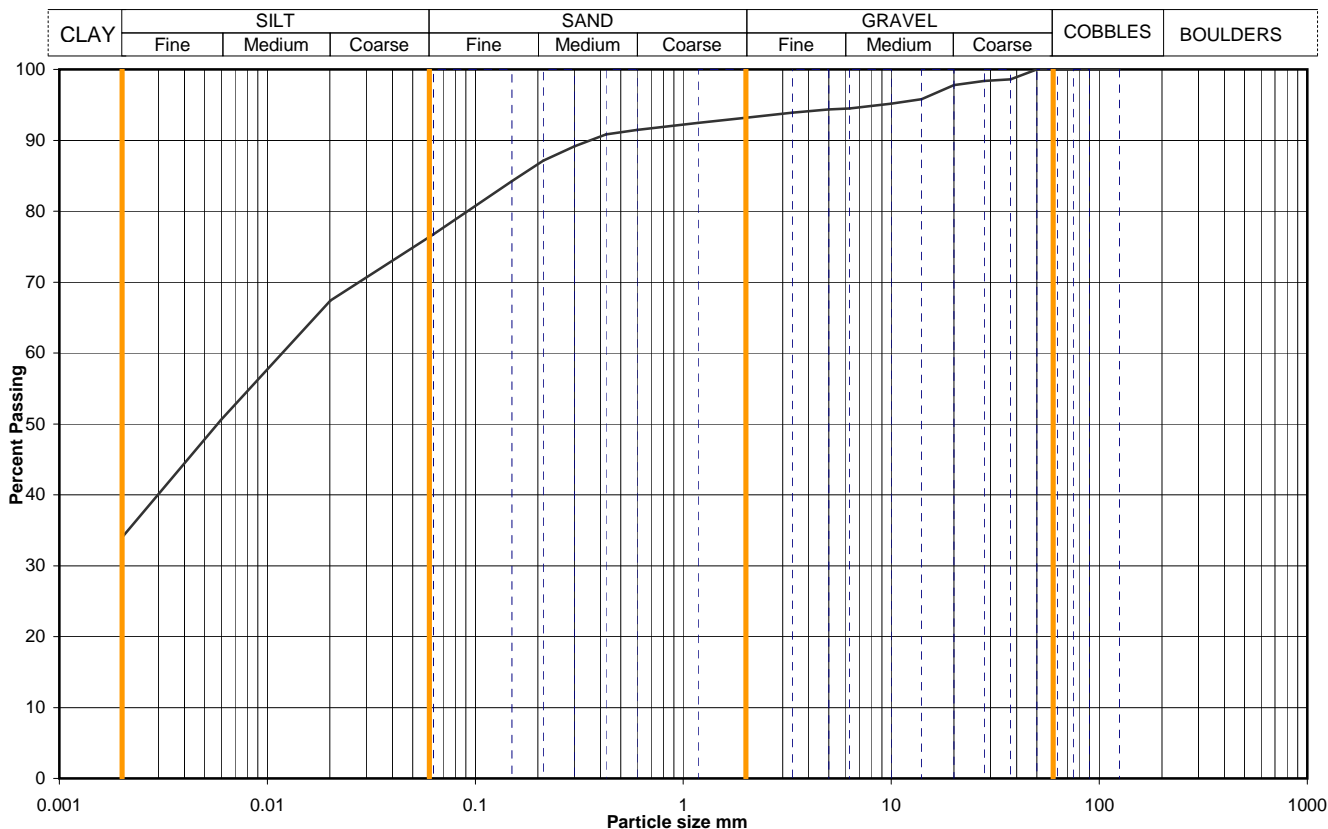
Soil description	Light brownish grey slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	3	3
	Silt	31	31
	Clay	43	43
		23	23

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH3
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	13.50
			Samp No	38
			Type	B
			ID	ESGA1077-11201110110000000163
			Spec Ref	



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	67
90	100	0.0060	51
75	100	0.0020	34
63	100		
50	100		
37.5	99		
28	98		
20	98		
14	96		
10	95		
6.3	94		
5.0	94		
3.35	94		
2.00	93		
1.18	92		
0.600	91		
0.425	91		
0.300	89		
0.212	87		
0.150	84		
0.063	77		

Particle density, Mg/m ³	
2.65	assumed
Dry mass of sample, kg	
13.1	

Soil description	Brownish grey slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	7	7
	Silt	17	17
	Clay	42	42
		34	34

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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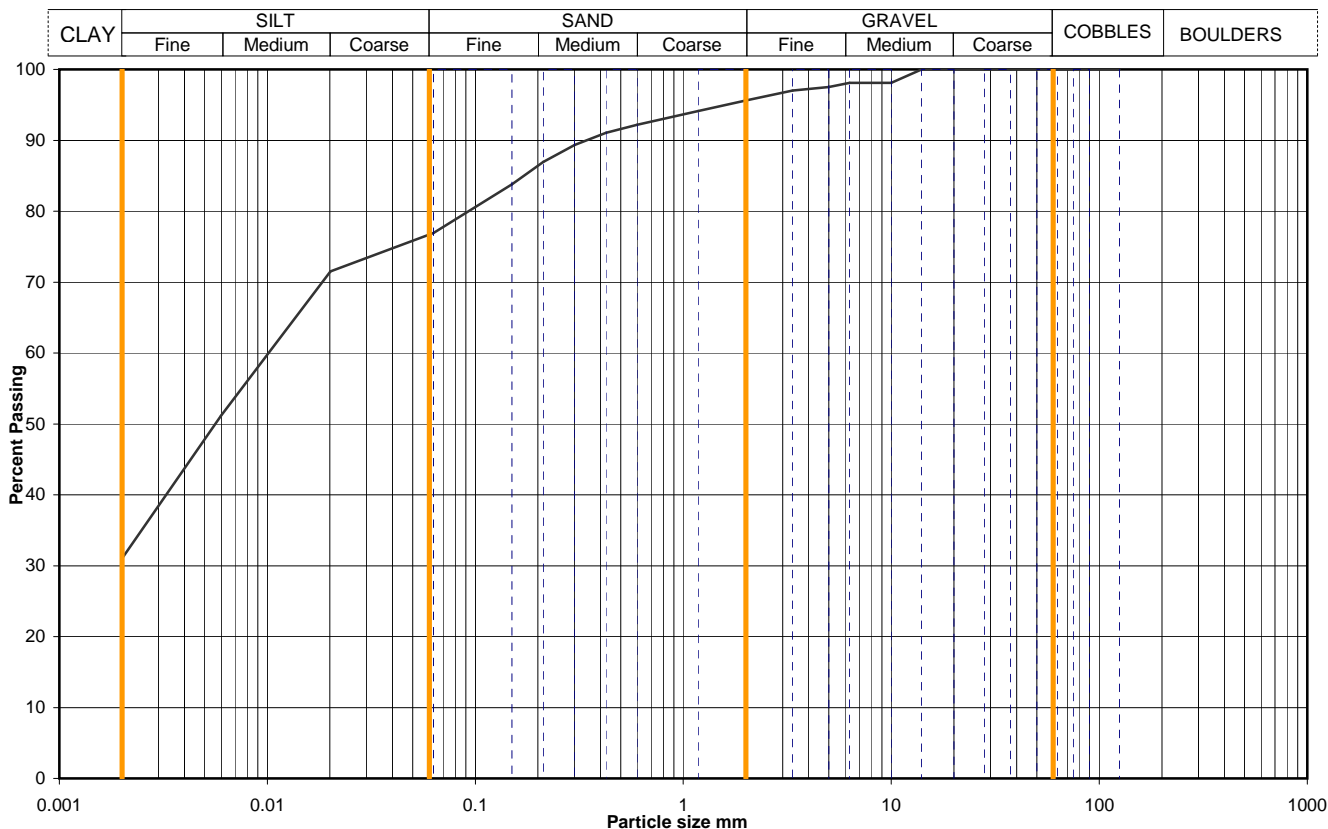
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Figure

PSD 23

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH3
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	13.50
			Samp No	37
			Type	D
			ID	ESGA1077-11201110110000000162
			Spec Ref	



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	72
90	100	0.0060	51
75	100	0.0020	31
63	100		
50	100		
37.5	100		
28	100		
20	100		
14	100		
10	98		
6.3	98		
5.0	98		
3.35	97		
2.00	96		
1.18	94		
0.600	92		
0.425	91		
0.300	89		
0.212	87		
0.150	84		
0.063	77		

Soil description	Brownish grey slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: pre dried, Pipette: as BS1377		
Remarks			
Sample Proportions *<60mm values to aid description only	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		4	4
		19	19
		46	46
		31	31

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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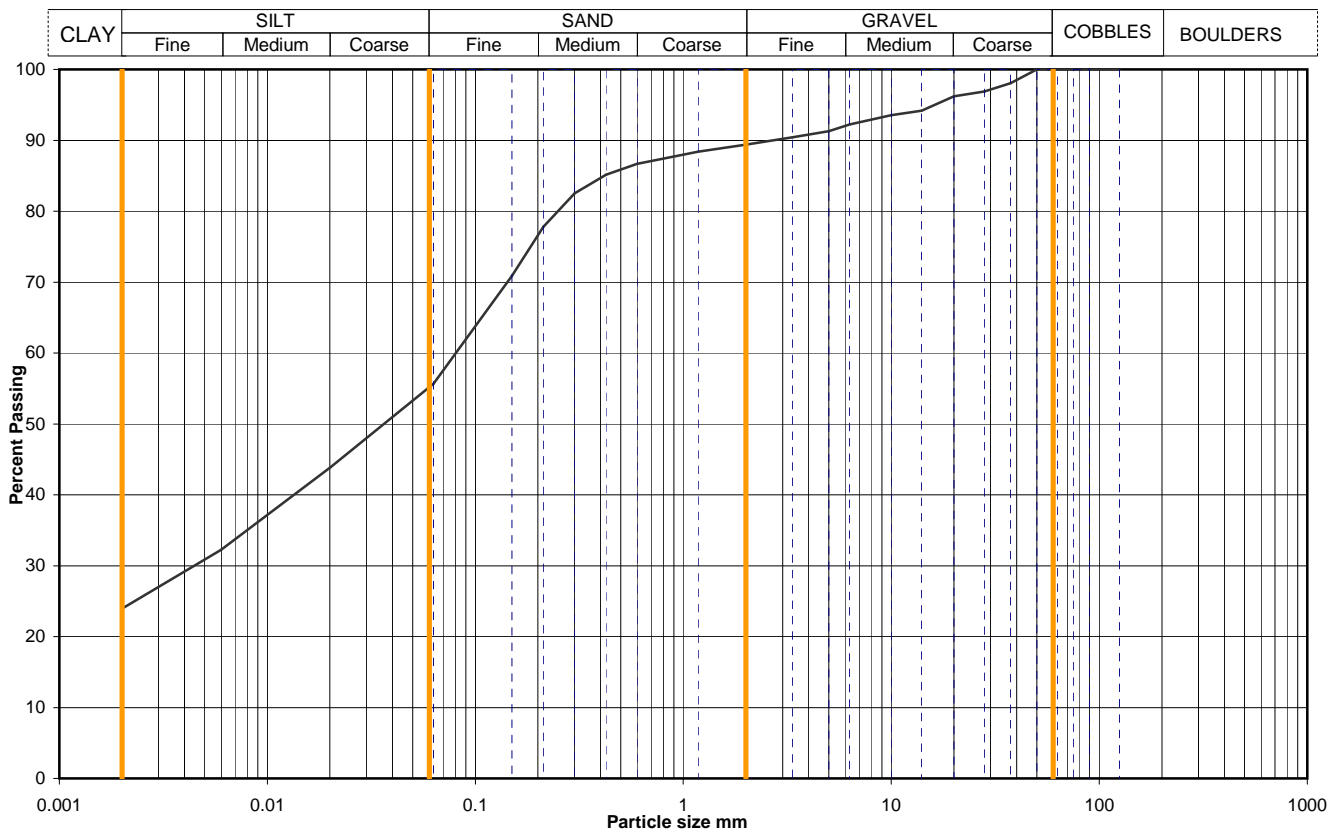
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Figure

PSD 24

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH3	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		16.65	
			Samp No	46	Type	B
			ID	ESGA1077-11201110110000000170		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	44
90	100	0.0060	32
75	100	0.0020	24
63	100		
50	100		
37.5	98		
28	97		
20	96		
14	94		
10	94		
6.3	92		
5.0	91		
3.35	90		
2.00	89		
1.18	88		
0.600	87	Particle density, Mg/m ³ 2.65 assumed	
0.425	85		
0.300	83	Dry mass of sample, kg 15.0	
0.212	78		
0.150	71		
0.063	56		

Soil description	Brownish grey slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	11	11
	Silt	34	34
	Clay	31	31
		24	24

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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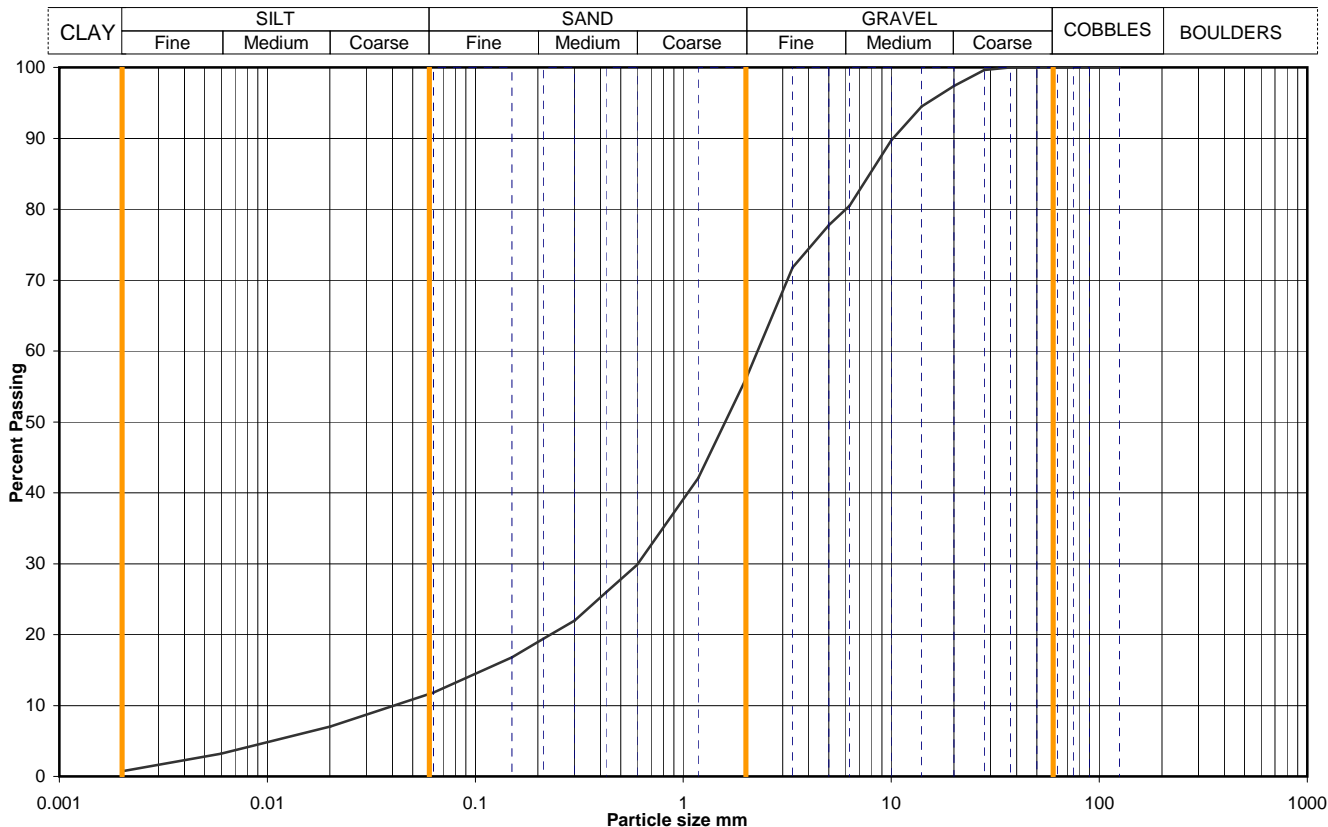
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Figure

PSD 25

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH4	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		0.20	
			Samp No	4	Type	B
			ID	ESGA1077-11201110110000000184		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	7
90	100	0.0060	3
75	100	0.0020	1
63	100		
50	100		
37.5	100		
28	100		
20	97		
14	95		
10	90		
6.3	81		
5.0	78		
3.35	72		
2.00	56		
1.18	42		
0.600	30	Particle density, Mg/m3 2.65 assumed	
0.425	26		
0.300	22	Dry mass of sample, kg 8.0	
0.212	19		
0.150	17		
0.063	12		

Soil description	Light grey sandy gravelly SILT.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	44	44
	Silt	44	44
	Clay	11	11

Uniformity Coefficient	D_{60} / D_{10}	56
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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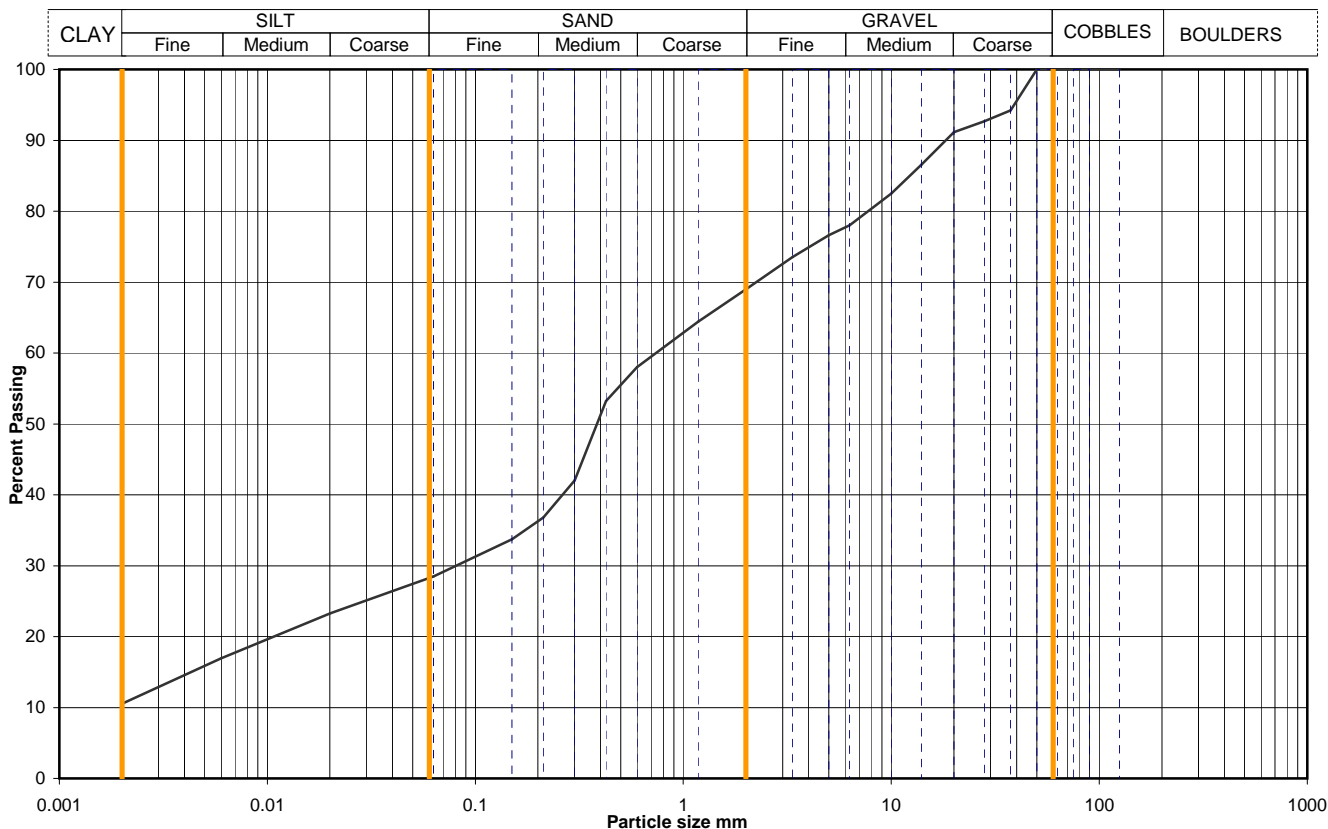


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Figure
PSD 26

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH4	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		0.80	
			Samp No	8	Type	B
			ID	ESGA1077-11201110110000000188		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	23
90	100	0.0060	17
75	100	0.0020	11
63	100		
50	100		
37.5	94		
28	93		
20	91		
14	87		
10	82		
6.3	78		
5.0	77		
3.35	74		
2.00	69		
1.18	64		
0.600	58	Particle density, Mg/m3 2.65 assumed	
0.425	53		
0.300	42	Dry mass of sample, kg 5.0	
0.212	37		
0.150	34		
0.063	28		

Soil description	Brown slightly gravelly sandy CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	31	31
	Silt	41	41
	Clay	18	18
		10	10

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref

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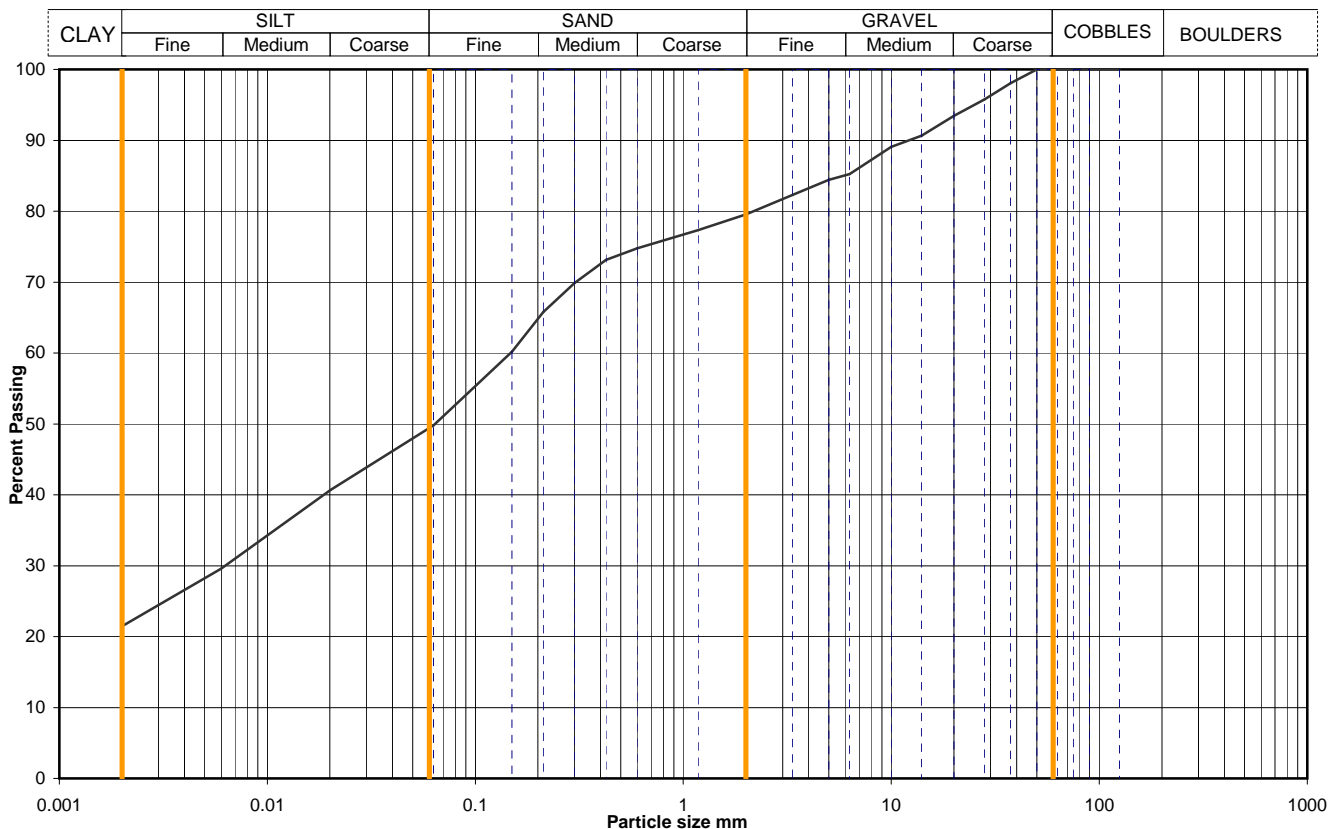
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Figure

PSD 27

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH4	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		2.00	
			Samp No	14	Type	B
			ID	ESGA1077-11201110110000000195		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	41
90	100	0.0060	30
75	100	0.0020	21
63	100		
50	100		
37.5	98		
28	96		
20	93		
14	91		
10	89		
6.3	85		
5.0	84		
3.35	82		
2.00	80		
1.18	77		
0.600	75		
0.425	73		
0.300	70		
0.212	66		
0.150	60		
0.063	50		

Particle density, Mg/m ³	
2.65	assumed
Dry mass of sample, kg	
10.8	

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	20	20
	Silt	30	30
	Clay	28	28
		22	22

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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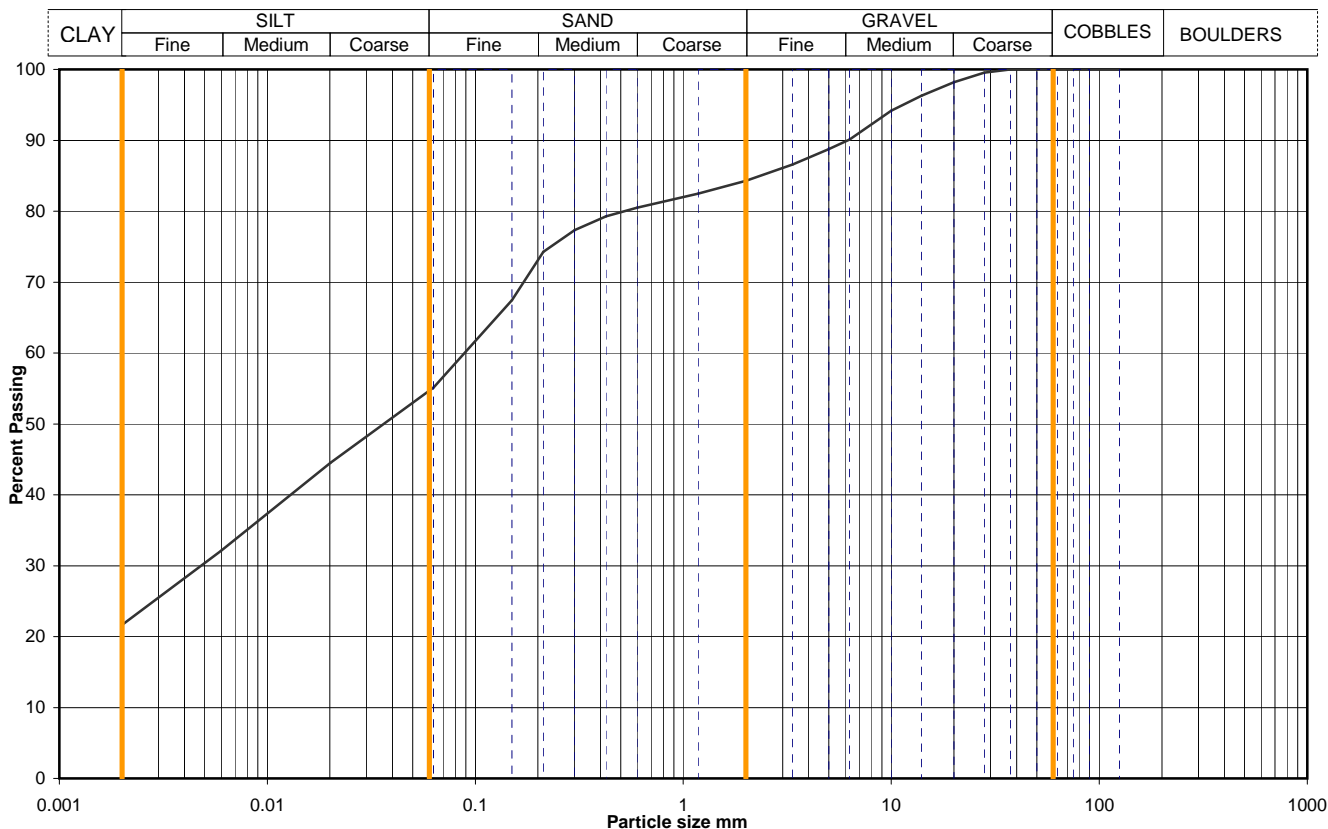
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Figure

PSD 28

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH4		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		4.00		
			Samp No	20	Type	B	
			ID	ESGA1077-11201110110000000201			
			Spec Ref				



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	45
90	100	0.0060	32
75	100	0.0020	22
63	100		
50	100		
37.5	100		
28	100		
20	98		
14	96		
10	94		
6.3	90		
5.0	89		
3.35	87		
2.00	84		
1.18	82		
0.600	80	Particle density, Mg/m ³ 2.65 assumed	
0.425	79		
0.300	77	Dry mass of sample, kg 9.1	
0.212	74		
0.150	67		
0.063	55		

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	16	16
	Silt	30	30
	Clay	33	33
		21	21

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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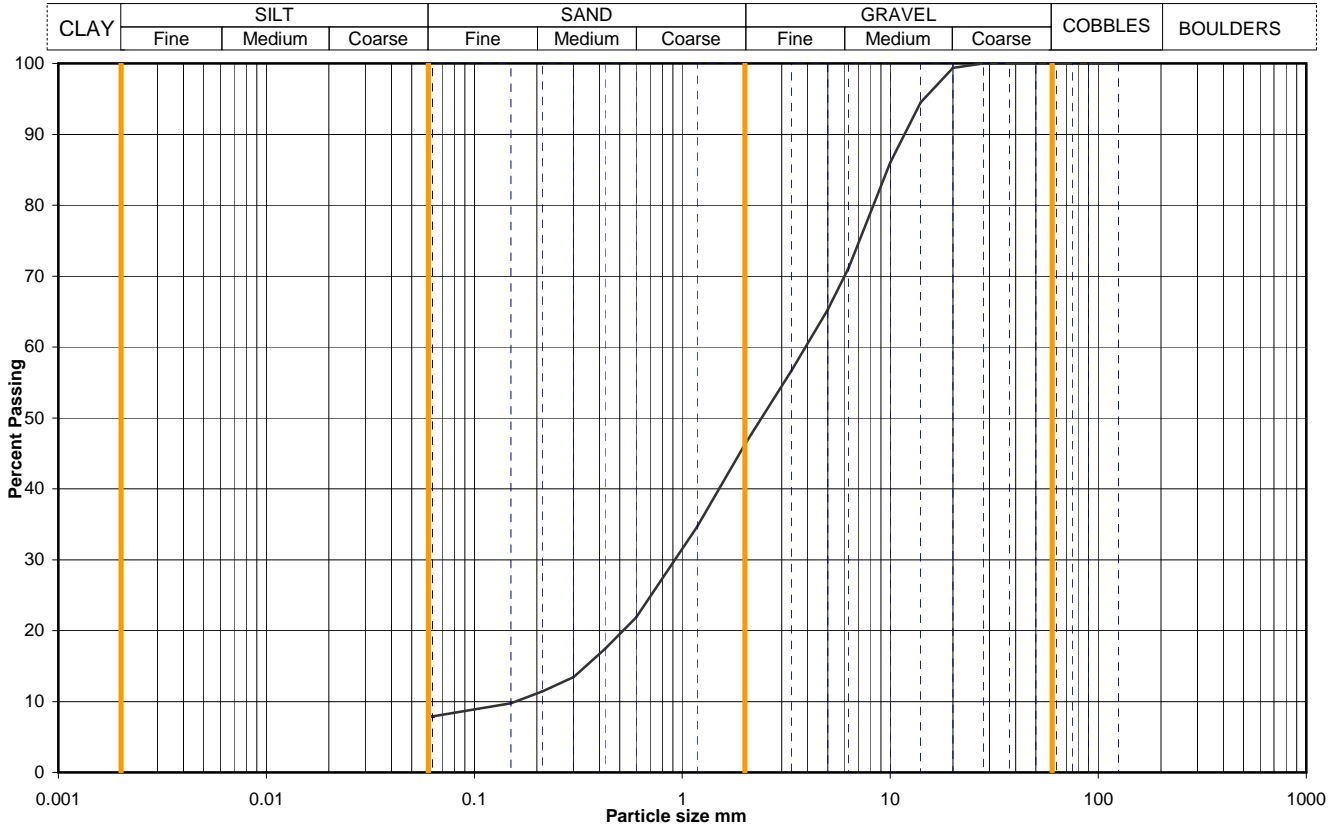
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Figure

PSD 29

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH5	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		0.10	
			Samp No	2	Type	B
			ID	ESGA1077-11201110130000000250		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	100		
37.5	100		
28	100		
20	99		
14	94		
10	86		
6.3	71		
5.0	65		
3.35	57		
2.00	46		
1.18	35		
0.600	22		
0.425	17		
0.300	13		
0.212	11		
0.150	10		
0.063	8		

Dry mass of sample, kg	
11.8	

Soil description	Dark grey very sandy clayey GRAVEL.		
Preparation / Pretreatment	Sieve: natural material		
Remarks			
Sample Proportions * < 60mm values to aid description only	Cobbles / boulders	Whole	* < 60mm
	Gravel	0	0
	Sand	54	54
	Silt	38	38
	Clay	silt+clay =	
		8	8

Uniformity Coefficient	D_{60} / D_{10}	25
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	none

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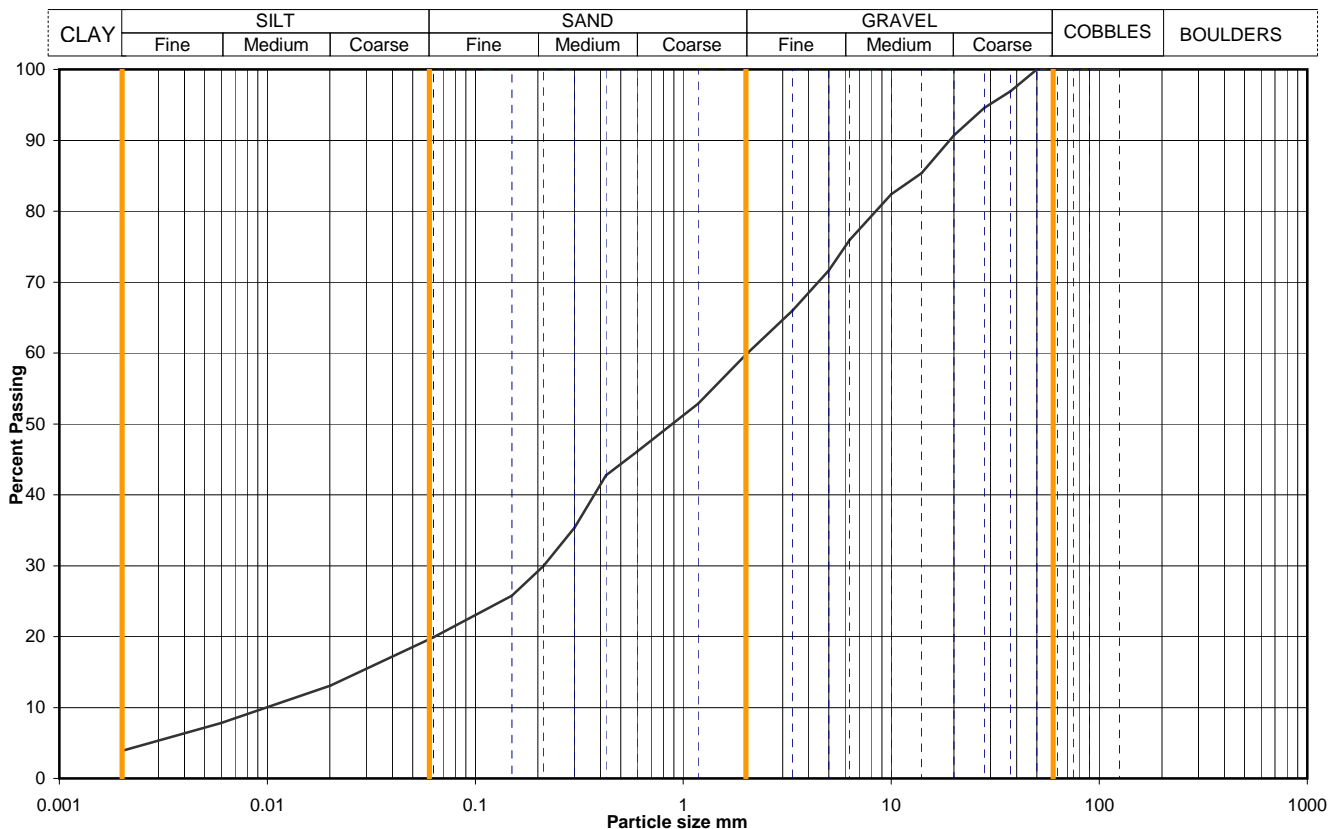


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Figure
PSD 30

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH5		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		0.90		
			Samp No	6	Type	B	
			ID	ESGA1077-11201110130000000254			
			Spec Ref				



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	13
90	100	0.0060	8
75	100	0.0020	4
63	100		
50	100		
37.5	97		
28	95		
20	91		
14	85		
10	82		
6.3	76		
5.0	72		
3.35	66		
2.00	60		
1.18	53		
0.600	46		
0.425	43		
0.300	35		
0.212	30		
0.150	26		
0.063	20		

Soil description	Grey sandy gravelly SILT.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders	Whole	*<60mm
	Gravel	0	0
	Sand	40	40
	Silt	40	40
	Clay	16	16
* <60mm values to aid description only		4	4

Uniformity Coefficient	D_{60} / D_{10}	206
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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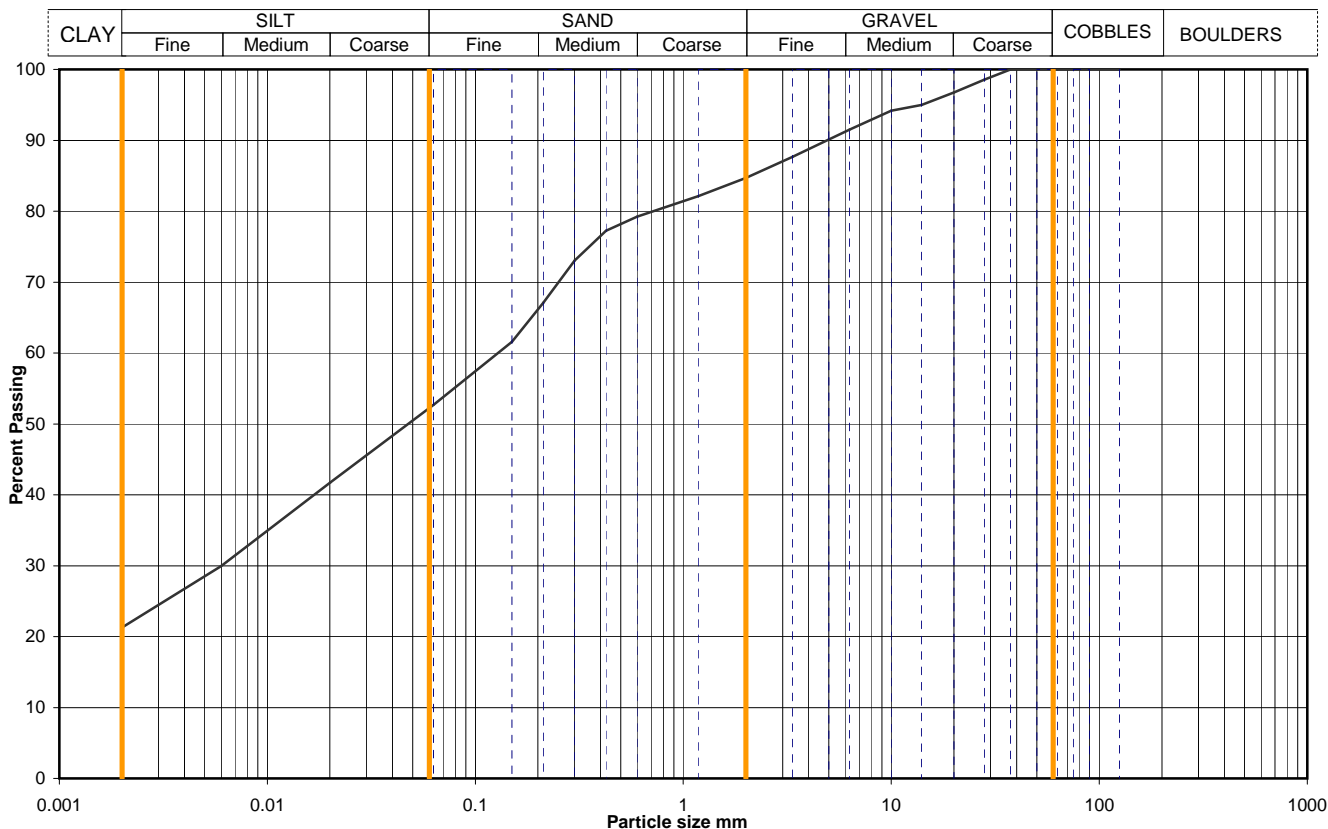
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Figure

PSD 31

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH5	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		1.20	
			Samp No	9	Type	B
			ID	ESGA1077-11201110130000000257		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	42
90	100	0.0060	30
75	100	0.0020	21
63	100		
50	100		
37.5	100		
28	99		
20	97		
14	95		
10	94		
6.3	92		
5.0	90		
3.35	88		
2.00	85		
1.18	82		
0.600	79	Particle density, Mg/m ³ 2.65 assumed	
0.425	77		
0.300	73	Dry mass of sample, kg 7.2	
0.212	67		
0.150	62		
0.063	53		

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	15	15
	Silt	33	33
	Clay	31	31

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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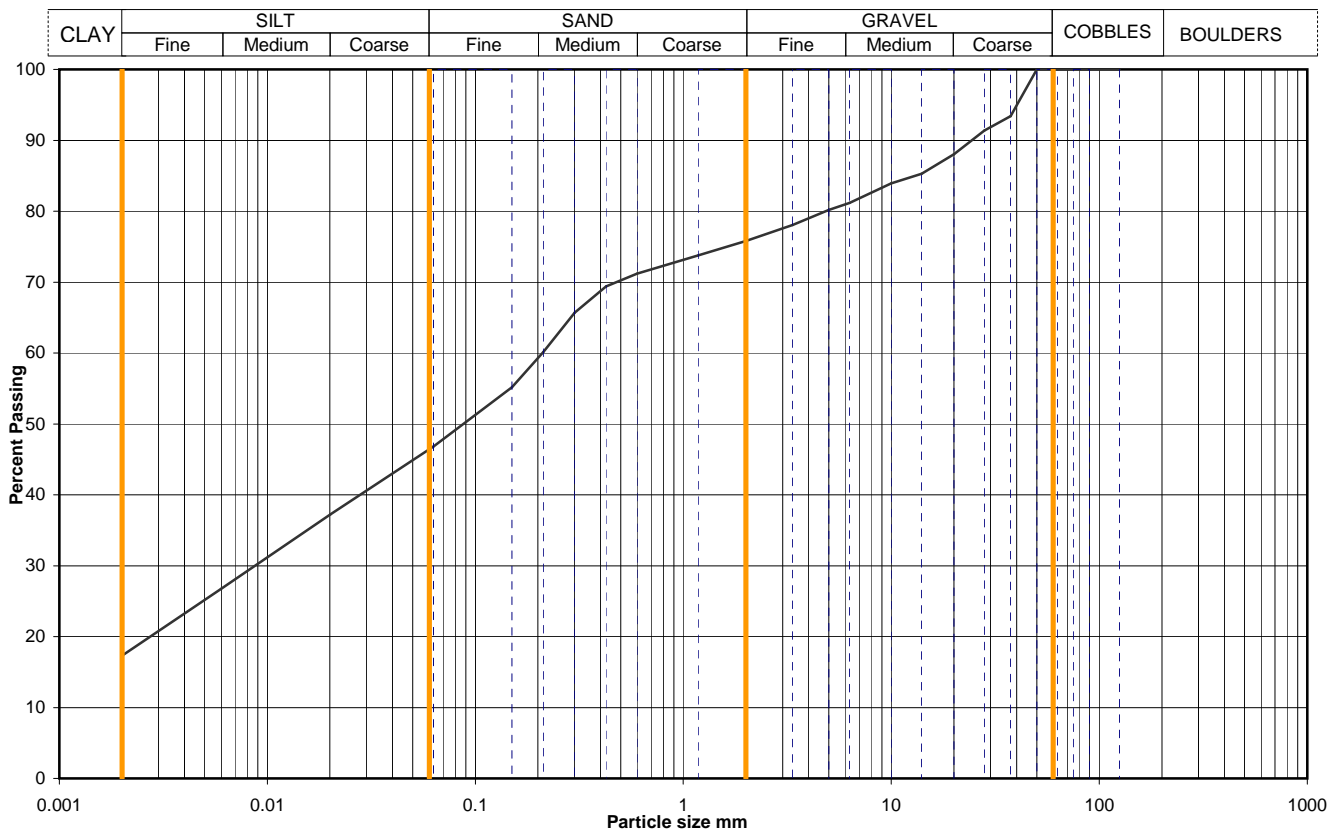
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Figure

PSD 32

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH5		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		2.00		
			Samp No	12	Type	B	
			ID	ESGA1077-11201110130000000260			
			Spec Ref				



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	37
90	100	0.0060	27
75	100	0.0020	17
63	100		
50	100		
37.5	93		
28	91		
20	88		
14	85		
10	84		
6.3	81		
5.0	80		
3.35	78		
2.00	76		
1.18	74		
0.600	71		
0.425	69		
0.300	66		
0.212	60		
0.150	55		
0.063	47		

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions *<60mm values to aid description only	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		24	24
		29	29
		29	29
		18	18

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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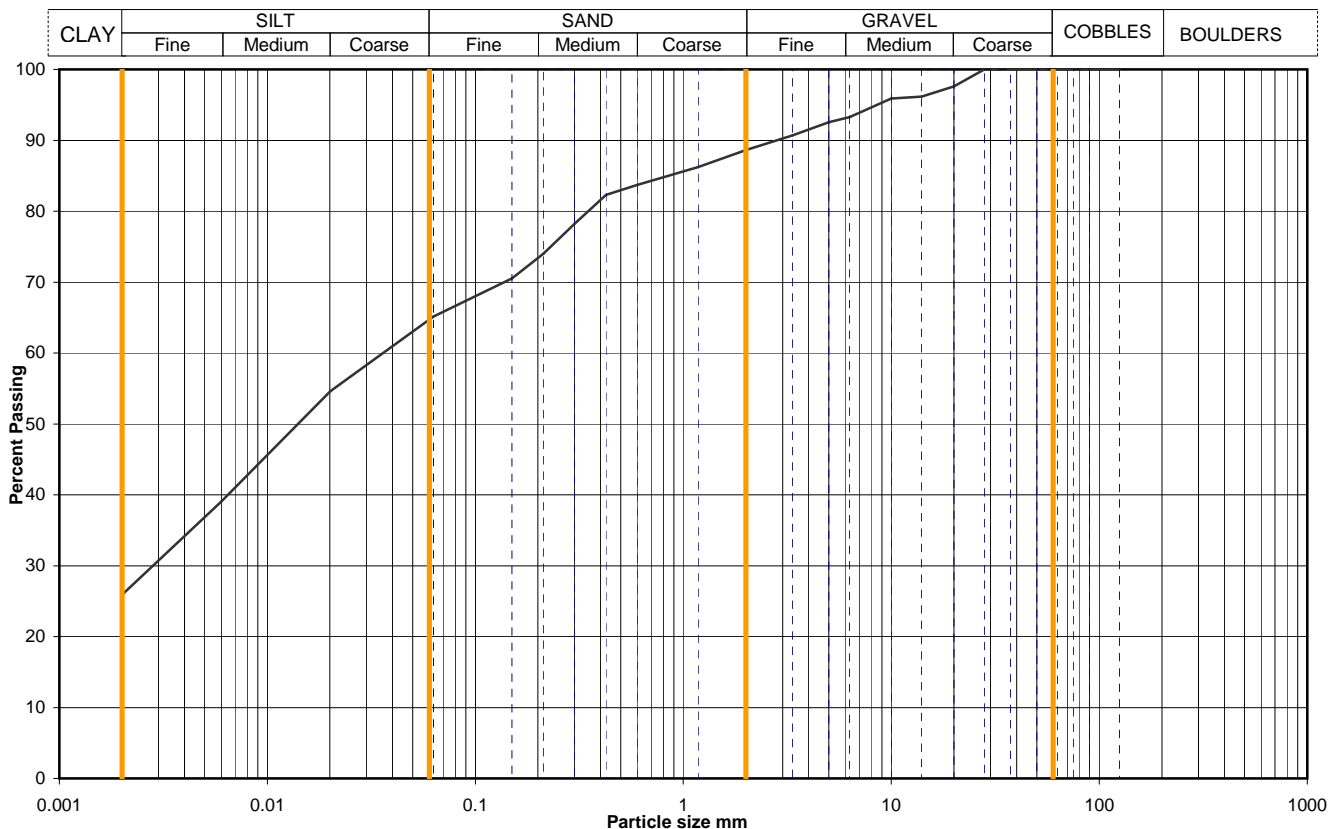
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Figure

PSD 33

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH5		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		3.50		
			Samp No	16	Type	B	
			ID	ESGA1077-11201110130000000264			
			Spec Ref				



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	55
90	100	0.0060	39
75	100	0.0020	26
63	100		
50	100		
37.5	100		
28	100		
20	98		
14	96		
10	96		
6.3	93		
5.0	93		
3.35	91		
2.00	89		
1.18	86		
0.600	84	Particle density, Mg/m ³ 2.65 assumed	
0.425	82		
0.300	78	Dry mass of sample, kg 3.6	
0.212	74		
0.150	71		
0.063	65		

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	11	11
	Silt	24	24
	Clay	39	39
		26	26

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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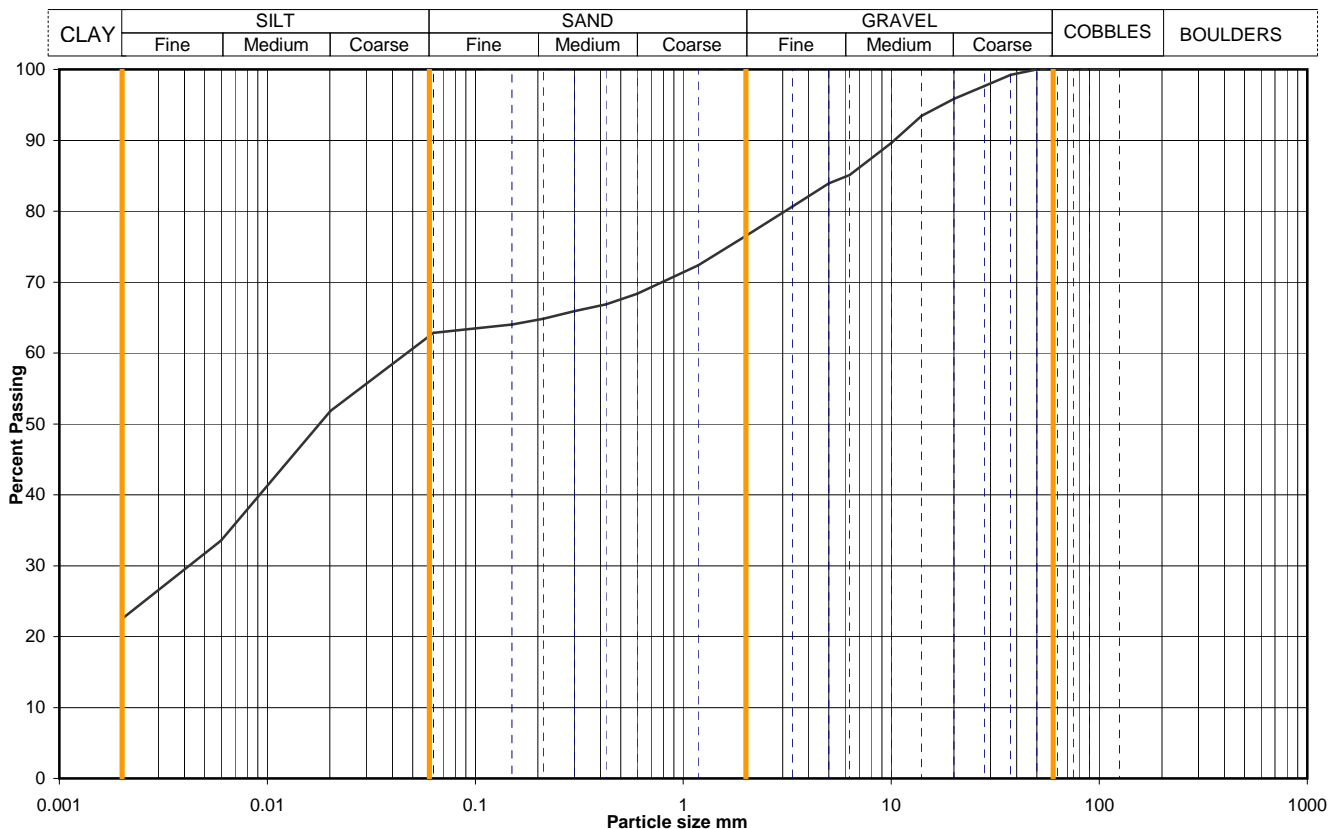
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Figure

PSD 34

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH5	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		4.00	
			Samp No	19	Type	B
			ID	ESGA1077-11201110130000000267		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	52
90	100	0.0060	34
75	100	0.0020	22
63	100		
50	100		
37.5	99		
28	98		
20	96		
14	93		
10	90		
6.3	85		
5.0	84		
3.35	81		
2.00	77		
1.18	72		
0.600	68	Particle density, Mg/m ³ 2.65 assumed	
0.425	67		
0.300	66	Dry mass of sample, kg 12.1	
0.212	65		
0.150	64		
0.063	63		

Soil description	Brownish grey slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	23	23
	Silt	14	14
	Clay	40	40
		23	23

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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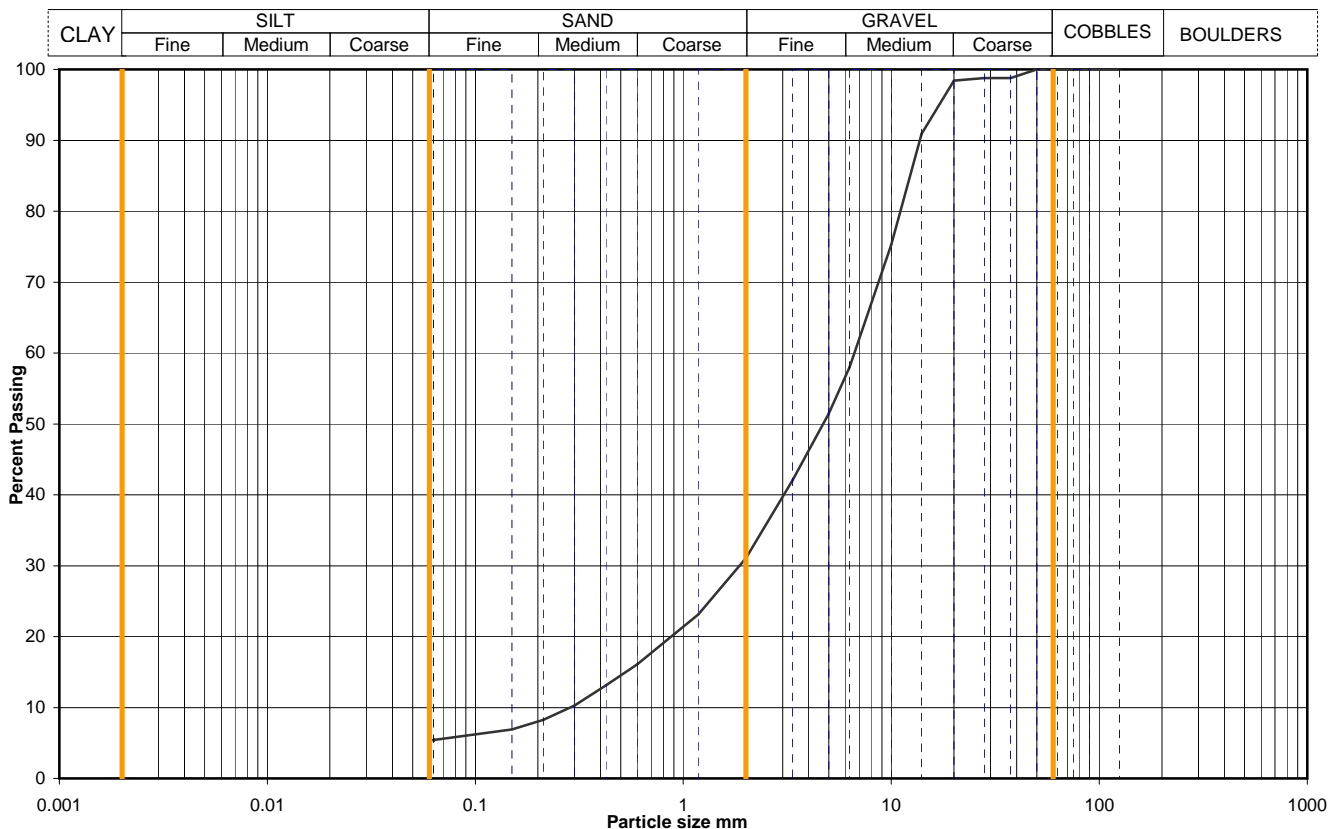
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Figure

PSD 35

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH6		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		0.10		
			Samp No	2	Type	B	
			ID	ESGA1077-11201110110000000207			
			Spec Ref				



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	100		
37.5	99		
28	99		
20	98		
14	91		
10	75		
6.3	58		
5.0	51		
3.35	42		
2.00	31		
1.18	23		
0.600	16		
0.425	13		
0.300	10		
0.212	8		
0.150	7		
0.063	5		
		Dry mass of sample, kg	
		15.4	

Soil description	Dark grey very sandy GRAVEL.		
Preparation / Pretreatment	Sieve: natural material		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	69	69
	Silt	26	26
	Clay	silt+clay =	
		5	5

Uniformity Coefficient	D_{60} / D_{10}	23
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	none

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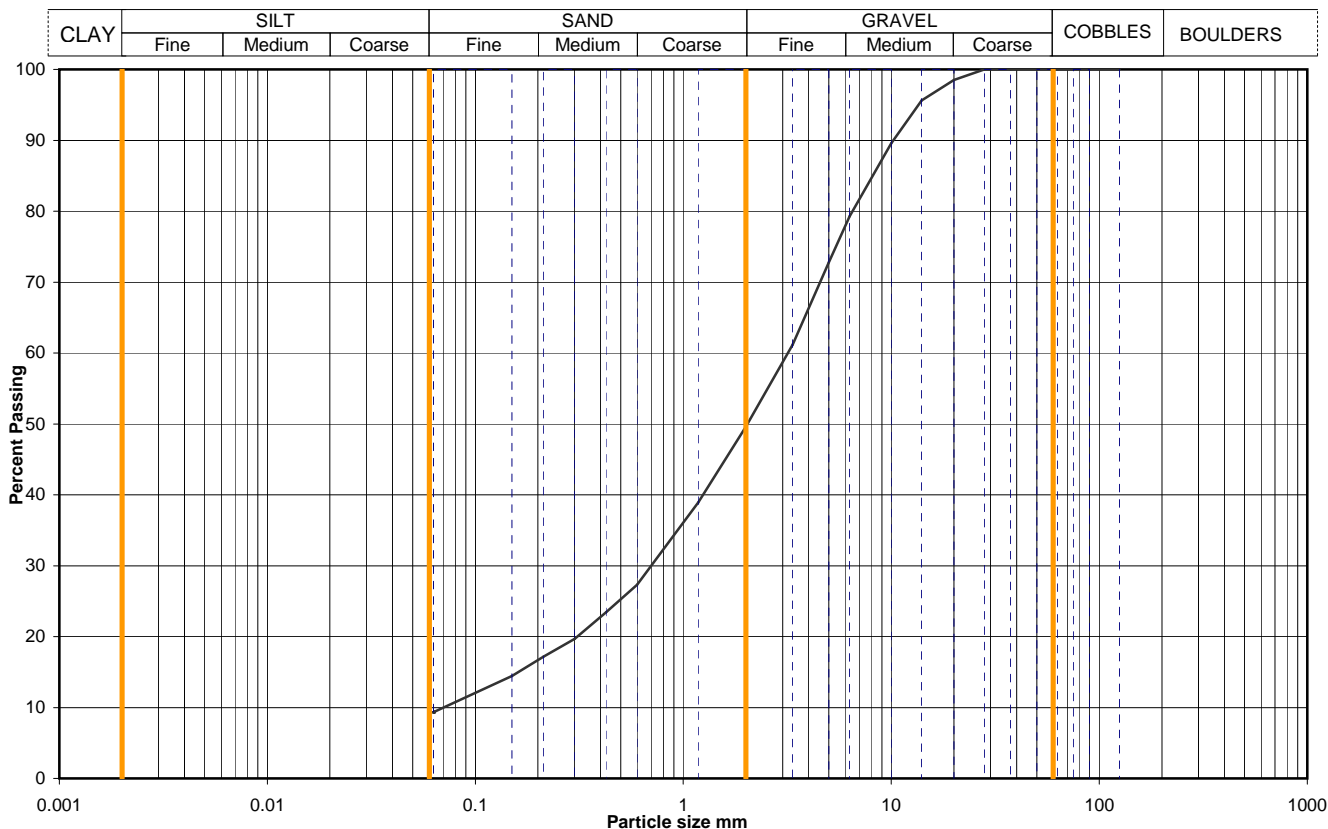


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Figure
PSD 36

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH6	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		0.40	
			Samp No	4	Type	B
			ID	ESGA1077-11201110110000000209		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	100		
37.5	100		
28	100		
20	98		
14	96		
10	90		
6.3	79		
5.0	73		
3.35	61		
2.00	50		
1.18	39		
0.600	27		
0.425	23		
0.300	20		
0.212	17		
0.150	14		
0.063	9		

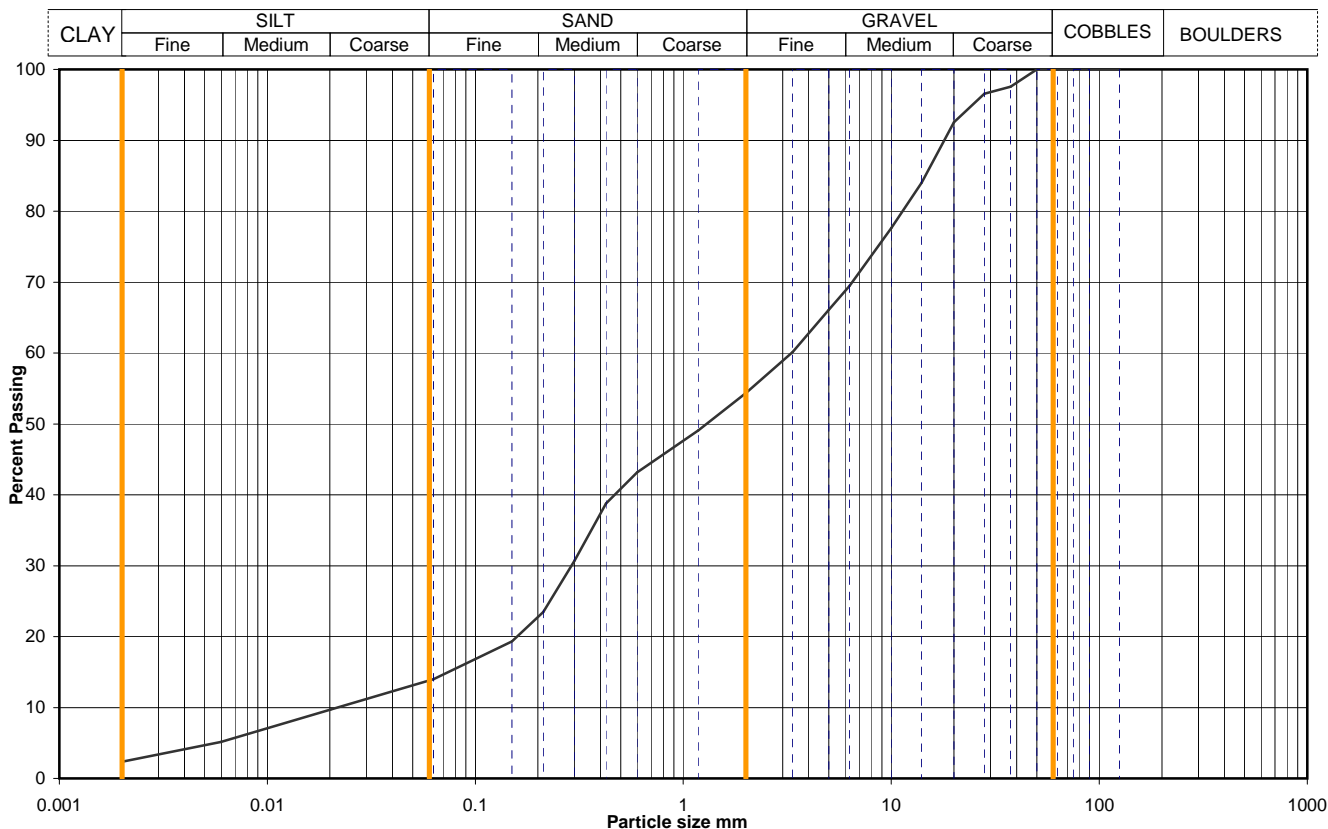
Soil description	Grey silty very sandy GRAVEL.		
Preparation / Pretreatment	Sieve: natural material		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	50	50
	Silt	40	40
	Clay	silt+clay =	

Uniformity Coefficient	D_{60} / D_{10}	45
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	none

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH6		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		0.80		
			Samp No	6	Type	B	
			ID	ESGA1077-11201110110000000211			
			Spec Ref				



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	10
90	100	0.0060	5
75	100	0.0020	2
63	100		
50	100		
37.5	98		
28	97		
20	93		
14	84		
10	78		
6.3	69		
5.0	66		
3.35	60		
2.00	54		
1.18	49		
0.600	43		
0.425	39		
0.300	31		
0.212	23		
0.150	19		
0.063	14		

Soil description	Dark grey sandy gravelly SILT.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions *<60mm values to aid description only	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		46	46
		41	41
		11	11
		2	2

Uniformity Coefficient	D_{60} / D_{10}	152
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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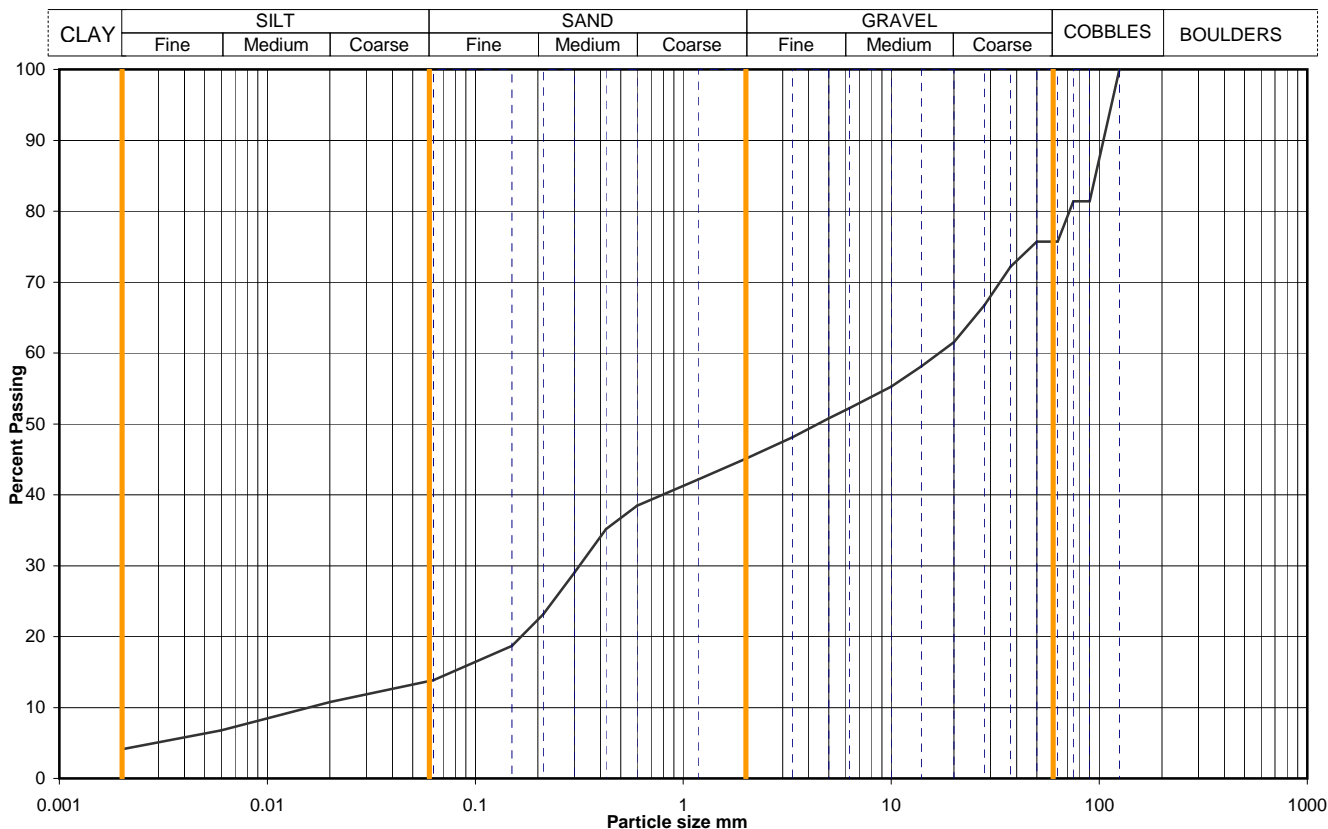
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Figure

PSD 38

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH6	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		1.20	
			Samp No	9	Type	B
			ID	ESGA1077-11201110110000000215		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	11
90	81	0.0060	7
75	81	0.0020	4
63	76		
50	76		
37.5	72		
28	67		
20	62		
14	58		
10	55		
6.3	52		
5.0	51		
3.35	48		
2.00	45		
1.18	42		
0.600	38	Particle density, Mg/m ³ 2.65 assumed	
0.425	35		
0.300	29	Dry mass of sample, kg 7.5	
0.212	23		
0.150	19		
0.063	14		

Soil description	Brown slightly sandy slightly gravelly CLAY with 2 cobbles.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	24	0
	Sand	31	41
	Silt	31	41
	Clay	10	13
		4	5

Uniformity Coefficient	D_{60} / D_{10}	1068
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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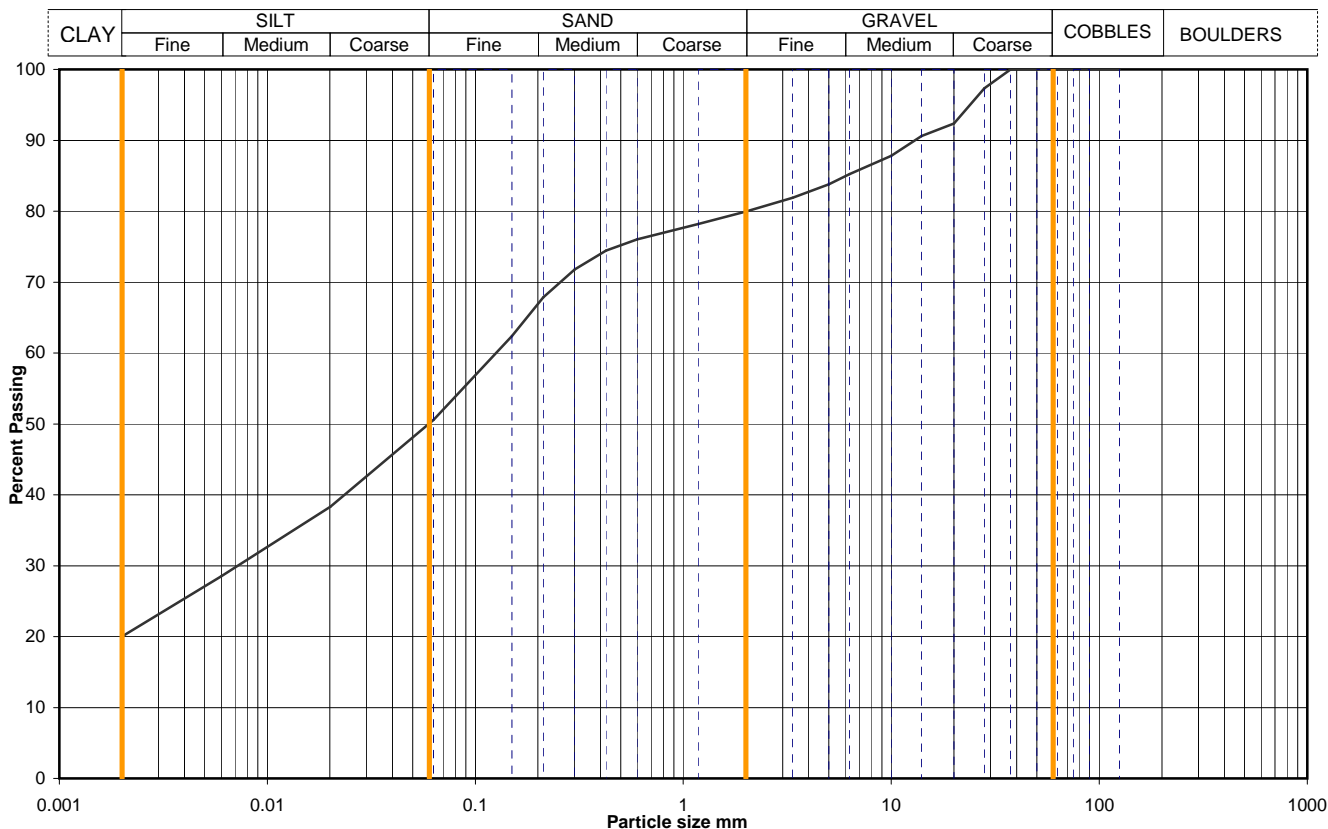
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Figure

PSD 39

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH6	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		2.00	
			Samp No	12	Type	B
			ID	ESGA1077-11201110110000000219		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	38
90	100	0.0060	29
75	100	0.0020	20
63	100		
50	100		
37.5	100		
28	97		
20	92		
14	91		
10	88		
6.3	85		
5.0	84		
3.35	82		
2.00	80		
1.18	78		
0.600	76	Particle density, Mg/m ³ 2.65 assumed	
0.425	74		
0.300	72	Dry mass of sample, kg 4.2	
0.212	68		
0.150	62		
0.063	51		

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	20	20
	Silt	30	30
	Clay	30	30

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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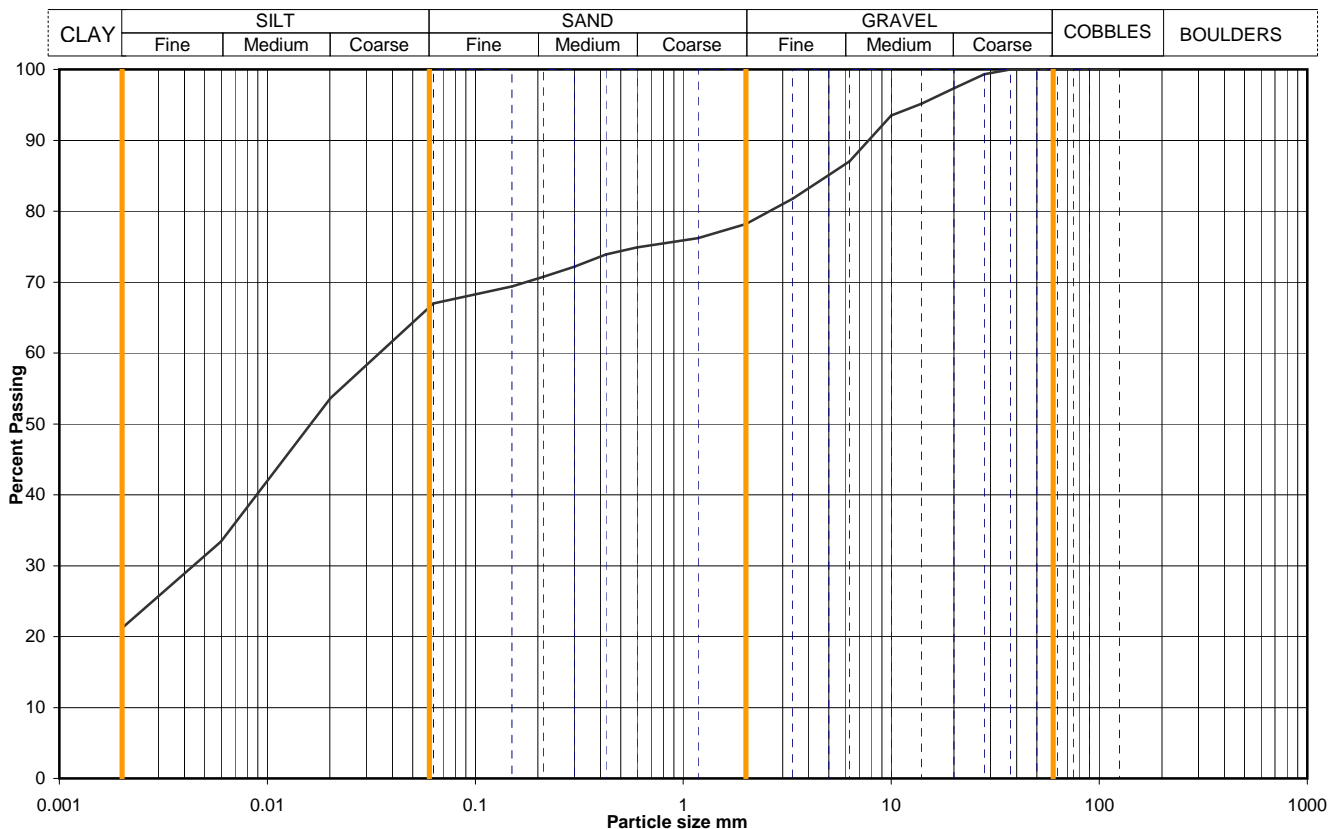
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Figure

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Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH6	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		3.50	
			Samp No	16	Type	B
			ID	ESGA1077-11201110110000000223		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	54
90	100	0.0060	33
75	100	0.0020	21
63	100		
50	100		
37.5	100		
28	99		
20	97		
14	95		
10	93		
6.3	87		
5.0	85		
3.35	82		
2.00	78		
1.18	76		
0.600	75	Particle density, Mg/m ³ 2.65 assumed	
0.425	74		
0.300	72	Dry mass of sample, kg 5.5	
0.212	71		
0.150	69		
0.063	67		

Soil description	Grey slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	22	22
	Silt	12	12
	Clay	45	45

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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Aug 11



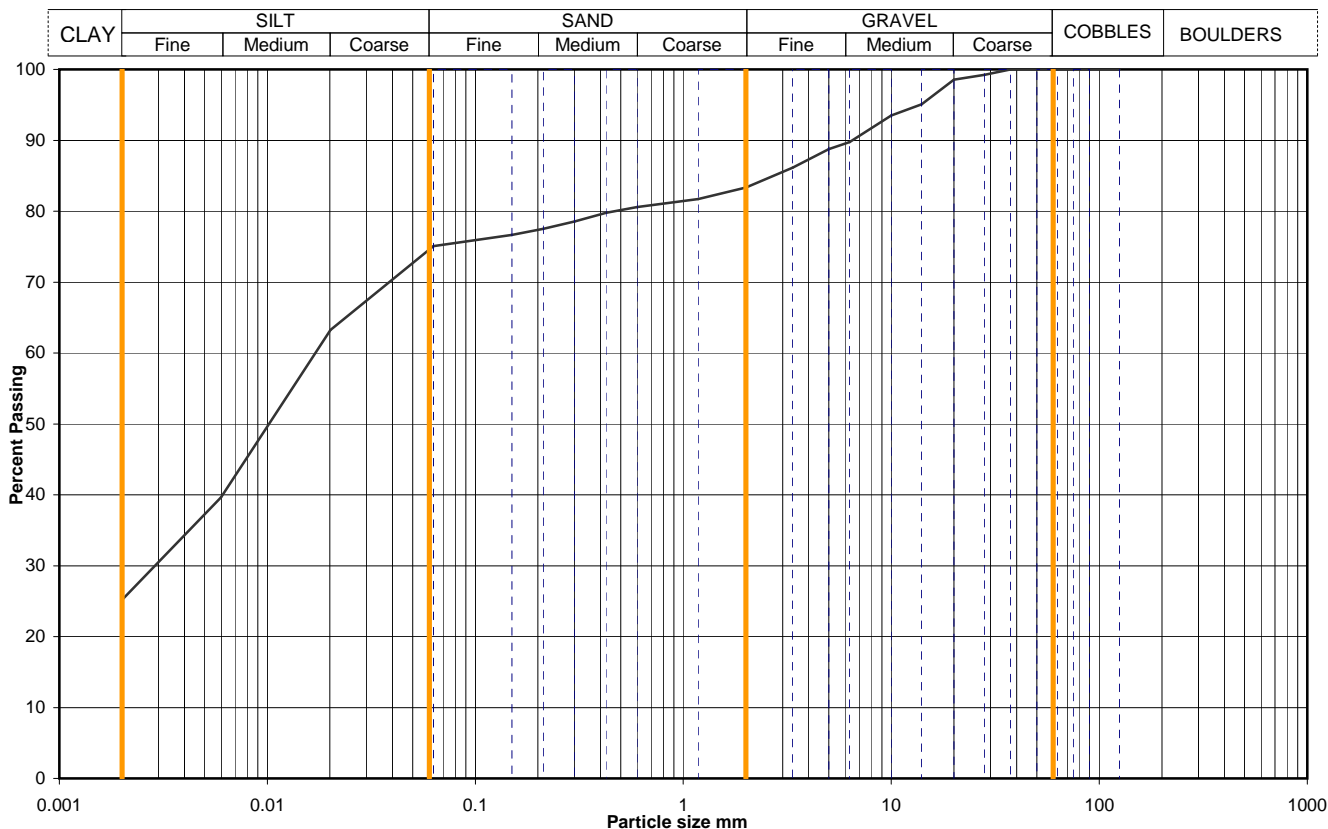
Printed: 18/11/2011 17:31

Figure

PSD 41

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		BH6		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		4.00		
			Samp No	19	Type	B	
			ID	ESGA1077-11201110110000000226			
			Spec Ref				



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	63
90	100	0.0060	40
75	100	0.0020	25
63	100		
50	100		
37.5	100		
28	99		
20	99		
14	95		
10	94		
6.3	90		
5.0	89		
3.35	86		
2.00	83		
1.18	82		
0.600	81	Particle density, Mg/m ³ 2.65 assumed	
0.425	80		
0.300	79	Dry mass of sample, kg 12.1	
0.212	78		
0.150	77		
0.063	75		

Soil description	Grey slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	17	17
	Silt	9	9
	Clay	49	49
		25	25

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref

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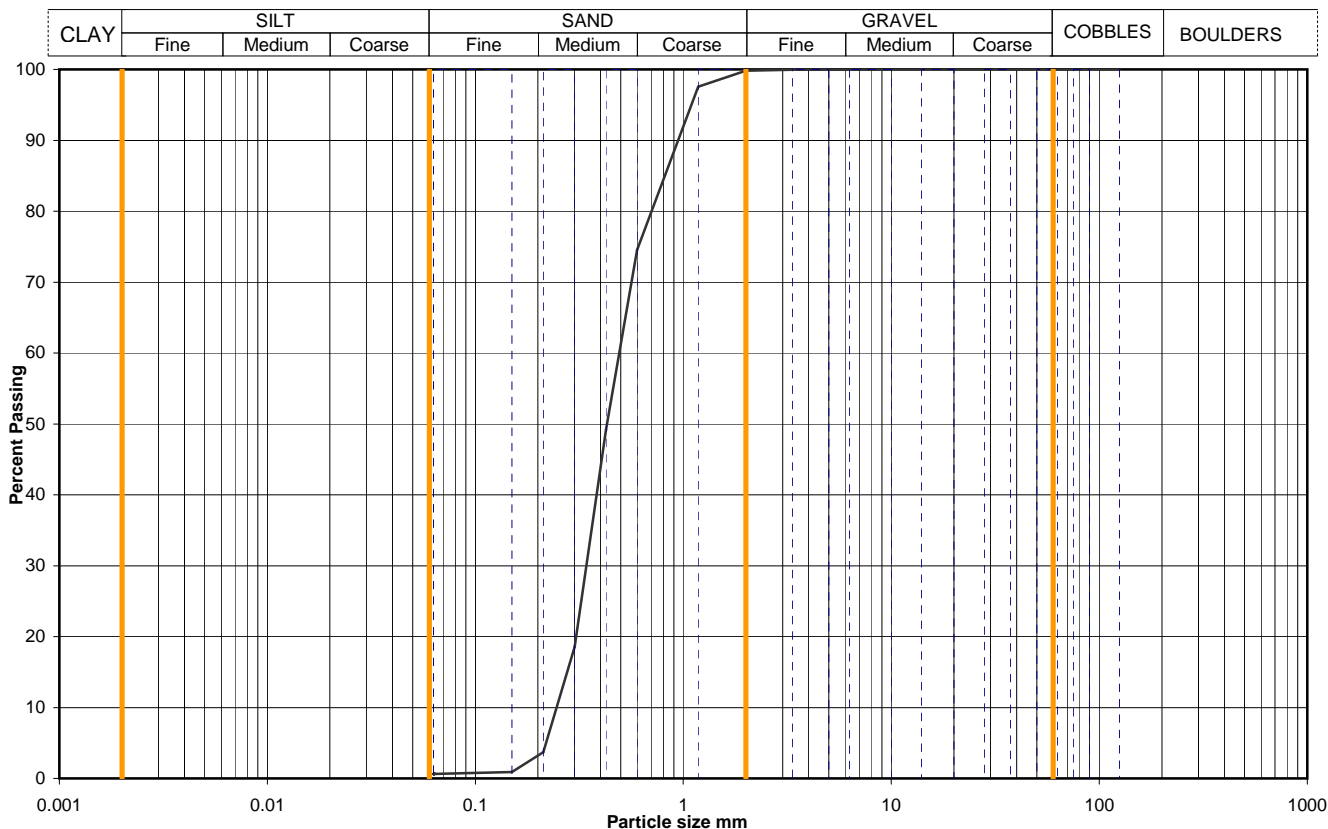
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Figure

PSD 42

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		WS1		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		0.30		
			Samp No	2	Type	B	
			ID	ESGA1077-11201110130000000229			
			Spec Ref				



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	100		
37.5	100		
28	100		
20	100		
14	100		
10	100		
6.3	100		
5.0	100		
3.35	100		
2.00	100		
1.18	98		
0.600	75		
0.425	49		
0.300	19		
0.212	4		
0.150	1		
0.063	1		

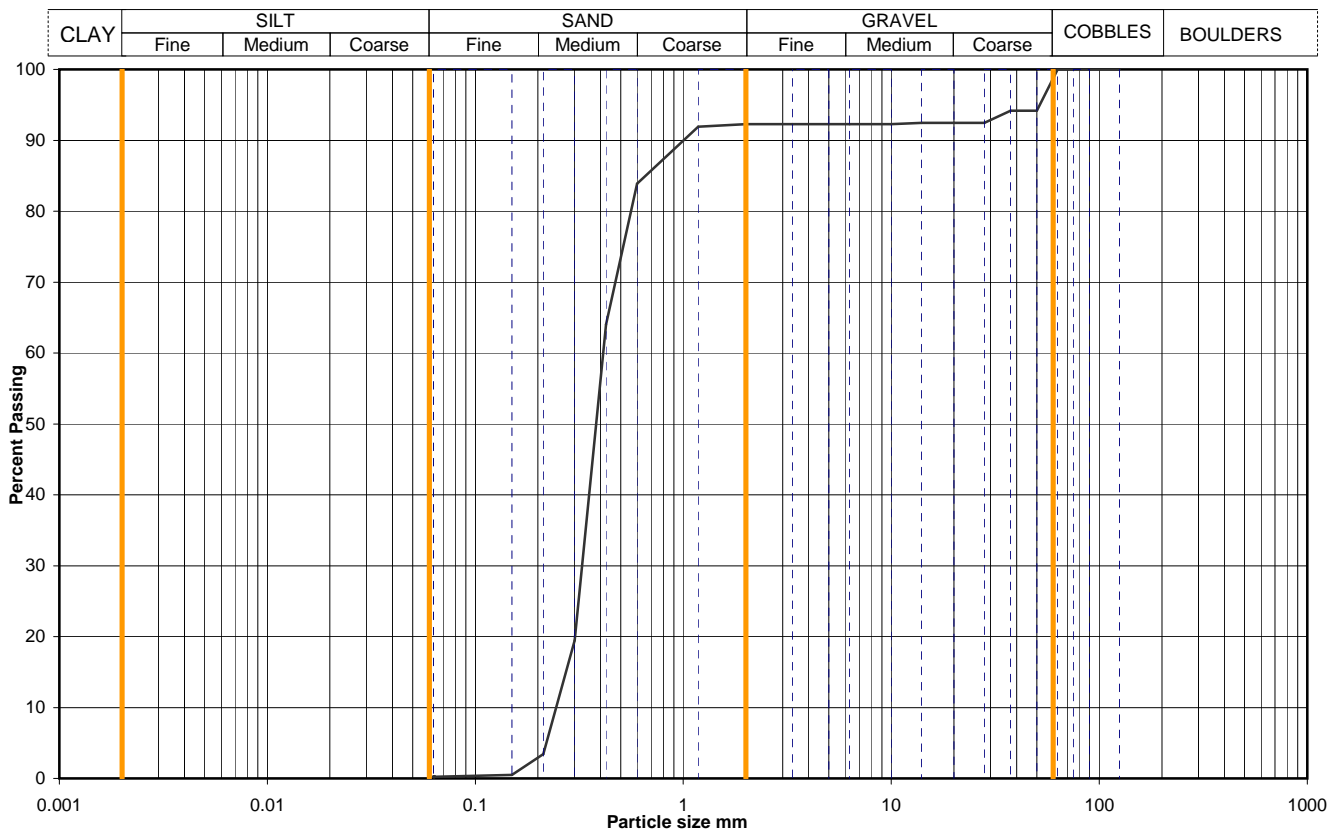
Soil description	Brown SAND.		
Preparation / Pretreatment	Sieve: natural material		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	0	0
	Silt	99	99
	Clay	silt+clay =	

Uniformity Coefficient	D_{60} / D_{10}	2
------------------------	-------------------	---

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	none

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		WS2		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		0.30		
			Samp No	2	Type	B	
			ID	ESGA1077-11201110130000000270			
			Spec Ref				



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	94		
37.5	94		
28	92		
20	92		
14	92		
10	92		
6.3	92		
5.0	92		
3.35	92		
2.00	92		
1.18	92		
0.600	84		
0.425	64		
0.300	19		
0.212	3		
0.150	0		
0.063	0		

Soil description	Brownish grey gravelly SAND.		
Preparation / Pretreatment	Sieve: natural material		
Remarks			
Sample Proportions * < 60mm values to aid description only	Cobbles / boulders	Whole	* < 60mm
	Gravel	1	0
	Sand	7	7
	Silt	92	93
	Clay	silt+clay =	
		0	0

Uniformity Coefficient	D_{60} / D_{10}	2
------------------------	-------------------	---

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	none

QA Ref
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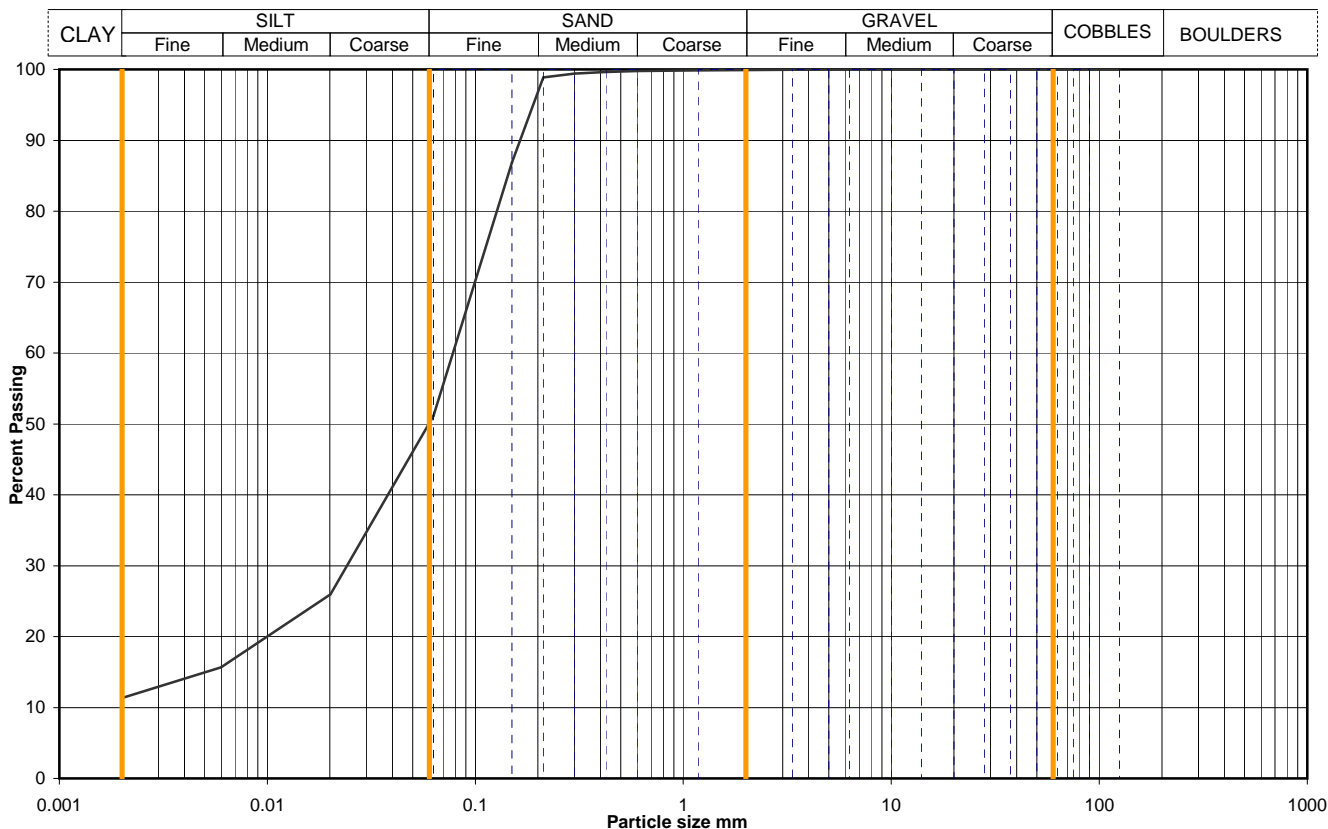


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Figure
PSD 44

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		WS2		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		2.80		
			Samp No	7	Type	D	
			ID	ESGA1077-11201110130000000275			
			Spec Ref				



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	26
90	100	0.0060	16
75	100	0.0020	11
63	100		
50	100		
37.5	100		
28	100		
20	100		
14	100		
10	100		
6.3	100		
5.0	100		
3.35	100		
2.00	100		
1.18	100		
0.600	100	Particle density, Mg/m ³	
0.425	100	2.65 assumed	
0.300	99	Dry mass of sample, kg	
0.212	99	1.1	
0.150	87		
0.063	51		

Soil description	Grey sandy clayey SILT.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	0	0
	Silt	50	50
	Clay	39	39

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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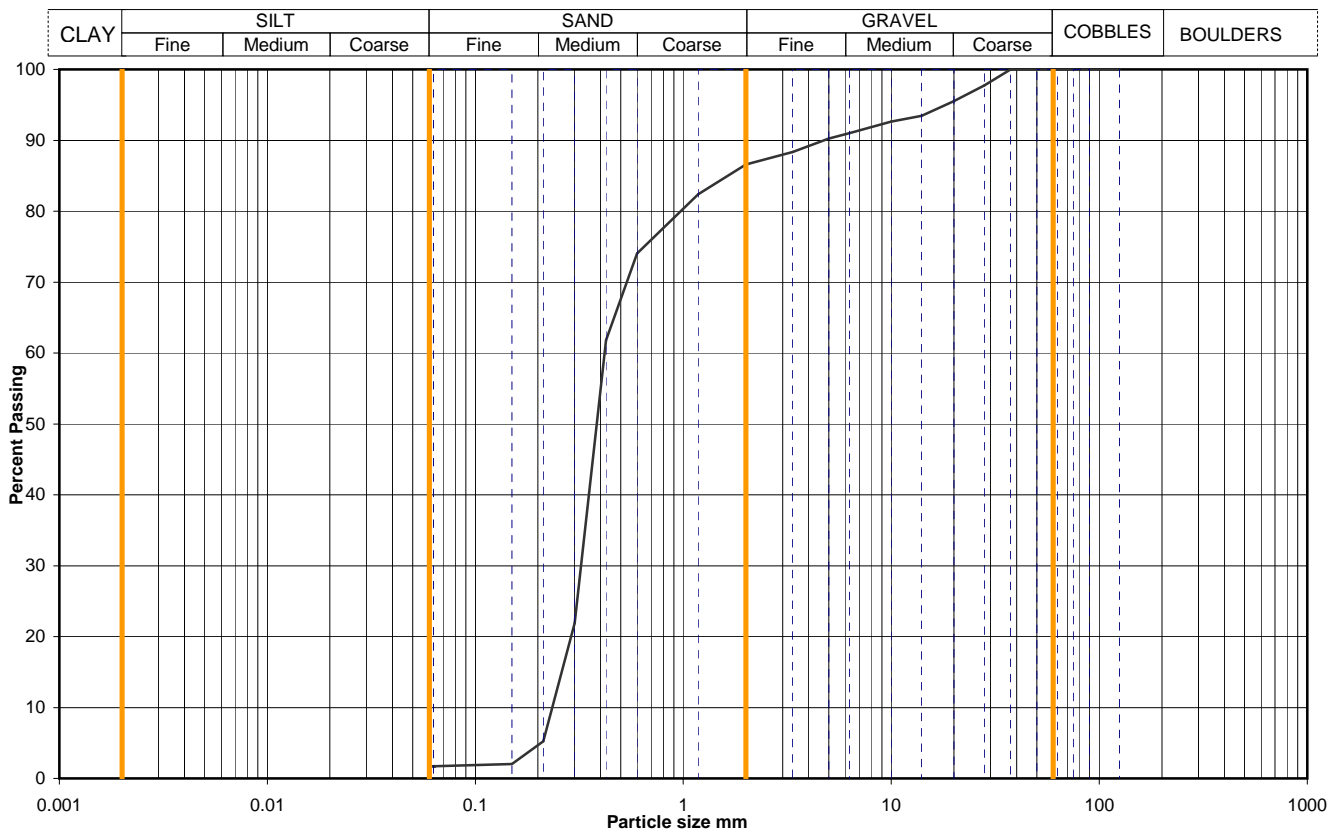
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Figure

PSD 45

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		WS3	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		0.30	
			Samp No	2	Type	B
			ID	ESGA1077-11201110130000000232		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	100		
37.5	100		
28	98		
20	96		
14	93		
10	93		
6.3	91		
5.0	90		
3.35	88		
2.00	87		
1.18	82		
0.600	74		
0.425	62		
0.300	22		
0.212	5		
0.150	2		
0.063	2		

Soil description	Brown gravelly SAND.		
Preparation / Pretreatment	Sieve: natural material		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	13	13
	Silt	85	85
	Clay	silt+clay =	

Uniformity Coefficient	D_{60} / D_{10}	2
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	none

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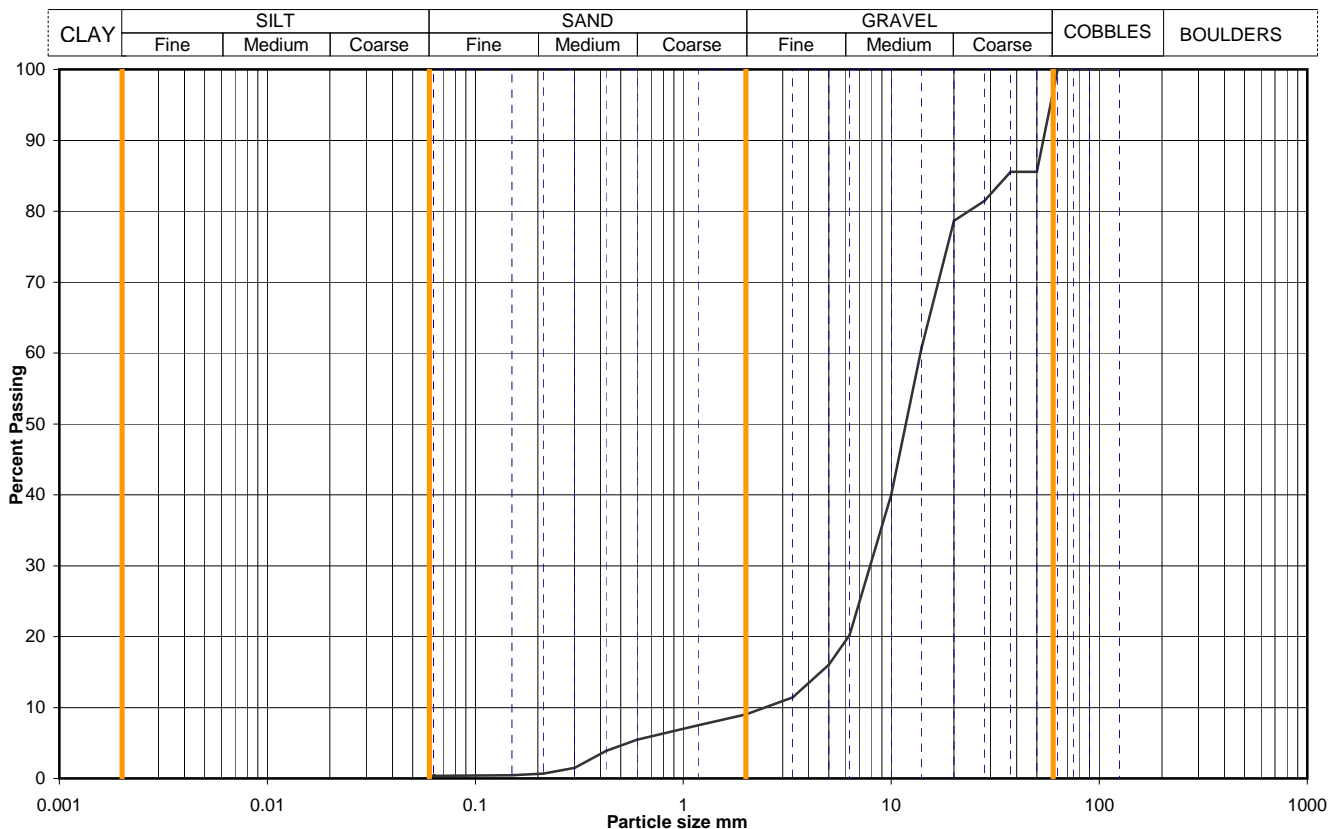


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Figure
PSD 46

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		WS3		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		1.80		
			Samp No	4	Type	D	
			ID	ESGA1077-11201110130000000234			
			Spec Ref				



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	86		
37.5	86		
28	81		
20	79		
14	61		
10	40		
6.3	20		
5.0	16		
3.35	11		
2.00	9		
1.18	8		
0.600	5		
0.425	4		
0.300	1		
0.212	1		
0.150	0		
0.063	0		

Dry mass of sample, kg			
1.8			

Soil description	Brownish grey sandy GRAVEL.		
Preparation / Pretreatment	Sieve: pre dried,		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	3	0
	Sand	88	91
	Silt	9	9
	Clay	silt+clay =	
		0	0

Uniformity Coefficient	D_{60} / D_{10}	6
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	none

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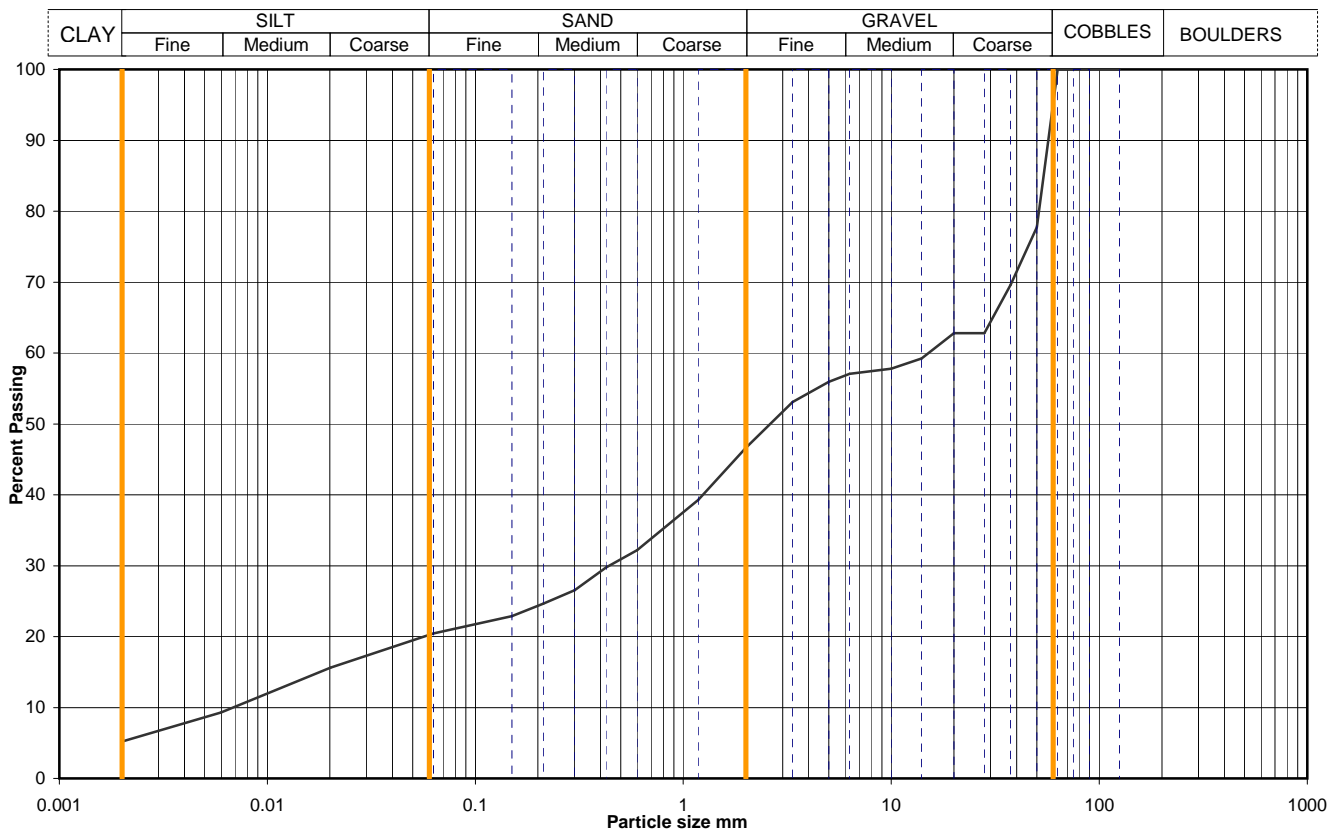
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Figure

PSD 47

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		WS3	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		2.50	
			Samp No	5	Type	D
			ID	ESGA1077-11201110130000000235		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	16
90	100	0.0060	9
75	100	0.0020	5
63	100		
50	78		
37.5	70		
28	63		
20	63		
14	59		
10	58		
6.3	57		
5.0	56		
3.35	53		
2.00	47		
1.18	39		
0.600	32		
0.425	30		
0.300	27		
0.212	25		
0.150	23		
0.063	20		

Particle density, Mg/m ³	
2.65	assumed
Dry mass of sample, kg	
1.0	

Soil description	Brownish grey slightly sandy gravelly CLAY.		
Preparation / Pretreatment	Sieve: pre dried, Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders	Whole	*<60mm
	Gravel	5	0
	Sand	48	51
	Silt	26	27
	Clay	15	16
* <60mm values to aid description only		6	6

Uniformity Coefficient	D_{60} / D_{10}	2203
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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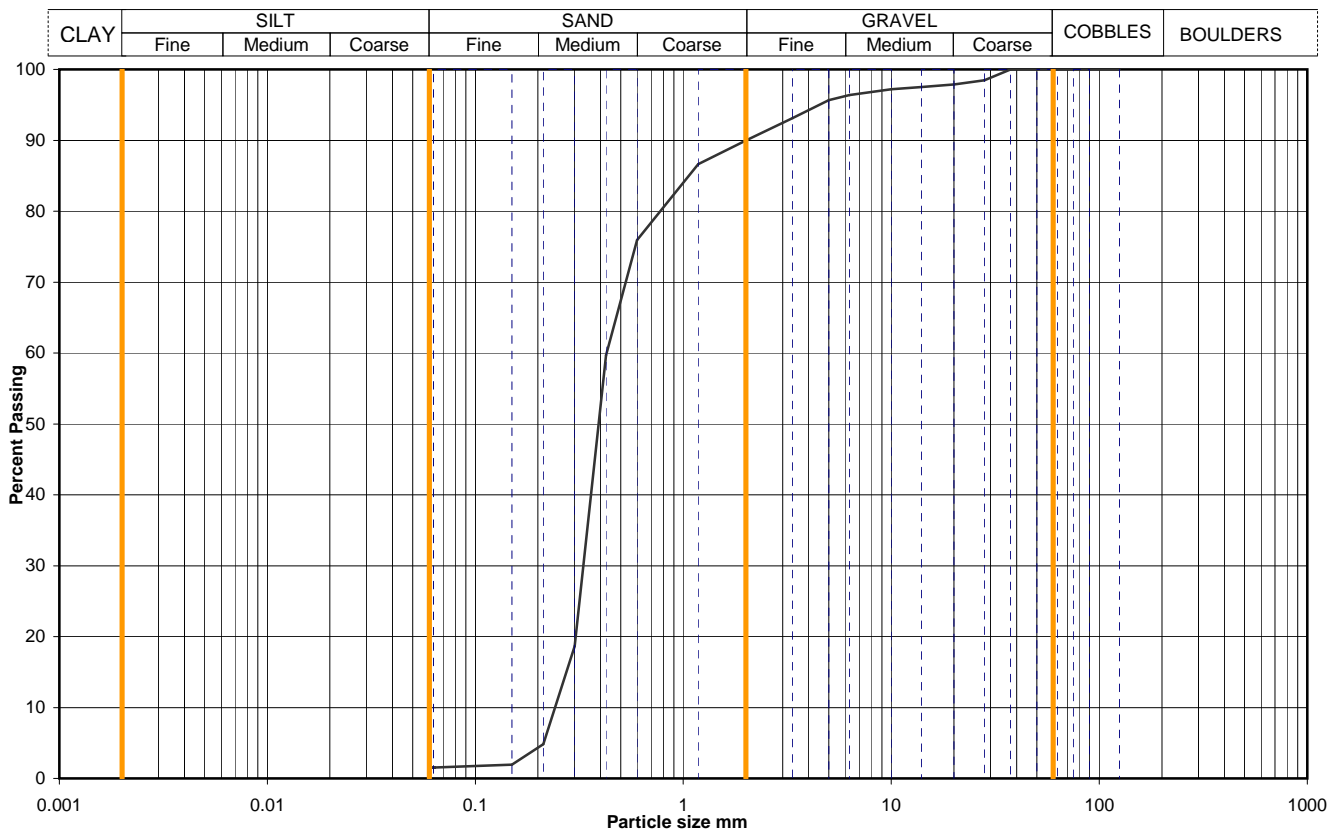
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Figure

PSD 48

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		WS5		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		0.60		
			Samp No	3	Type	B	
			ID	ESGA1077-11201110130000000241			
			Spec Ref				



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	100		
37.5	100		
28	98		
20	98		
14	97		
10	97		
6.3	96		
5.0	96		
3.35	93		
2.00	90		
1.18	87		
0.600	76		
0.425	60		
0.300	19		
0.212	5		
0.150	2		
0.063	2		

Dry mass of sample, kg		
		3.7

Soil description	Brown gravelly SAND.		
Preparation / Pretreatment	Sieve: natural material		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	10	10
	Silt	88	88
	Clay	silt+clay =	
		2	2

Uniformity Coefficient	D_{60} / D_{10}	2
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	none

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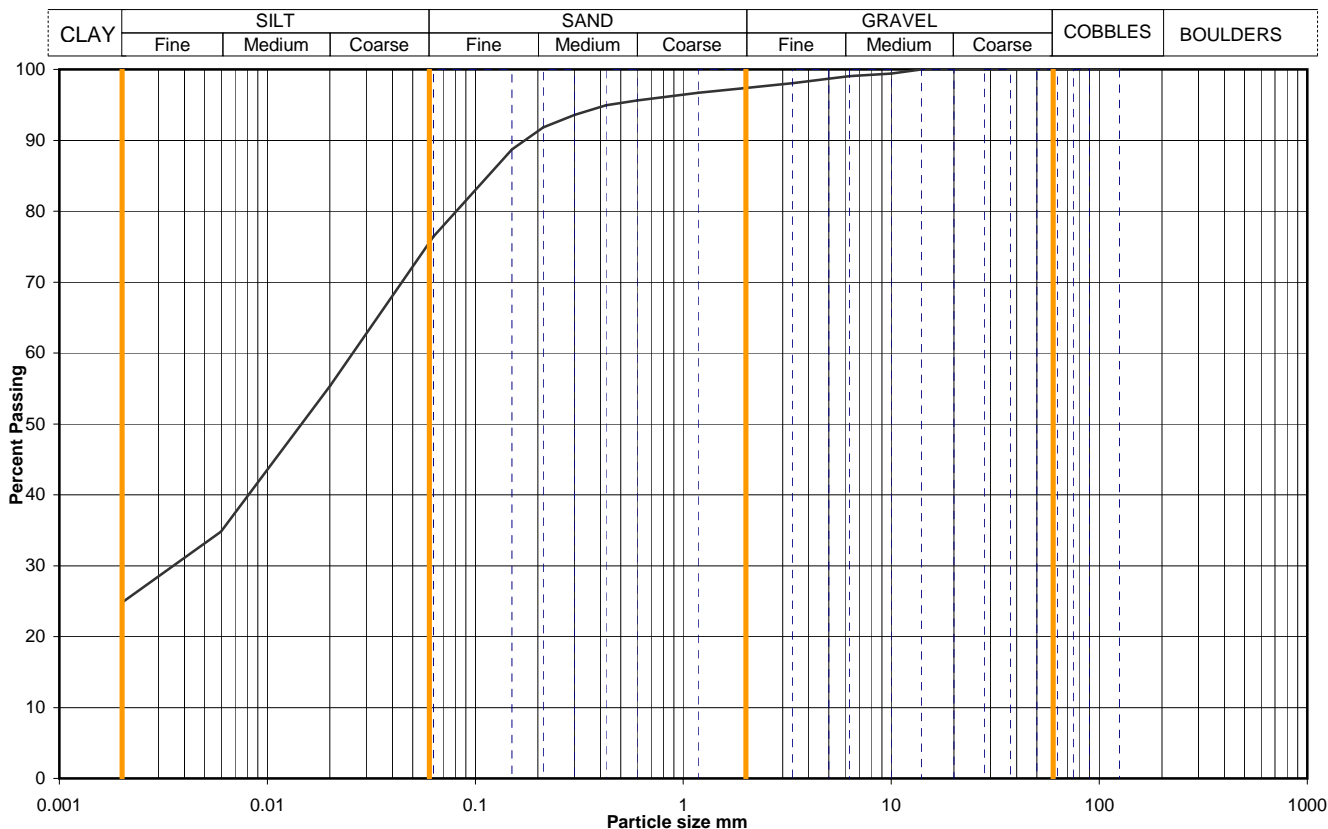
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Figure

PSD 49

Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No		WS5		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		3.00		
			Samp No	6	Type	D	
			ID	ESGA1077-11201110130000000245			
			Spec Ref				



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	55
90	100	0.0060	35
75	100	0.0020	25
63	100		
50	100		
37.5	100		
28	100		
20	100		
14	100		
10	99		
6.3	99		
5.0	99		
3.35	98		
2.00	97		
1.18	97		
0.600	96	Particle density, Mg/m ³ 2.65 assumed	
0.425	95		
0.300	94	Dry mass of sample, kg 1.0	
0.212	92		
0.150	89		
0.063	76		

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions * <60mm values to aid description only	Cobbles / boulders	Whole	* <60mm
	Gravel	0	0
	Sand	3	3
	Silt	22	22
	Clay	50	50
		25	25

Uniformity Coefficient	D_{60} / D_{10}	Not applicable
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Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

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Figure
PSD 50

UNCONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TESTS WITHOUT MEASUREMENT OF PORE PRESSURE - SUMMARY OF RESULTS

Project No	Project Name
A1077-11	SANDSEND BOREHOLES, NORTH YORKSHIRE

Hole No.	Sample				Soil Description	Density		w	Test type	Dia.	σ ₃	At failure / end of stage				Remarks
	No.	Depth (m)		type		bulk	dry					Axial strain	σ ₁ - σ ₃	C _u	M O D E	
		from	to													
BH1	9	3.00	3.45	U	Firm to stiff greyish brown slightly sandy slightly gravelly CLAY.	2.18	1.89	15	UUM	102.6 102.6 102.6	50 70 90	5.9 7.9 19.7	115 127 154	58 64 77	P	
BH1	25	9.50	9.95	U	Firm to stiff brown slightly sandy slightly gravelly CLAY.	2.24	1.95	15	UUM	101.1 101.1 101.1	140 200 250	6.4 8.8 19.2	109 123 148	54 62 74	P	
BH1	45	17.00	17.45	U	Stiff brown slightly sandy slightly gravelly CLAY.	2.18	1.86	17	UUM	101.9 101.9 101.9	240 350 430	12.3 14.3 18.8	300 312 324	150 156 162	P	
BH1	56	21.00	21.45	U	Stiff to very stiff brown slightly sandy slightly gravelly CLAY.	2.20	1.94	13	UUM	102.6 102.6 102.6	300 430 530	19.6	385 0 0	193 0 0	P	20% axial strain achieved at 300kPa
BH2	8	3.00	3.45	U	Stiff greyish brown slightly sandy slightly gravelly CLAY.	2.16	1.90	14	UUM	102.4 102.4 102.4	50 70 90	10.4 14.3 19.7	350 378 397	175 189 199	P	
BH2	16	7.00	7.45	U	Stiff brown slightly sandy slightly gravelly CLAY.	2.15	1.87	15	UUM	101.9 101.9 101.9	105 150 180	9.8 12.3 19.2	175 188 202	88 94 101	C	
BH2	34	16.00	16.45	U	Stiff brown slightly sandy slightly gravelly CLAY.	2.11	1.74	21	UUM	102.7 102.7 102.7	230 330 410	7.9 9.9 12.4	148 160 169	74 80 84	C	
BH2	47	21.00	21.45	U	Stiff greyish brown slightly sandy slightly gravelly CLAY.	2.13	1.87	14	UUM	102.3 102.3 102.3	300 430 430	16.7 19.7 19.7	277 297 297	139 148 148	P	20% axial strain achieved at 430kPa
BH2	53	24.00	24.45	U	Stiff brown slightly sandy slightly gravelly CLAY.	2.16	1.90	14	UUM	101.6 101.6 101.6	340 490 600	16.8 19.7 19.7	366 385 0	183 193 0	P	20% axial strain achieved at 490kPa
BH3	8	2.50	2.95	U	Firm to very stiff brown slightly sandy slightly gravelly CLAY.	2.15	1.85	16	UUM	102.3 102.3 102.3	40 60 75	9.9 12.9 19.3	371 392 408	186 196 204	C	
BH3	16	5.50	5.95	U	Firm brown slightly sandy slightly gravelly CLAY.	2.17	1.86	17	UUM	103.2 103.2 103.2	85 120 150	4.9 6.4 19.8	83 92 124	41 46 62	P	
BH3	39	14.50	14.95	U	Firm to stiff greyish brown slightly sandy slightly gravelly CLAY.	2.21	1.95	14	UUM	103.1 103.1 103.1	150 185 205	19.7	270 0 0	135 0 0	P	20% axial strain achieved at 150kPa
BH3	51	19.00	19.45	U	Stiff greyish brown slightly sandy slightly gravelly CLAY.	2.23	1.97	13	UUM	102.7 102.7 102.7	270 390 485	19.7	461 0 0	231 0 0	P	20% axial strain achieved at 270kPa

General notes: Tests carried out in accordance with BS1377: Part 7: 1990, clause 8 for single stage, clause 9 for multistage tests. Specimens nominally 2:1 height diameter ratio and tested at a rate of strain of 2%/minute, unless annotated otherwise. See individual test reports for further details.

Legend	UU - single stage test (may be in sets of specimens)	σ_3	cell pressure	Mode of failure	P	plastic
	UUM - multistage test on a single specimen	$\sigma_1 - \sigma_3$	deviator stress		B	brittle
	suffix R - remoulded or recompacted	C_u	undrained shear strength		C	compound

QA Ref SLR 2 Rev 67 Aug 11		Printed:18/11/2011 17:28	Table UUSUM 1
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**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure
(BS1377 : Part 8 : 1990) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No		BH1	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		5.00-5.45	
			No	13	Type	U
			ID			
			Spec Ref			

Specimen Details

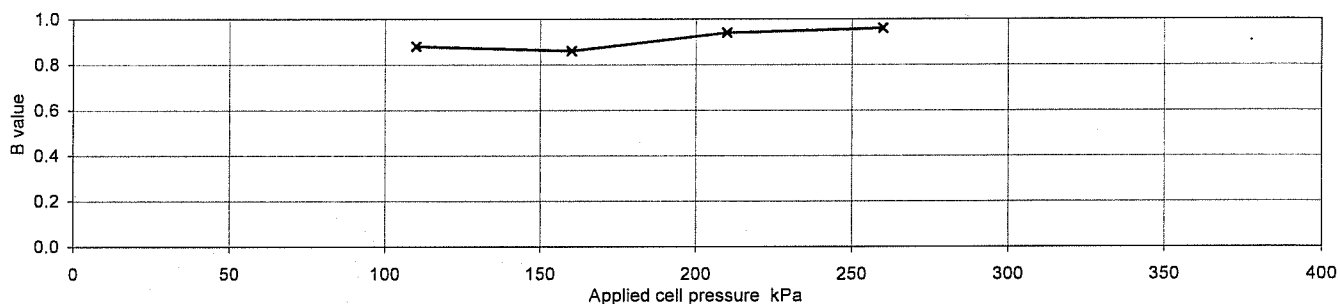
Initial		
Length	mm	203.85
Diameter	mm	101.92
Bulk Density	Mg/m ³	2.26
Water Content	%	14
Dry density	Mg/m ³	1.97
After test		
Bulk Density	Mg/m ³	2.28
Water Content	%	13
Dry density	Mg/m ³	2.01

Soil Description Firm to stiff greyish brown slightly sandy slightly gravelly CLAY.

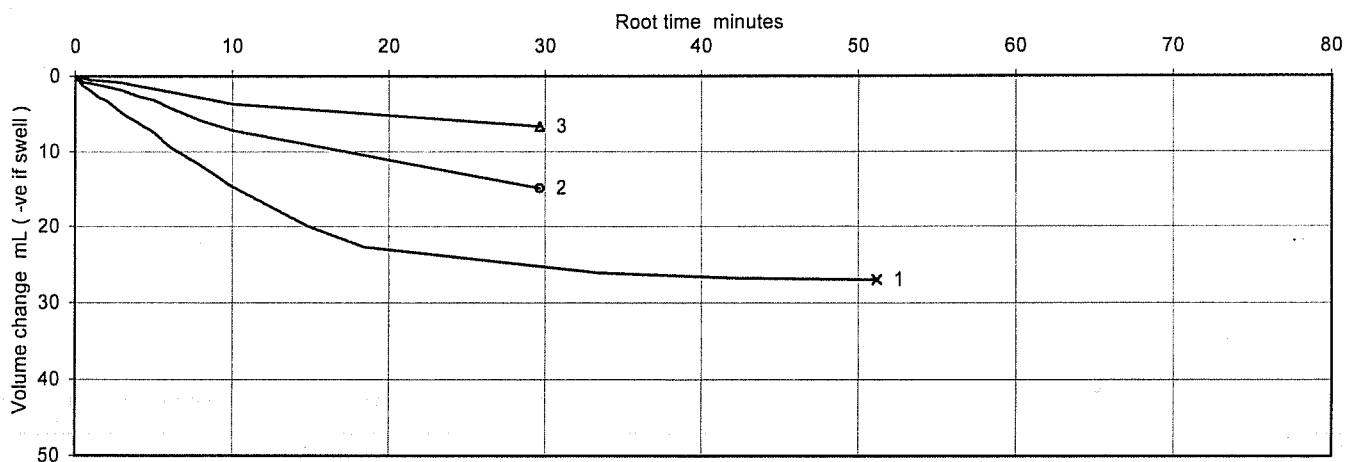
Specimen Type /Preparation UNDISTURBED

Saturation Details

		Method of Saturation	
		Increments of cell and back pressure	
Cell pressure increments	kPa	50	
Differential Pressure	kPa	10	
Final Cell Pressure	kPa	260	
Final pore water pressure	kPa	200	
Final B Value		0.96	



Consolidation Details		Drainage Conditions			
		Stage No.	1	2	3
		Cell Pressure applied	380	420	450
		Back Pressure applied	300	300	300
		Effective Pressure	80	120	150
		Pore pressure at start of consolidation	368	376	376
		Pore pressure at end of consolidation	303	300	302
		Pore pressure dissipation at end of consolidation	96	100	97
Consolidation parameters (see note to BS1377 : pt 8, clause 6.3.4)	Coefficient of Consolidation	C _{vi}	0.62	0.55	0.70
	Coefficient of Compressibility	M _{vi}	0.25	0.12	0.06
	Coefficient of Permeability (calculated)	k _{vi}	4.7E-11	2.0E-11	1.2E-11



Ref

SLR8.1
Rev 85
May 09



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Figure

CUM 1

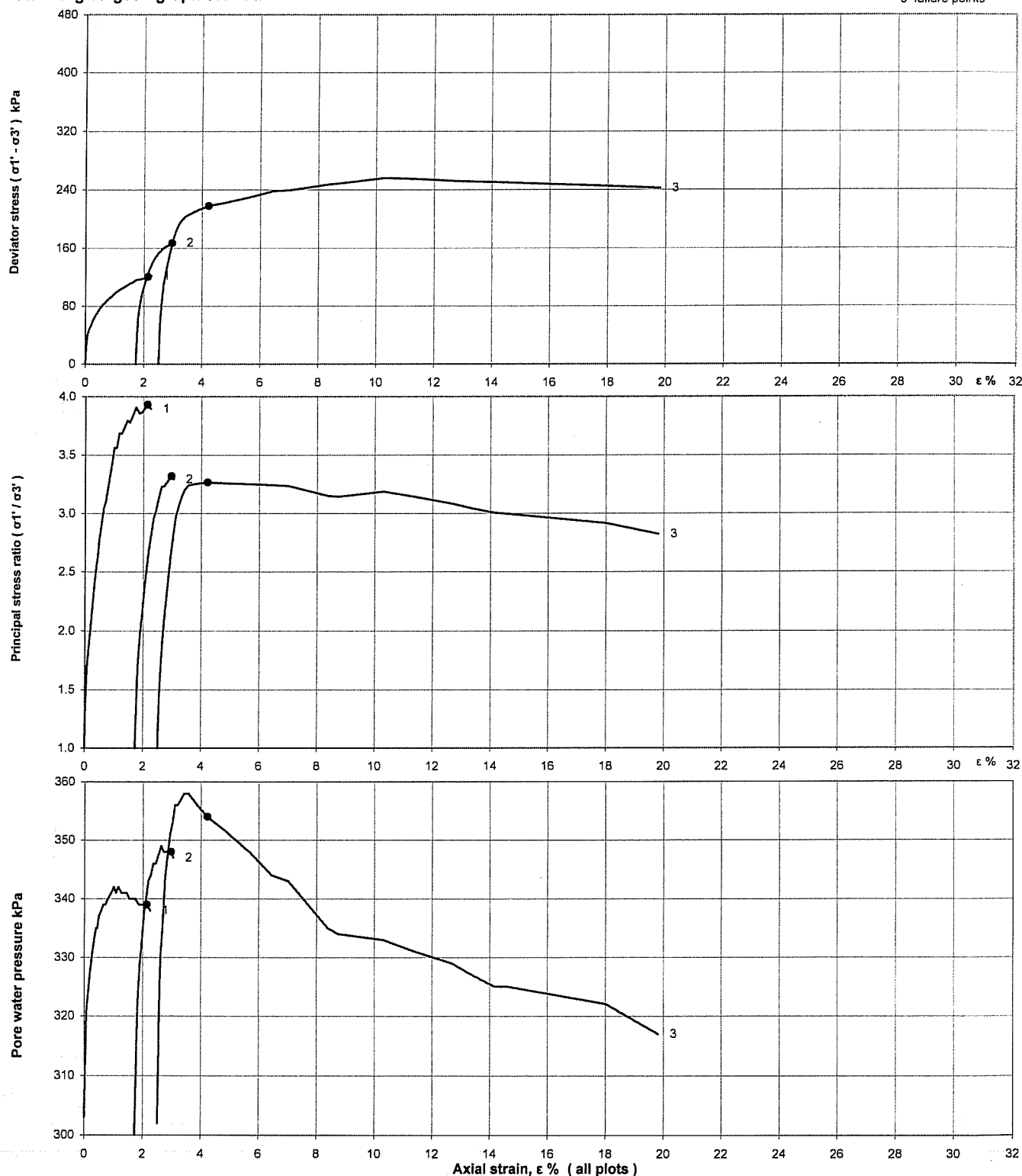
sheet 1 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure
(BS1377 : Part 8 : 1990) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No		BH1		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		5.00-5.45		
			No	13	Type	U	
			ID				
			Spec Ref				

Shearing stages - graphical data

o failure points



Ref

SLR8.1
Rev 85
May 09



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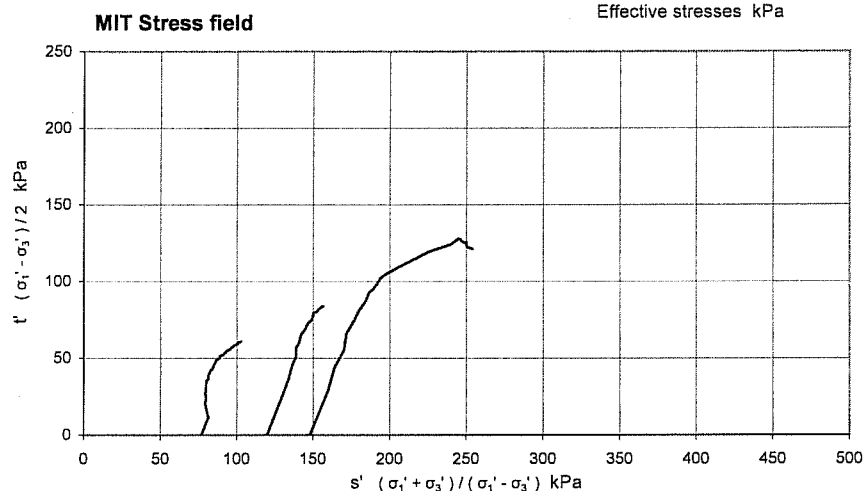
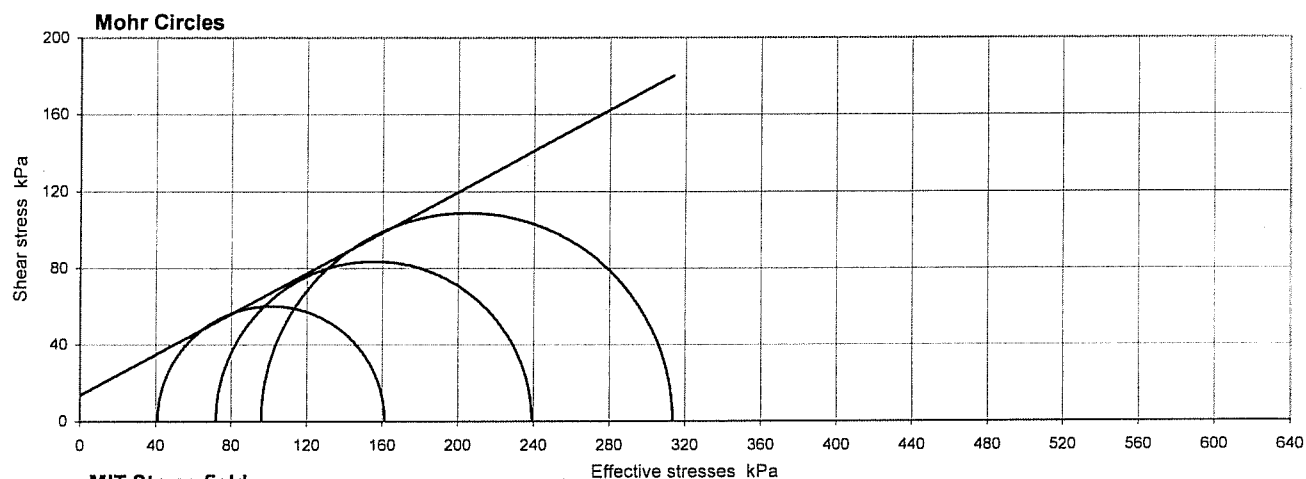
Figure

CUM 1

sheet 2 of 3

Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure (BS1377 : Part 8 : 1990) - Multistage test on a single specimen

Project No	A1077-11	Sample Details:	Hole No	BH1
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	5.00-5.45
			No	13
			Type	U
			ID	
			Spec Ref	



Compression stages

Stage	1	2	3	
Cell pressure	380	420	450	kPa
Initial pwp	303	300	302	kPa
Initial σ_3'	77	120	148	kPa
Rate of strain	0.48	0.48	0.48	%/hr

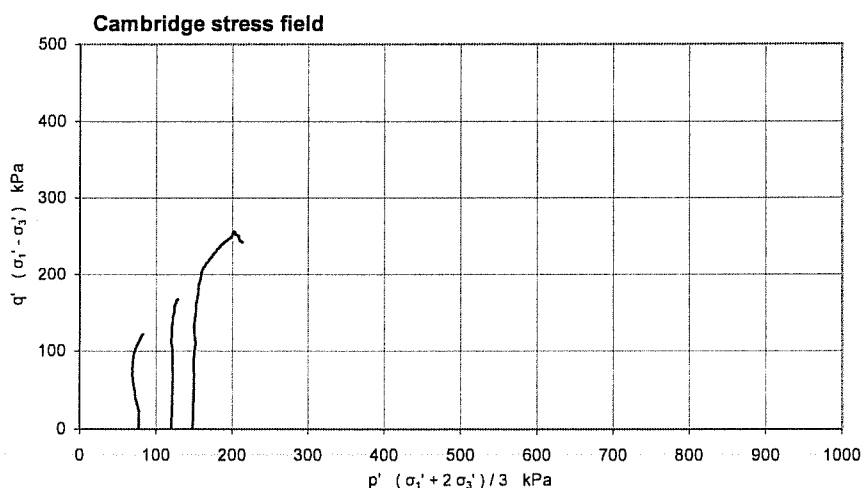
Failure conditions

Criterion	Maximum effective principal stress ratio			
Axial strain	2.13	2.97	4.22	%
$(\sigma_1' / \sigma_3')_f$	3.932	3.320	3.266	
$(\sigma_1' - \sigma_3')_f$	120.2	167.0	217.6	kPa
u_f	339	348	354	kPa
$\sigma_3' f$	41	72	96	kPa
$\sigma_1' f$	161	239	314	kPa
A_f	0.30	0.29	0.24	
Time to failure	4.4	6.2	8.8	hrs

Shear Strength Parameters

at peak stress ratio

		Linear regression	
c'	kPa	13.6	
ϕ'	degrees	28.0	
		Manual re-assessment	
c'	kPa	-	
ϕ'	degrees	-	



Notes : Deviator stresses corrected for area change, vertical side drains and 0.45 mm thick rubber membrane(s)

Mode of failure



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Figure

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**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure
(BS1377 : Part 8 : 1990) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH1
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	8.00-8.45
			No	21
			Type	U
			ID	
			Spec Ref	

Specimen Details

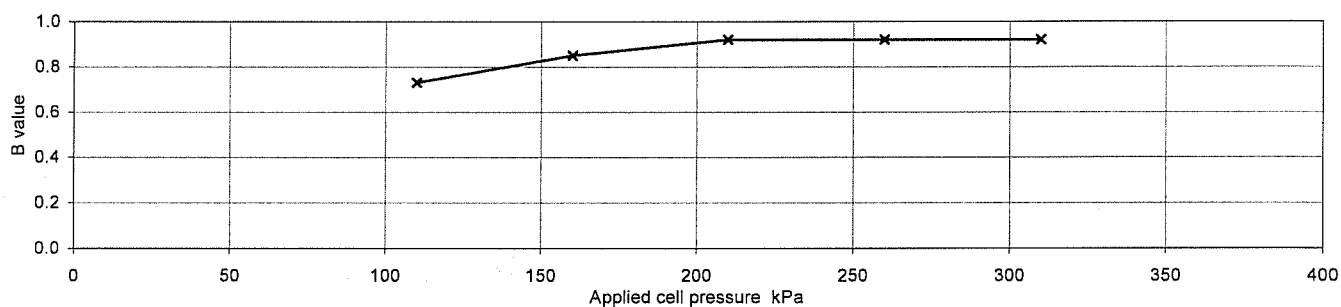
Initial		
Length	mm	204.04
Diameter	mm	103.12
Bulk Density	Mg/m ³	2.20
Water Content	%	15
Dry density	Mg/m ³	1.91
After test		
Bulk Density	Mg/m ³	2.25
Water Content	%	13
Dry density	Mg/m ³	1.99

Soil Description Firm to stiff greyish brown slightly sandy slightly gravelly CLAY.

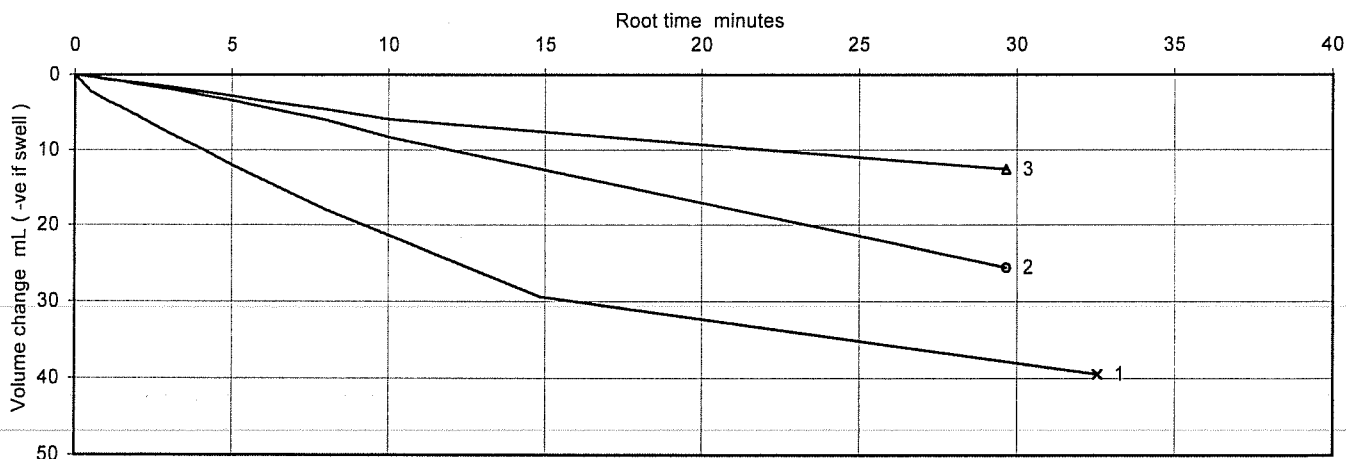
Specimen Type /Preparation UNDISTURBED

Saturation Details

		Method of Saturation
		Increments of cell and back pressure
Cell pressure increments	kPa	50
Differential Pressure	kPa	10
Final Cell Pressure	kPa	310
Final pore water pressure	kPa	250
Final B Value		0.92



Consolidation Details		Drainage Conditions			
		Stage No.	1	2	3
		Cell Pressure applied	420	470	520
		Back Pressure applied	300	300	300
		Effective Pressure	120	170	220
		Pore pressure at start of consolidation	406	406	419
		Pore pressure at end of consolidation	301	303	303
		Pore pressure dissipation at end of consolidation	100	98	97
Consolidation parameters (see note to BS1377 : pt 8, clause 6.3.4)	Coefficient of Consolidation	C _{vi}	0.66	0.25	0.49
	Coefficient of Compressibility	M _{vi}	0.22	0.15	0.07
	Coefficient of Permeability (calculated)	k _{vi}	4.4E-11	1.1E-11	9.9E-12



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Figure

CUM 2

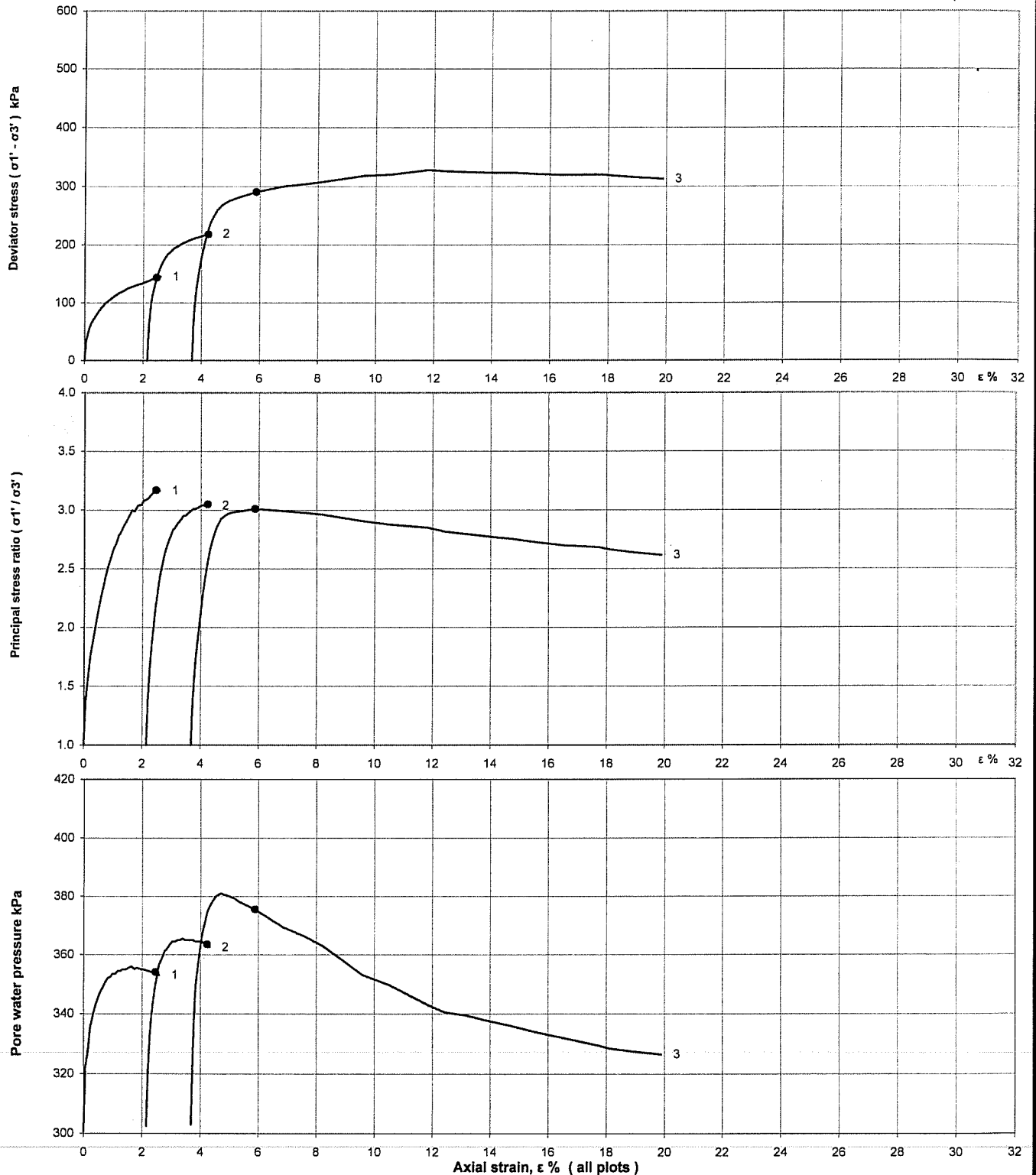
sheet 1 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure
(BS1377 : Part 8 : 1990) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH1
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	8.00-8.45
			No	21
			Type	U
			ID	
			Spec Ref	

Shearing stages - graphical data

o failure points



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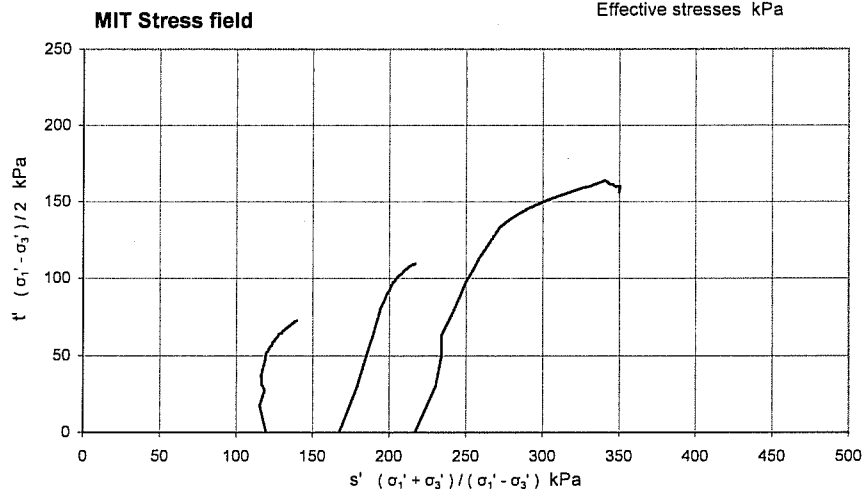
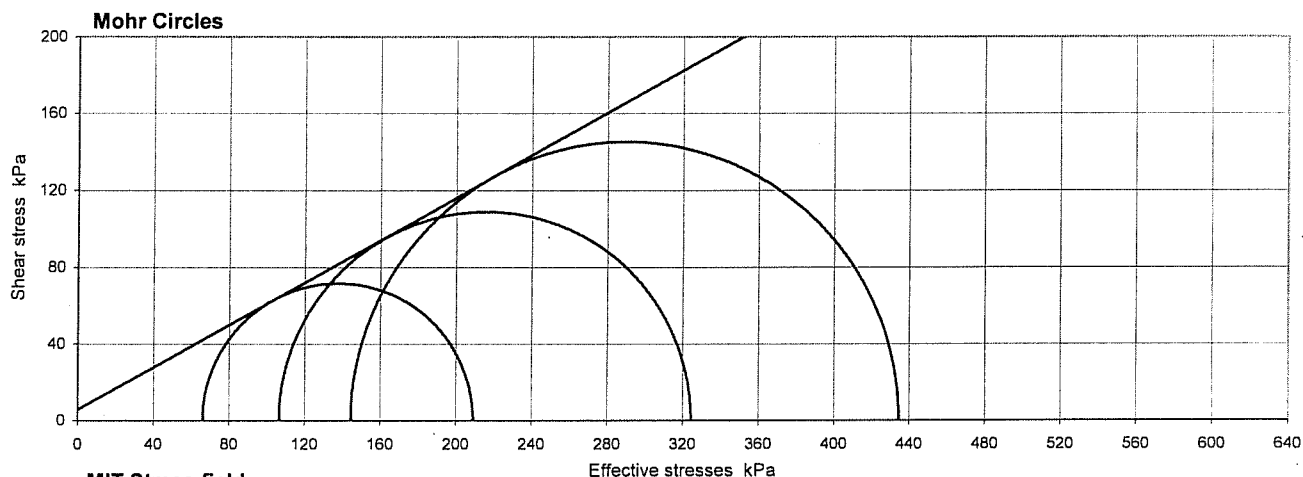
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Figure

CUM 2
sheet 2 of 3

Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure (BS1377 : Part 8 : 1990) - Multistage test on a single specimen

Project No	A1077-11	Sample Details:	Hole No	BH1
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	8.00-8.45
			No	21
			Type	U
			ID	
			Spec Ref	

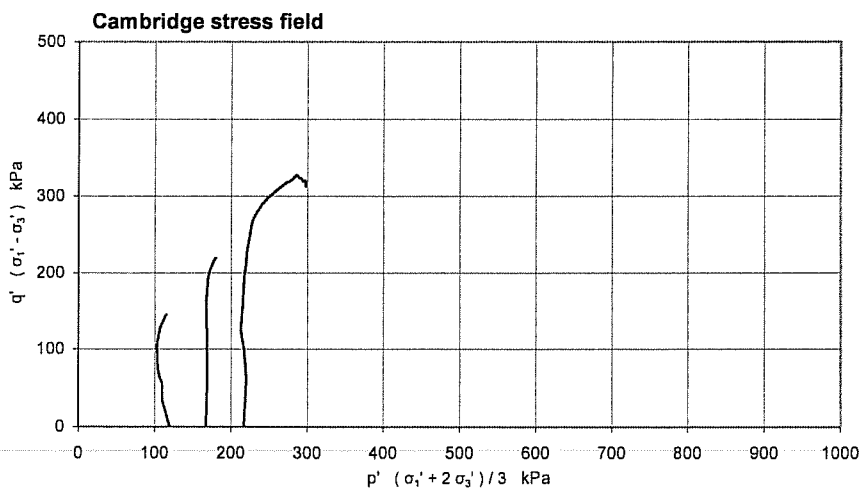


Compression stages

Stage	1	2	3	
Cell pressure	420	470	520	kPa
Initial pwp	301	303	303	kPa
Initial σ_3'	120	168	217	kPa
Rate of strain	0.49	0.49	0.49	%/hr

Failure conditions

Criterion	Maximum effective principal stress ratio			
Axial strain	2.46	4.23	5.88	%
$(\sigma_1' / \sigma_3')_f$	3.168	3.047	3.008	
$(\sigma_1' - \sigma_3')_f$	143.1	218.0	290.2	kPa
u_f	354	364	376	kPa
$\sigma_3' f$	66	107	145	kPa
$\sigma_1' f$	209	325	435	kPa
A_f	0.37	0.28	0.25	
Time to failure	5.0	8.6	12.0	hrs



Shear Strength Parameters

at peak stress ratio

		Linear regression	
c'	kPa	5.6	
ϕ'	degrees	28.9	
		Manual re-assessment	
c'	kPa	-	
ϕ'	degrees	-	

Notes : Deviator stresses corrected for area change, vertical side drains and 0.45 mm thick rubber membrane(s)

Mode of failure



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Figure

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**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure
(BS1377 : Part 8 : 1990) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No		BH1		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		15.50-15.95		
			No	41	Type	U	
			ID				
			Spec Ref				

Specimen Details

Initial		
Length	mm	205.22
Diameter	mm	103.64
Bulk Density	Mg/m ³	2.11
Water Content	%	19
Dry density	Mg/m ³	1.78
After test		
Bulk Density	Mg/m ³	2.14
Water Content	%	17
Dry density	Mg/m ³	1.84

Soil Description

Stiff brown thinly laminated CLAY with thin laminations of sand becoming slightly sandy slightly gravelly CLAY at base.

Specimen Type /Preparation

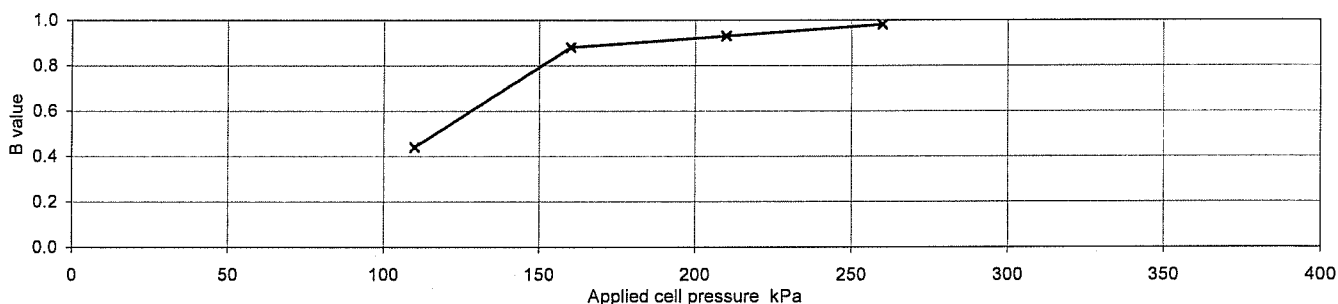
UNDISTURBED

Saturation Details

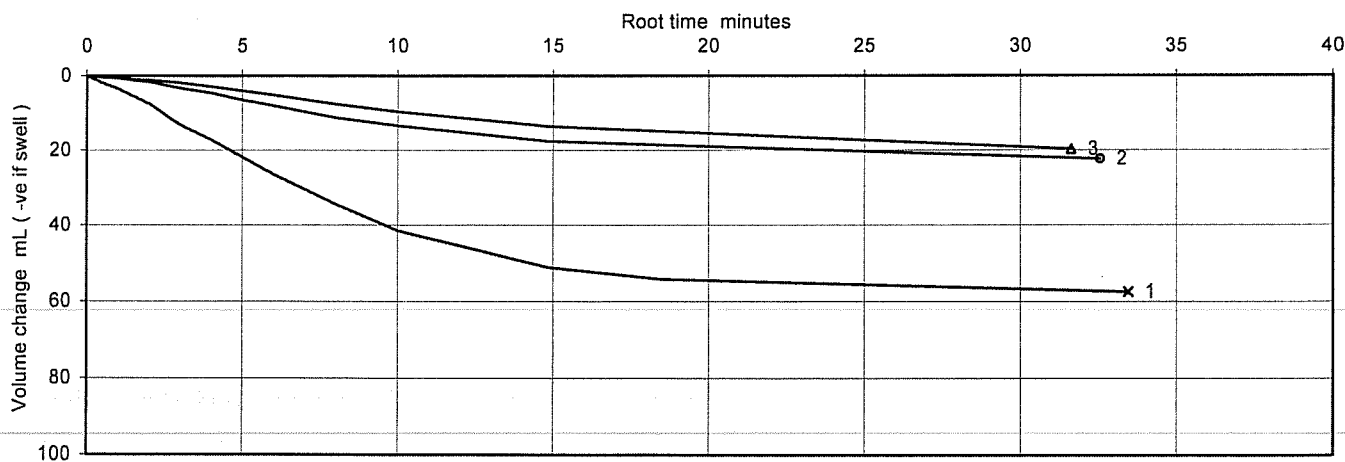
Method of Saturation

Increments of cell and back pressure

Cell pressure increments	kPa	50
Differential Pressure	kPa	10
Final Cell Pressure	kPa	260
Final pore water pressure	kPa	200
Final B Value		0.98



Consolidation Details	Drainage Conditions		From radial boundary and one end			
	Stage No.		1	2	3	
	Cell Pressure applied		505	590	665	kPa
	Back Pressure applied		300	300	300	kPa
	Effective Pressure		205	290	365	kPa
	Pore pressure at start of consolidation		491	487	496	kPa
	Pore pressure at end of consolidation		300	303	306	kPa
	Pore pressure dissipation at end of consolidation		100	99	97	%
Consolidation parameters (see note to BS1377 : pt 8, clause 6.3.4)	Coefficient of Consolidation	C _{vi}	1.37	0.97	0.63	m ² /year
	Coefficient of Compressibility	M _{vi}	0.17	0.07	0.06	m ² /MN
	Coefficient of Permeability (calculated)	k _{vi}	7.2E-11	2.1E-11	1.2E-11	m/s



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Figure

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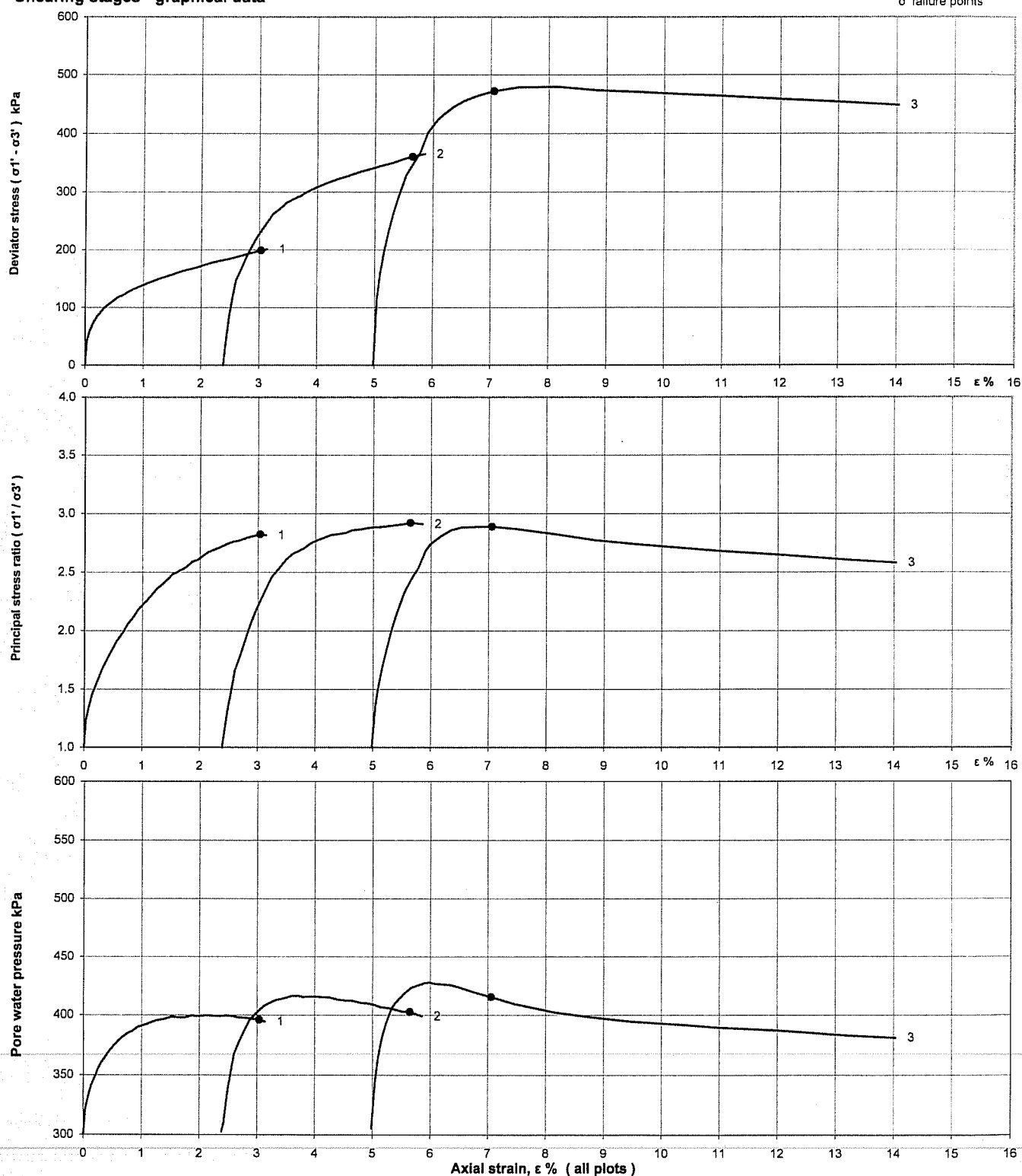
sheet 1 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure
(BS1377 : Part 8 : 1990) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH1		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	15.50-15.95		
			No	41	Type	U
			ID			
			Spec Ref			

Shearing stages - graphical data

o failure points



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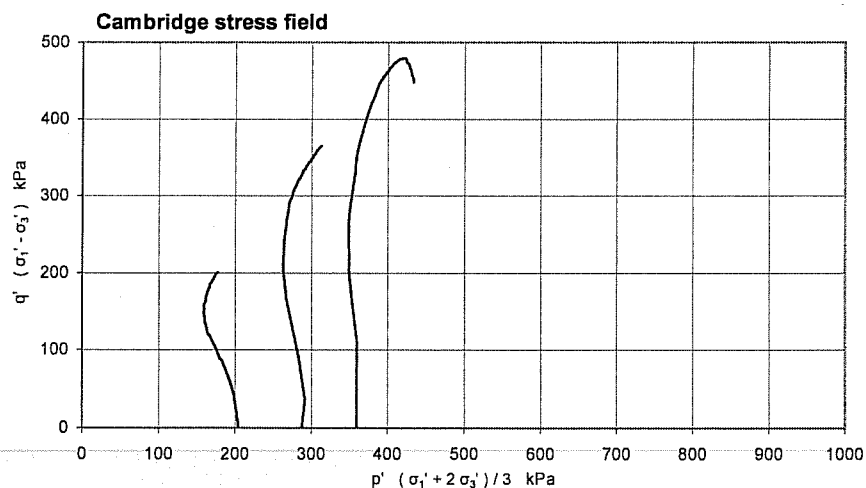
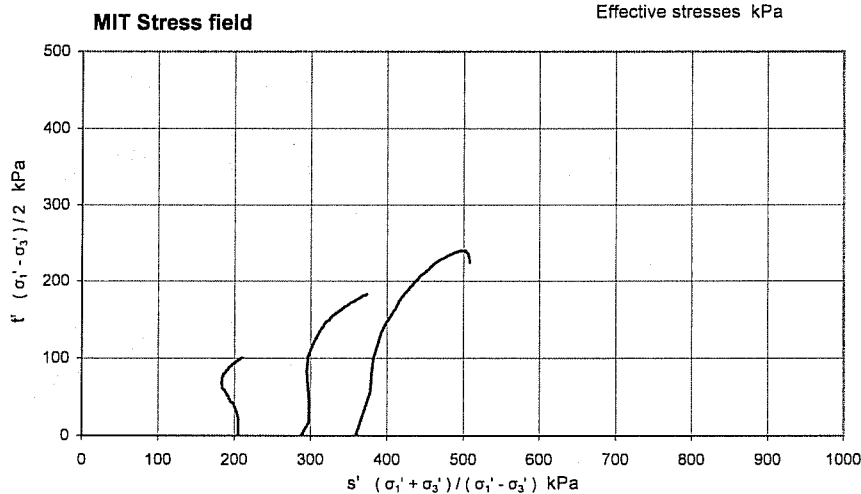
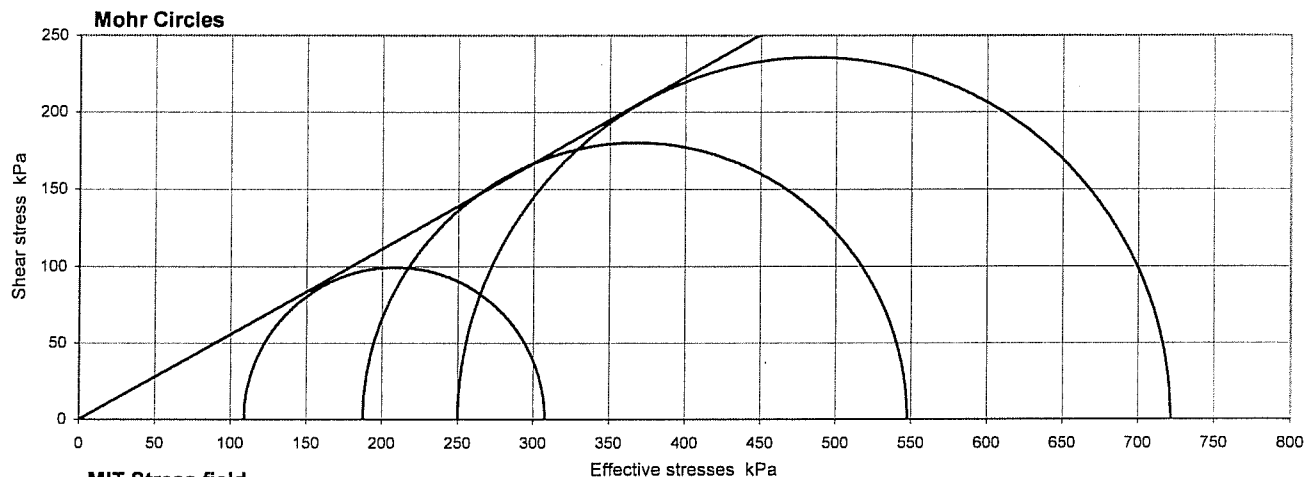
Figure

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sheet 2 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure
(BS1377 : Part 8 : 1990) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH1
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	15.50-15.95
			No	41
			Type	U
			ID	
			Spec Ref	



Compression stages

Stage	1	2	3	
Cell pressure	505	590	665	kPa
Initial pwp	300	303	306	kPa
Initial σ_3'	205	288	360	kPa
Rate of strain	1.00	1.00	1.00	%/hr

Failure conditions

Criterion	Maximum effective principal stress ratio			
Axial strain	3.04	5.65	7.06	%
$(\sigma_1' / \sigma_3')_f$	2.822	2.920	2.887	
$(\sigma_1' - \sigma_3')_f$	198.6	360.1	471.6	kPa
u_f	396	403	415	kPa
$\sigma_3' f$	109	188	250	kPa
$\sigma_1' f$	308	548	722	kPa
A_f	0.48	0.28	0.23	
Time to failure	3.0	5.6	7.1	hrs

Shear Strength Parameters

at peak stress ratio

		Linear regression	
c'	kPa	(-3.1)	
ϕ'	degrees	(29.5)	
		Manual re-assessment	
c'	kPa	0	
ϕ'	degrees	29.1	

Notes : Deviator stresses corrected for area change, vertical side drains and 0.45 mm thick rubber membrane(s)

Mode of failure



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**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure
(BS1377 : Part 8 : 1990) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH1
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	19.50-19.95
			No	52
			Type	U
			ID	
			Spec Ref	

Specimen Details

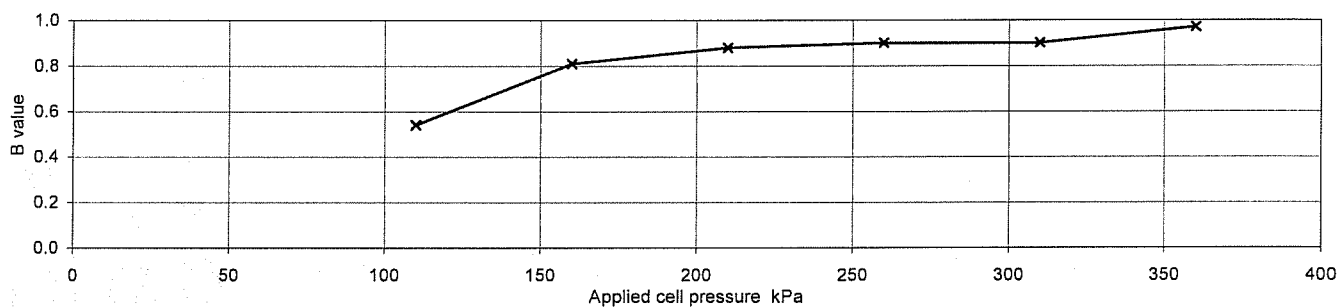
Initial		
Length	mm	204.42
Diameter	mm	101.75
Bulk Density	Mg/m ³	2.24
Water Content	%	12
Dry density	Mg/m ³	1.99
After test		
Bulk Density	Mg/m ³	2.27
Water Content	%	11
Dry density	Mg/m ³	2.04

Soil Description Stiff greyish brown slightly sandy slightly gravelly CLAY.

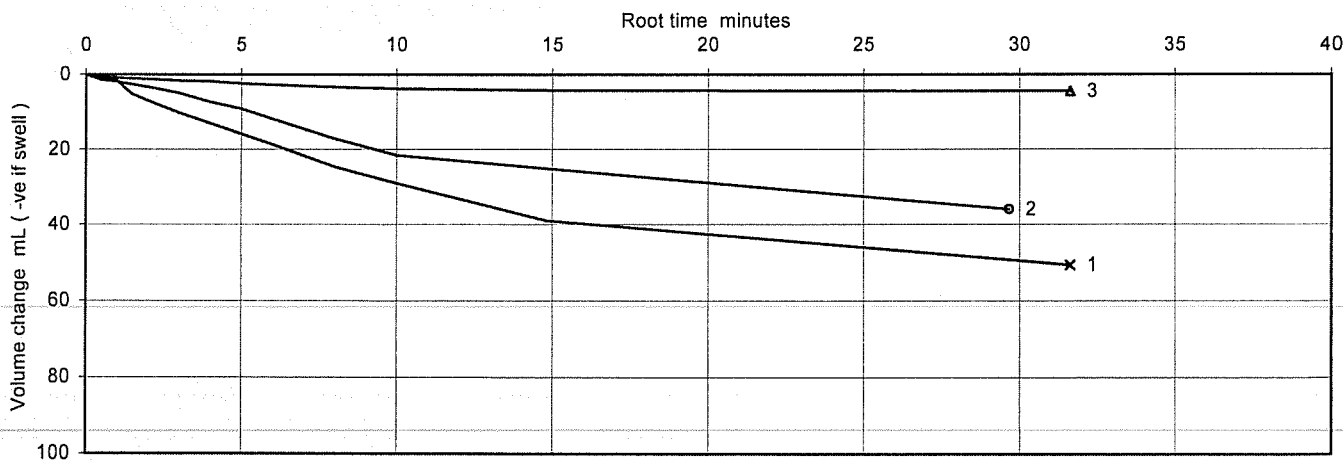
Specimen Type /Preparation UNDISTURBED

Saturation Details

		Method of Saturation
		Increments of cell and back pressure
Cell pressure increments	kPa	50
Differential Pressure	kPa	10
Final Cell Pressure	kPa	360
Final pore water pressure	kPa	300
Final B Value		0.97



Consolidation Details		Drainage Conditions			
		Stage No.	1	2	3
		Cell Pressure applied	580	700	790
		Back Pressure applied	300	300	300
		Effective Pressure	280	400	490
		Pore pressure at start of consolidation	563	521	536
		Pore pressure at end of consolidation	306	301	310
		Pore pressure dissipation at end of consolidation	98	100	96
Consolidation parameters (see note to BS1377 : pt 8, clause 6.3.4)	Coefficient of Consolidation	C _{vi}	0.74	0.83	1.99
	Coefficient of Compressibility	M _{vi}	0.12	0.10	0.01
	Coefficient of Permeability (calculated)	k _{vi}	2.6E-11	2.5E-11	7.5E-12



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Figure

CUM 4

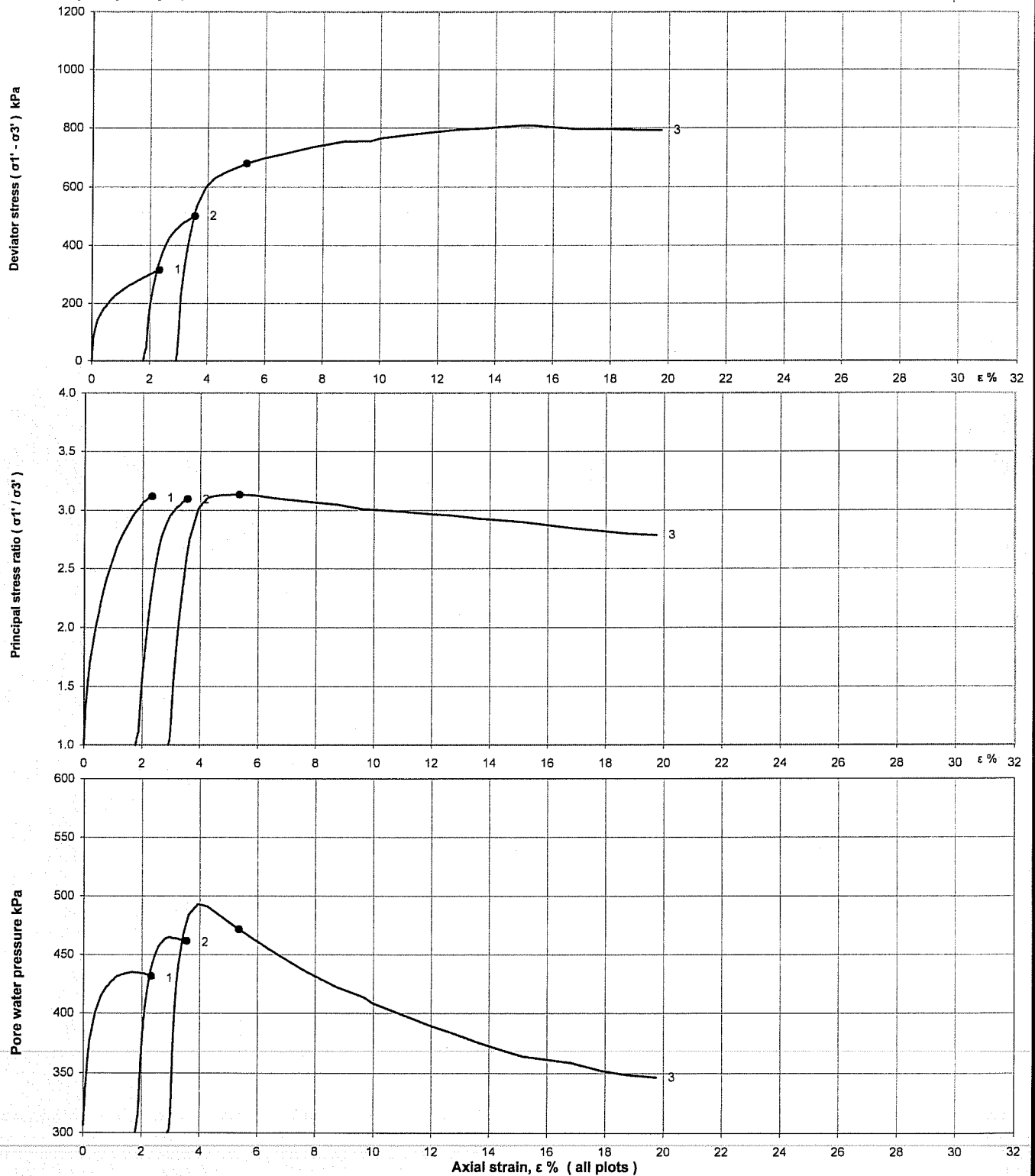
sheet 1 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure
(BS1377 : Part 8 : 1990) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH1
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	19.50-19.95
			No	52
			Type	U
			ID	
			Spec Ref	

Shearing stages - graphical data

o failure points



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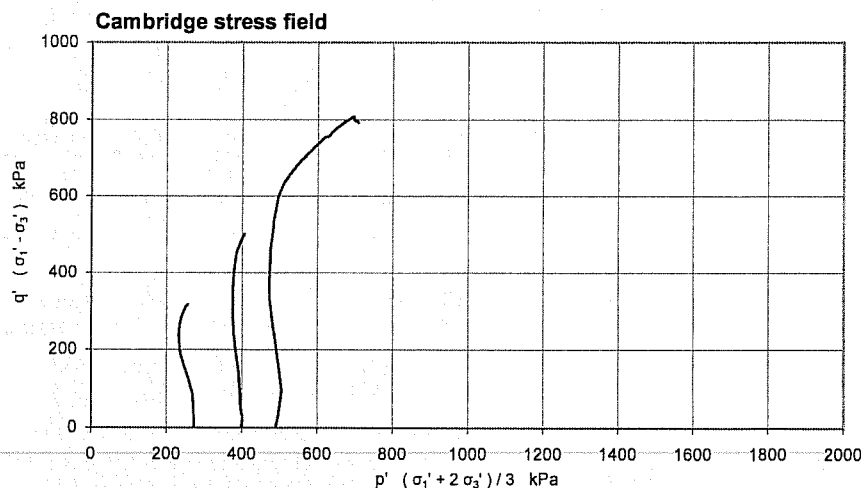
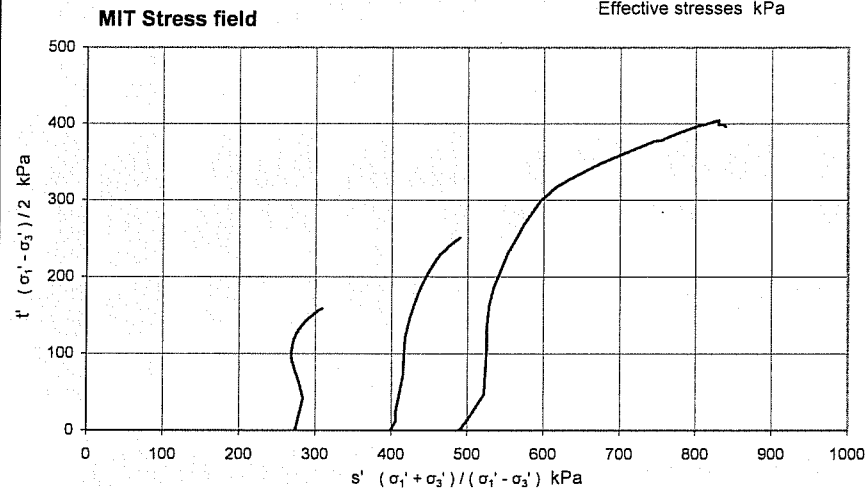
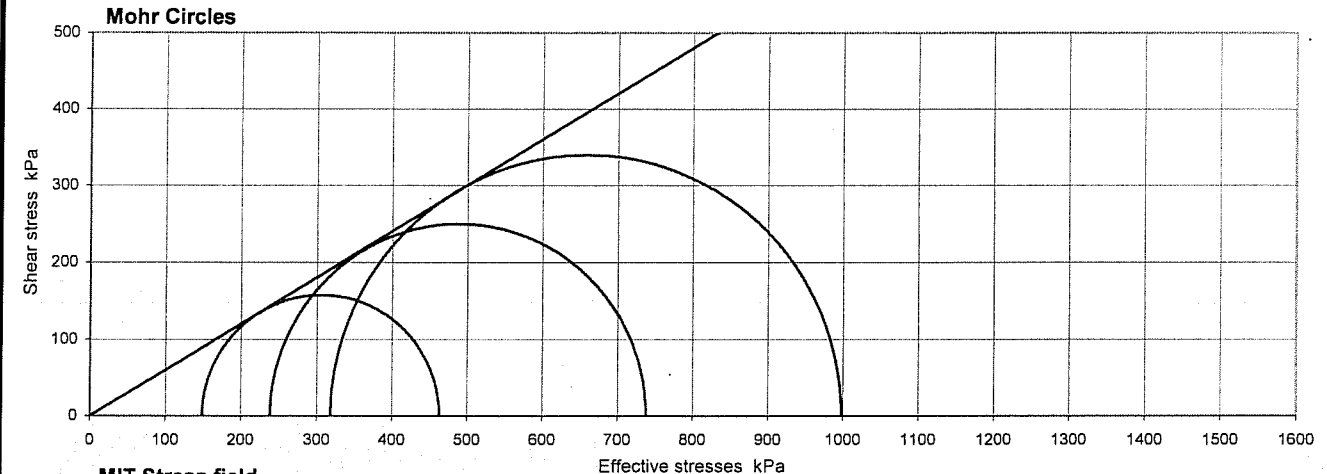
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Figure

CUM 4
sheet 2 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure
(BS1377 : Part 8 : 1990) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH1
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	19.50-19.95
			No	52
			Type	U
			ID	
			Spec Ref	



Compression stages

Stage	1	2	3	
Cell pressure	580	700	790	kPa
Initial pwp	306	301	300	kPa
Initial σ_3'	274	400	490	kPa
Rate of strain	0.57	0.57	0.57	%/hr

Failure conditions

Criterion	Maximum effective principal stress ratio			
Axial strain	2.33	3.56	5.35	%
$(\sigma_1' / \sigma_3')_f$	3.118	3.094	3.133	
$(\sigma_1' - \sigma_3')_f$	314.5	499.4	679.4	kPa
u_f	432	462	472	kPa
$\sigma_3' f$	149	239	319	kPa
$\sigma_1' f$	463	738	998	kPa
A_f	0.40	0.32	0.25	
Time to failure	4.1	6.2	9.4	hrs

Shear Strength Parameters

at peak stress ratio

		Linear regression
c'	kPa	(-1.9)
ϕ'	degrees	(31.2)
		Manual re-assessment
c'	kPa	0
ϕ'	degrees	31.0

Notes : Deviator stresses corrected for area change, vertical side drains and 0.9 mm thick rubber membrane(s)

Mode of failure



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Figure

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sheet 3 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure
(BS1377 : Part 8 : 1990) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No		BH2		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		1.20-1.65		
			No	4	Type	U	
			ID				
			Spec Ref				

Specimen Details

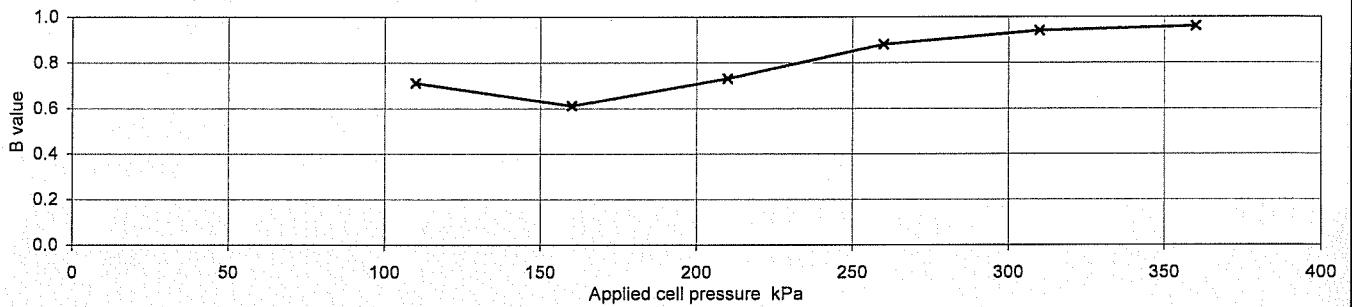
Initial		
Length	mm	204.29
Diameter	mm	101.26
Bulk Density	Mg/m ³	2.07
Water Content	%	21
Dry density	Mg/m ³	1.72
After test		
Bulk Density	Mg/m ³	2.07
Water Content	%	22
Dry density	Mg/m ³	1.69

Soil Description Firm to stiff greyish brown and brownish grey slightly sandy slightly gravelly CLAY.

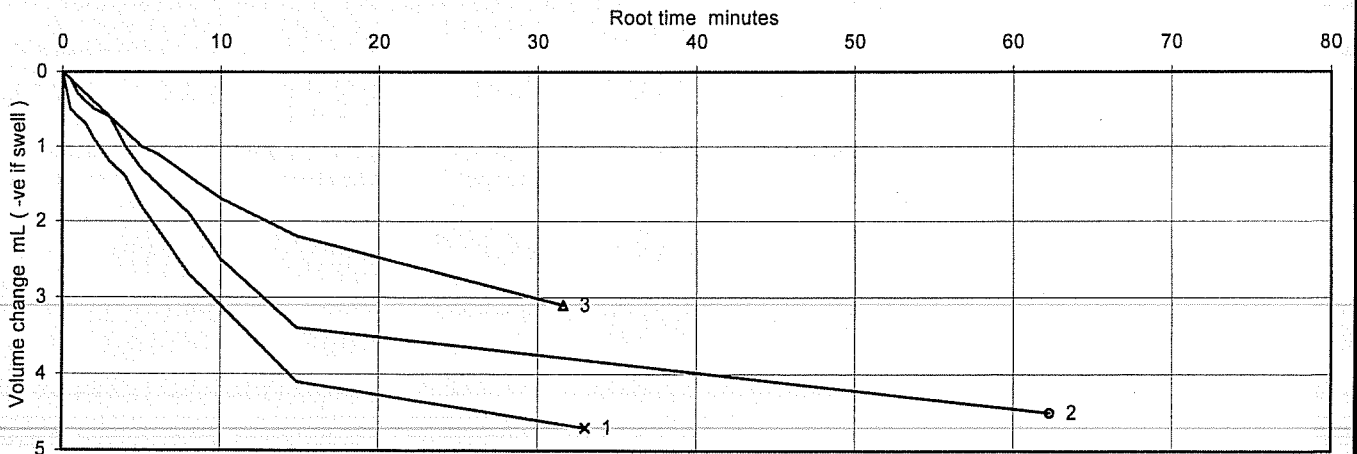
Specimen Type /Preparation UNDISTURBED

Saturation Details

		Method of Saturation	
		Increments of cell and back pressure	
Cell pressure increments	kPa	50	
Differential Pressure	kPa	10	
Final Cell Pressure	kPa	360	
Final pore water pressure	kPa	300	
Final B Value		0.96	



Consolidation Details		Drainage Conditions			
		Stage No.	1	2	3
		Cell Pressure applied	375	385	395
		Back Pressure applied	350	350	350
		Effective Pressure	25	35	45
		Pore pressure at start of consolidation	362	361	362
		Pore pressure at end of consolidation	350	350	351
		Pore pressure dissipation at end of consolidation	100	100	96
Consolidation parameters (see note to BS1377 : pt 8, clause 6.3.4)	Coefficient of Consolidation	C _{vi}	1.01	0.62	0.57
	Coefficient of Compressibility	M _{vi}	0.24	0.24	0.17
	Coefficient of Permeability (calculated)	k _{vi}	7.6E-11	4.7E-11	3.0E-11



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Figure

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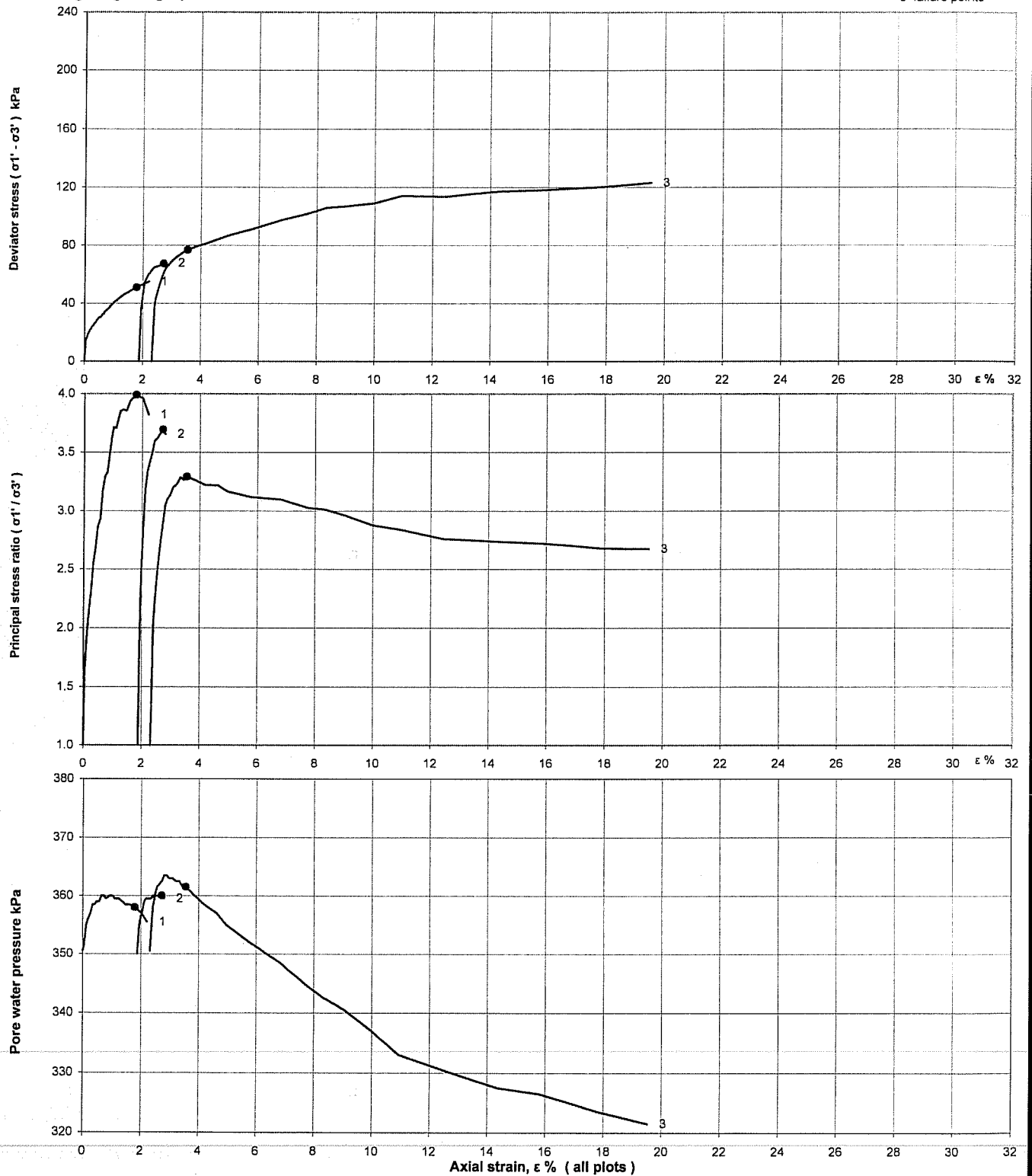
sheet 1 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure
(BS1377 : Part 8 : 1990) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No		BH2	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		1.20-1.65	
			No	4	Type	U
			ID			
			Spec Ref			

Shearing stages - graphical data

o failure points



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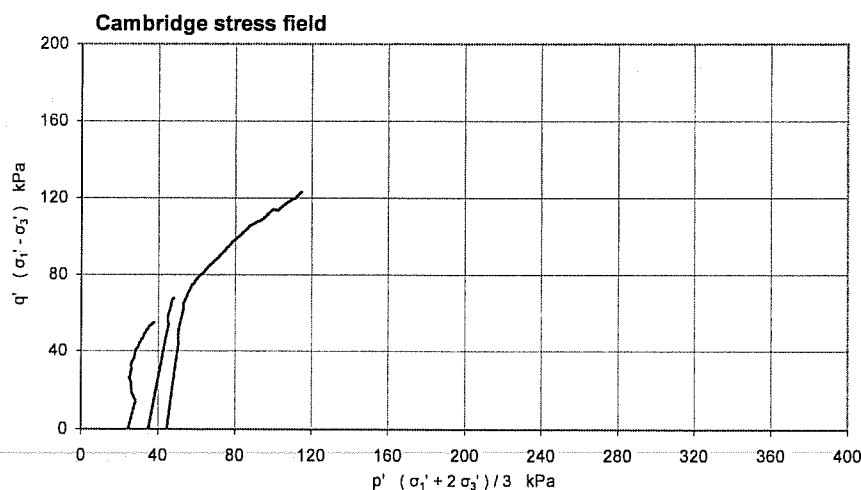
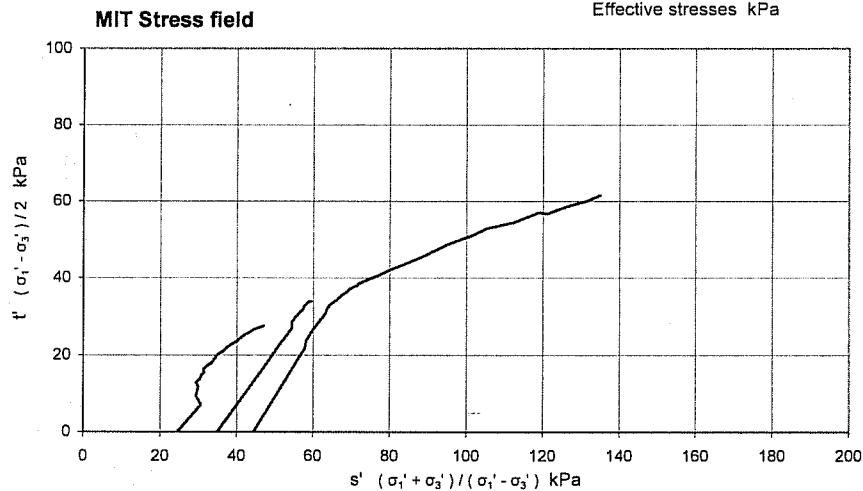
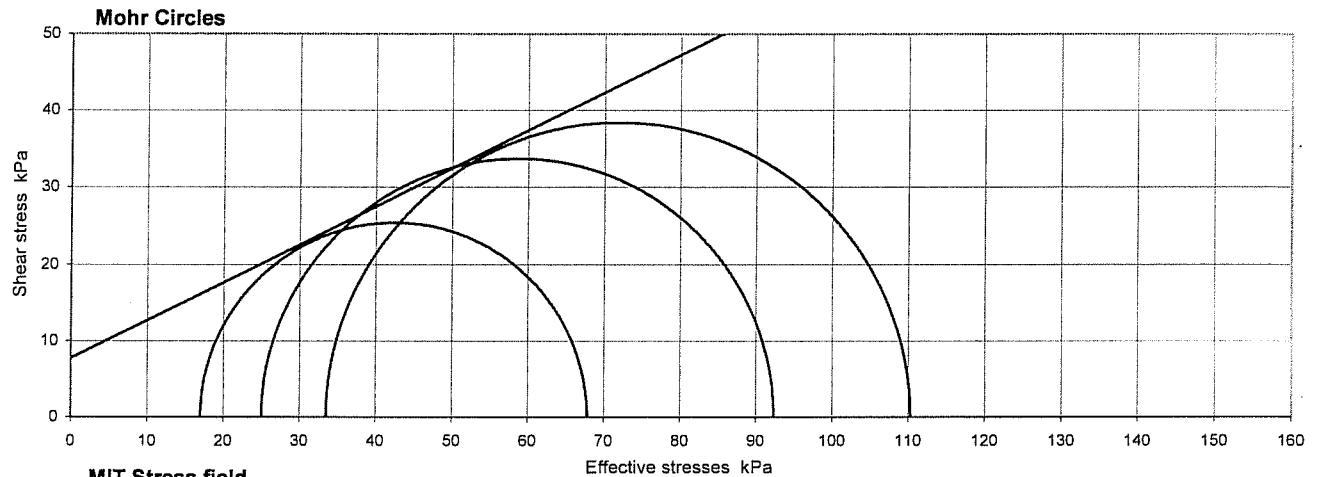
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Figure

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sheet 2 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure
(BS1377 : Part 8 : 1990) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH2
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	1.20-1.65
			No	4
			Type	U
			ID	
			Spec Ref	



Compression stages

Stage	1	2	3	
Cell pressure	375	385	395	kPa
Initial pwp	351	350	351	kPa
Initial σ_3'	25	35	45	kPa
Rate of strain	0.79	0.79	0.79	%/hr

Failure conditions

Criterion	Maximum effective principal stress ratio			
Axial strain	1.79	2.73	3.56	%
$(\sigma_1' / \sigma_3')_f$	3.990	3.695	3.292	
$(\sigma_1' - \sigma_3')_f$	50.8	67.4	76.8	kPa
u_f	358	360	362	kPa
$\sigma_3' f$	17	25	34	kPa
$\sigma_1' f$	68	92	110	kPa
A_f	0.15	0.15	0.14	
Time to failure	2.3	3.5	4.5	hrs

Shear Strength Parameters

at peak stress ratio

		Linear regression	
c'	kPa	7.8	
ϕ'	degrees	26.3	
		Manual re-assessment	
c'	kPa	-	
ϕ'	degrees	-	

Notes : Deviator stresses corrected for area change, vertical side drains and 0.45 mm thick rubber membrane(s)

Mode of failure



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Figure

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sheet 3 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure
(BS1377 : Part 8 : 1990) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH2
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	10.00-10.45
			No	22
			Type	U
			ID	
			Spec Ref	

Specimen Details

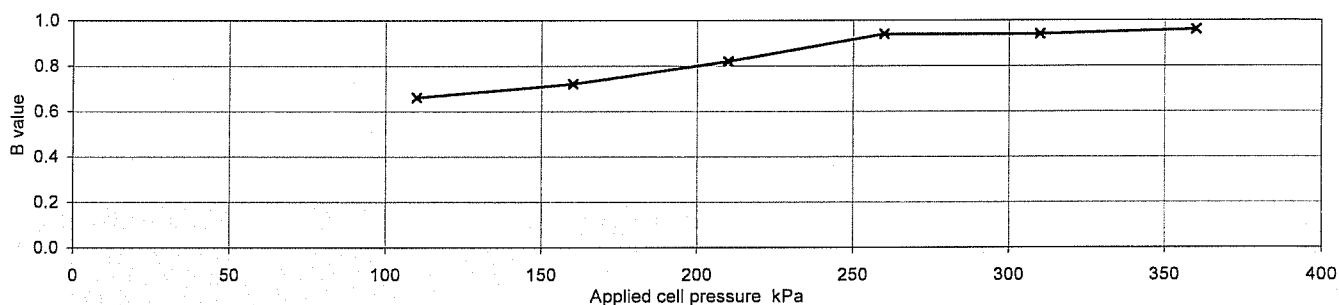
Initial		
Length	mm	203.69
Diameter	mm	103.43
Bulk Density	Mg/m ³	2.18
Water Content	%	16
Dry density	Mg/m ³	1.88
After test		
Bulk Density	Mg/m ³	2.24
Water Content	%	14
Dry density	Mg/m ³	1.97

Soil Description Firm becoming soft to firm at base greyish brown slightly sandy slightly gravelly CLAY.

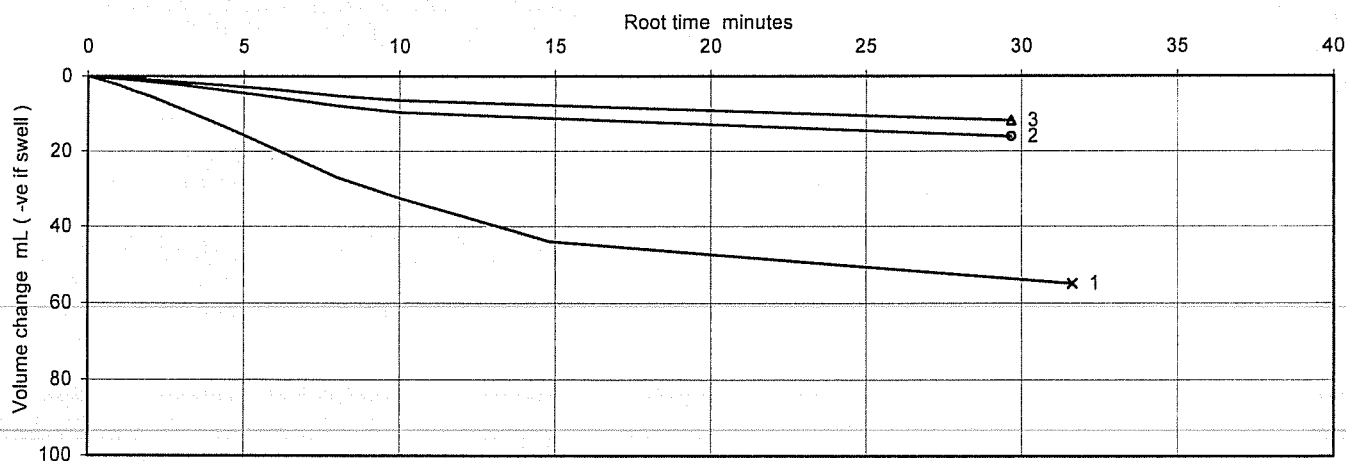
Specimen Type /Preparation UNDISTURBED

Saturation Details

		Method of Saturation
		Increments of cell and back pressure
Cell pressure increments	kPa	50
Differential Pressure	kPa	10
Final Cell Pressure	kPa	360
Final pore water pressure	kPa	300
Final B Value		0.96



Consolidation Details		Drainage Conditions			
		Stage No.	1	2	3
		Cell Pressure applied	450	510	565
		Back Pressure applied	300	300	300
		Effective Pressure	150	210	265
		Pore pressure at start of consolidation	426	434	446
		Pore pressure at end of consolidation	300	302	302
		Pore pressure dissipation at end of consolidation	100	99	99
Consolidation parameters (see note to BS1377 : pt 8, clause 6.3.4)	Coefficient of Consolidation	C _{vi}	0.96	0.94	0.80
	Coefficient of Compressibility	M _{vi}	0.25	0.07	0.05
	Coefficient of Permeability (calculated)	k _{vi}	7.5E-11	2.1E-11	1.2E-11



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Figure

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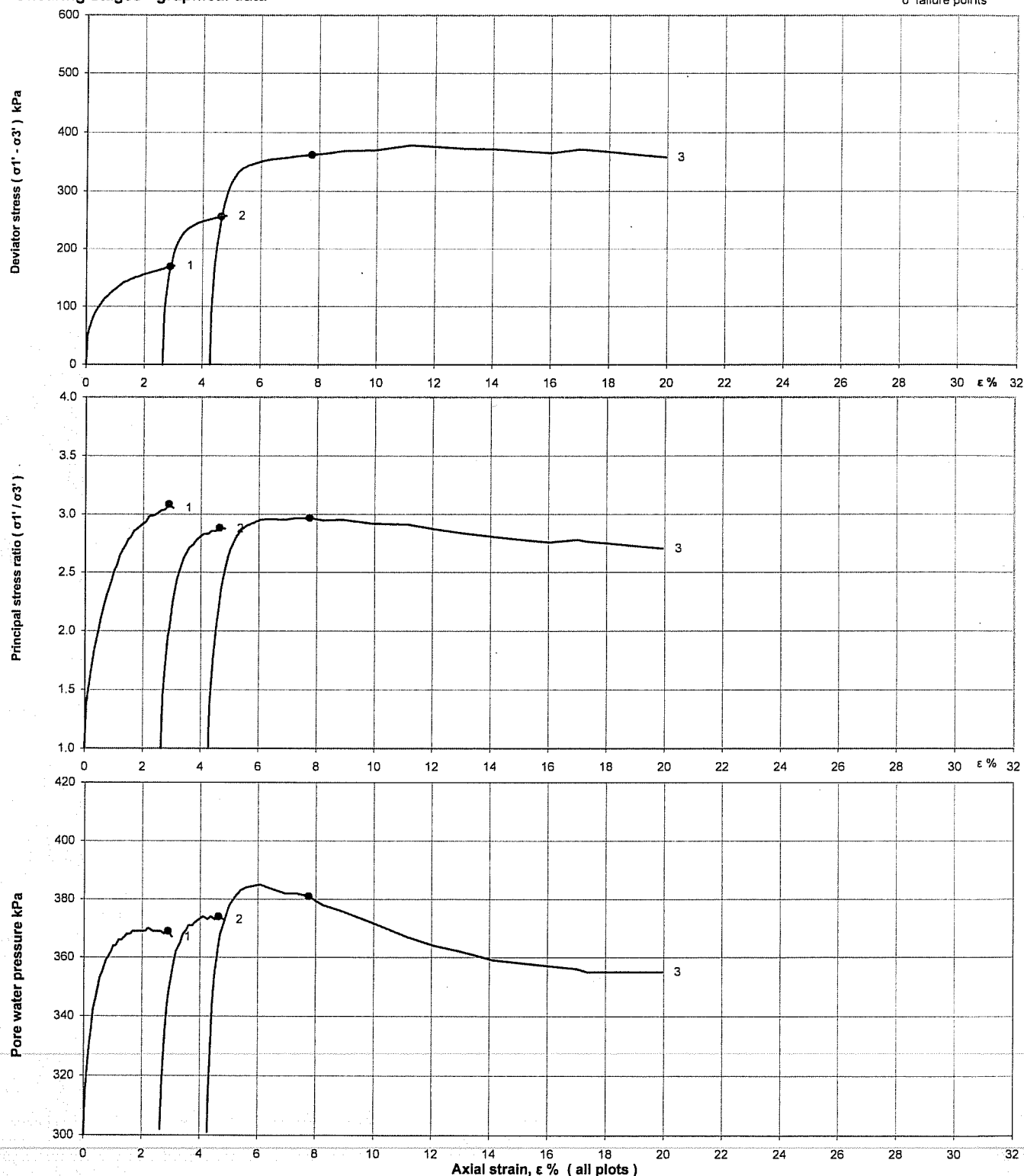
sheet 1 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure
(BS1377 : Part 8 : 1990) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH2
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	10.00-10.45
			No	22
			Type	U
			ID	
			Spec Ref	

Shearing stages - graphical data

o failure points



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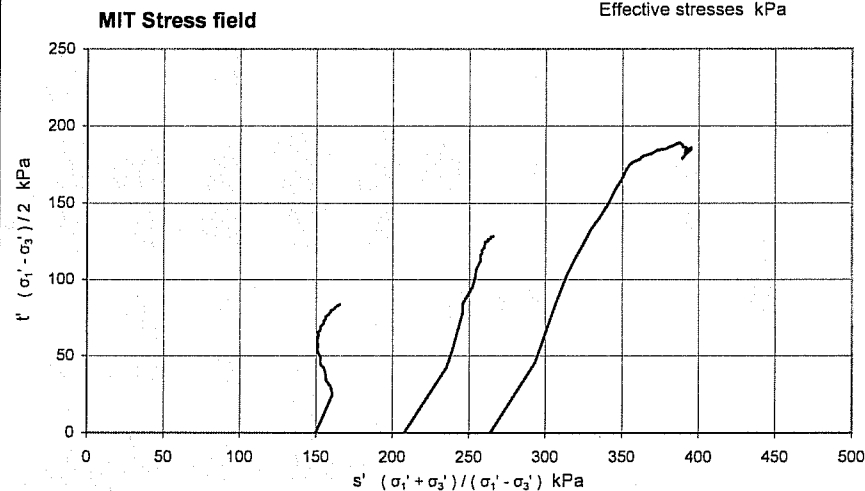
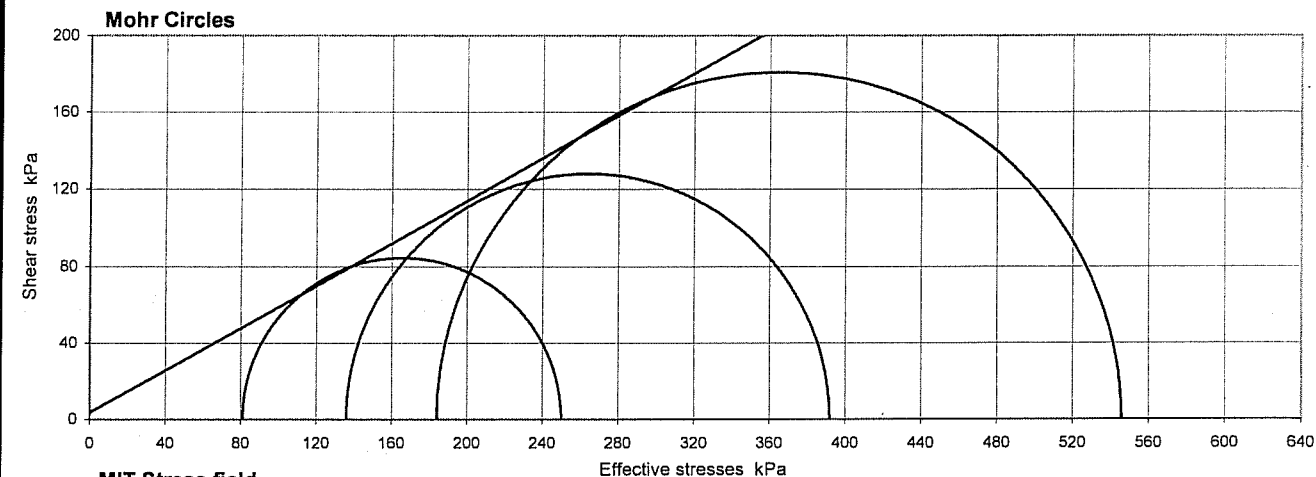
Figure

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sheet 2 of 3

Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure (BS1377 : Part 8 : 1990) - Multistage test on a single specimen

Project No	A1077-11	Sample Details:	Hole No	BH2
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	10.00-10.45
			No	22
			Type	U
			ID	
			Spec Ref	

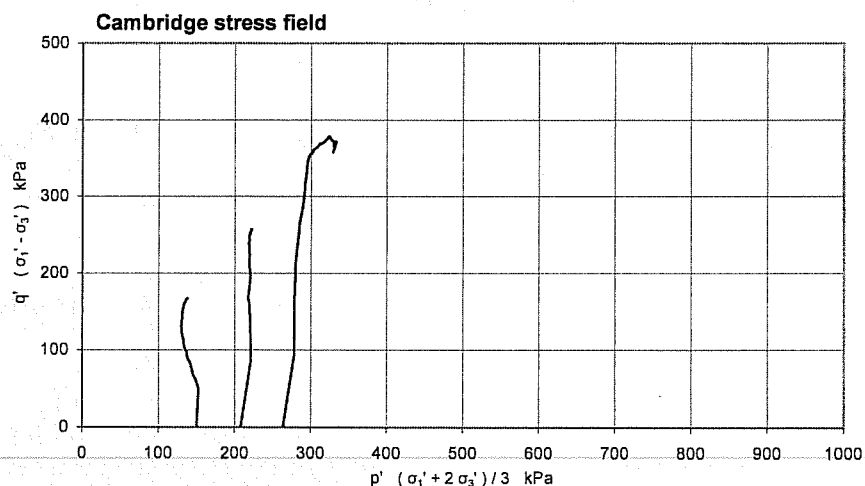


Compression stages

Stage	1	2	3	
Cell pressure	450	510	565	kPa
Initial pwp	300	302	301	kPa
Initial σ_3'	150	208	264	kPa
Rate of strain	0.70	0.70	0.70	%/hr

Failure conditions

Criterion	Maximum effective principal stress ratio			
Axial strain	2.90	4.64	7.75	%
$(\sigma_1'/\sigma_3')_f$	3.085	2.881	2.966	
$(\sigma_1' - \sigma_3')_f$	168.9	255.9	361.8	kPa
u_f	369	374	381	kPa
$\sigma_3'_f$	81	136	184	kPa
$\sigma_1'_f$	250	392	546	kPa
A_f	0.41	0.28	0.22	
Time to failure	4.1	6.6	11.1	hrs



Shear Strength Parameters

at peak stress ratio

		Linear regression	
c'	kPa	3.5	
ϕ'	degrees	28.9	
		Manual re-assessment	
c'	kPa	-	
ϕ'	degrees	-	

Mode of failure



Notes : Deviator stresses corrected for area change, vertical side drains and 0.45 mm thick rubber membrane(s)

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Figure

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**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure
(BS1377 : Part 8 : 1990) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH3
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	10.00-10.45
			No	28
			Type	U
			ID	
			Spec Ref	

Specimen Details

Initial		
Length	mm	204.74
Diameter	mm	102.64
Bulk Density	Mg/m³	2.21
Water Content	%	14
Dry density	Mg/m³	1.94

After test		
Bulk Density	Mg/m³	2.26
Water Content	%	13
Dry density	Mg/m³	2.01

Soil Description Firm to stiff greyish brown slightly sandy slightly gravelly CLAY.

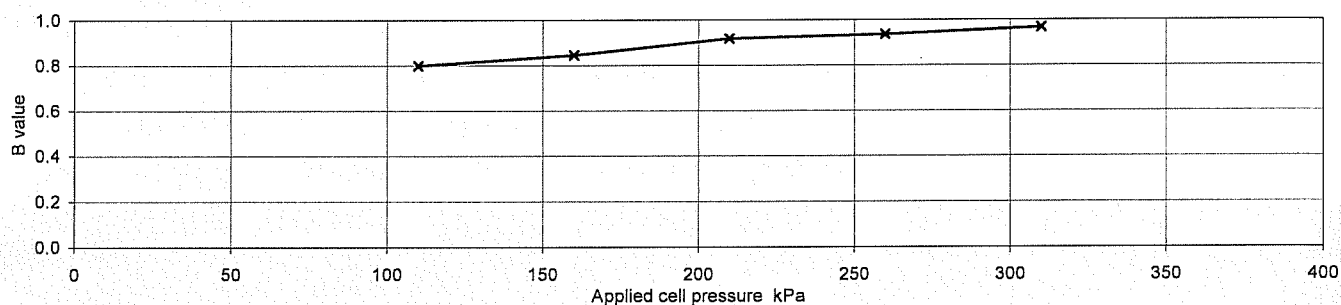
Specimen Type /Preparation UNDISTURBED

Saturation Details

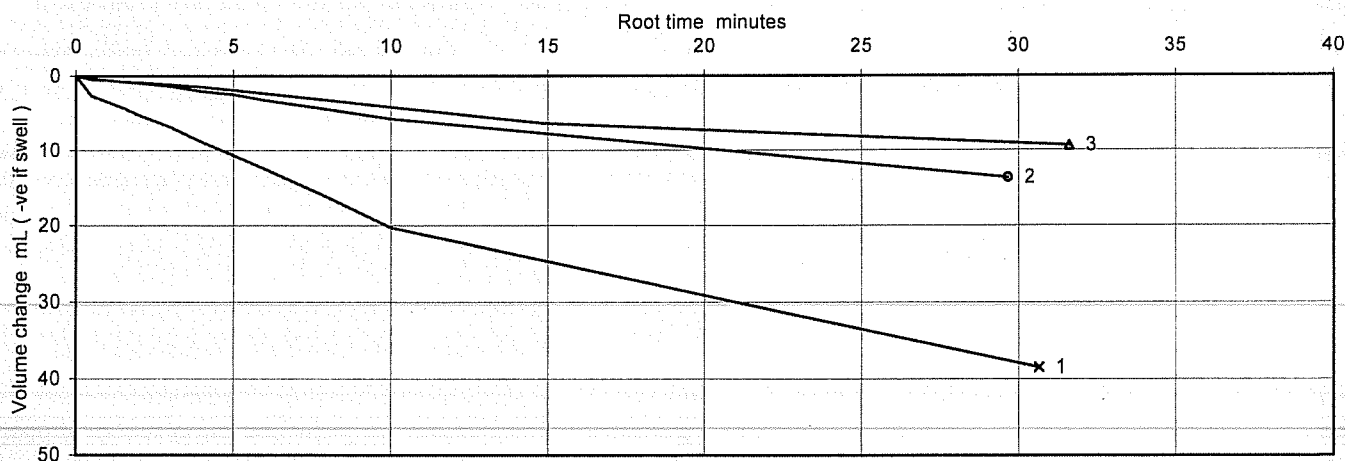
Method of Saturation

Increments of cell and back pressure

Cell pressure increments	kPa	50
Differential Pressure	kPa	10
Final Cell Pressure	kPa	310
Final pore water pressure	kPa	250
Final B Value		0.97



Consolidation Details	Drainage Conditions		From radial boundary and one end			
	Stage No.		1	2	3	
	Cell Pressure applied		450	510	565	kPa
	Back Pressure applied		300	300	300	kPa
	Effective Pressure		150	210	265	kPa
	Pore pressure at start of consolidation		437	420	427	kPa
	Pore pressure at end of consolidation		303	305	306	kPa
	Pore pressure dissipation at end of consolidation		98	96	95	%
Consolidation parameters (see note to BS1377 : pt 8, clause 6.3.4)	Coefficient of Consolidation	C_{vi}	0.56	0.41	0.46	m²/year
	Coefficient of Compressibility	M_{vi}	0.17	0.07	0.05	m²/MN
	Coefficient of Permeability (calculated)	k_{vi}	3.0E-11	9.1E-12	6.8E-12	m/s



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Figure

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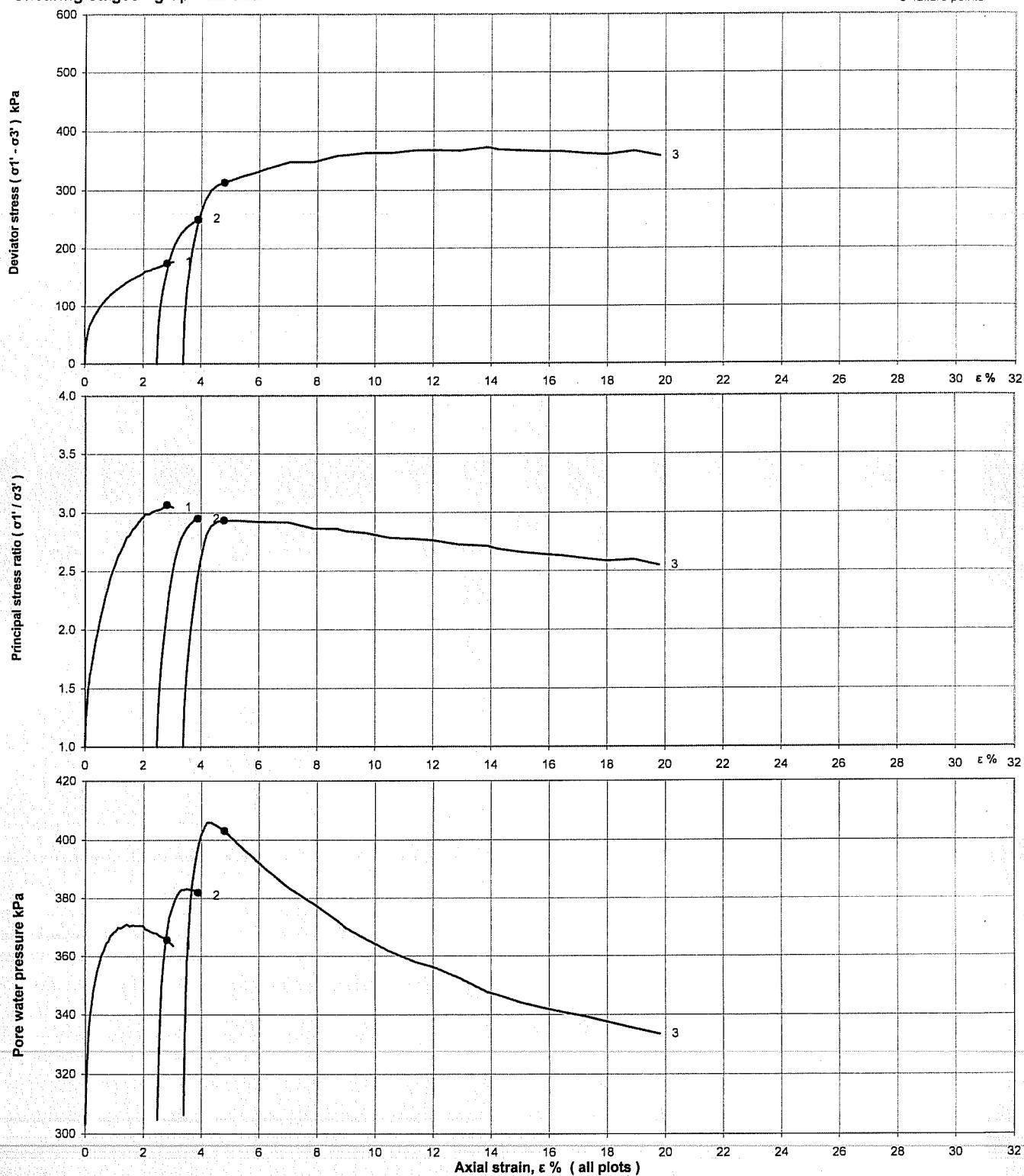
sheet 1 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure
(BS1377 : Part 8 : 1990) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No		BH3	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		10.00-10.45	
			No	28	Type	U
			ID			
			Spec Ref			

Shearing stages - graphical data

o failure points



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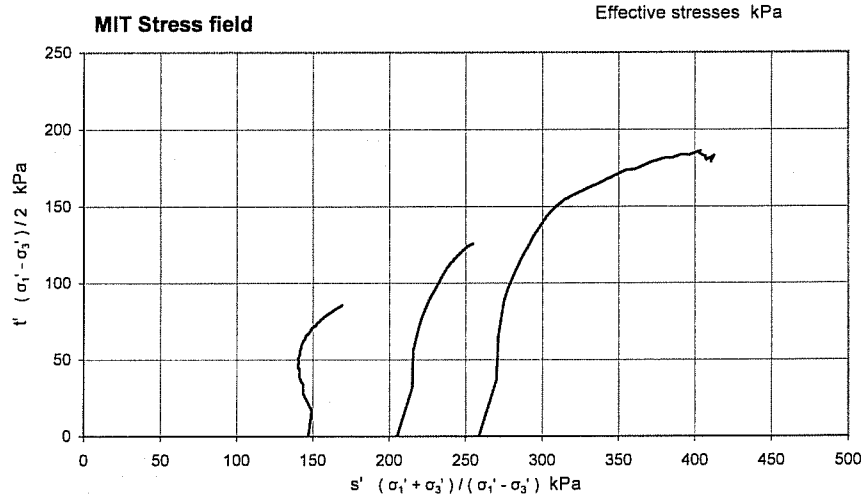
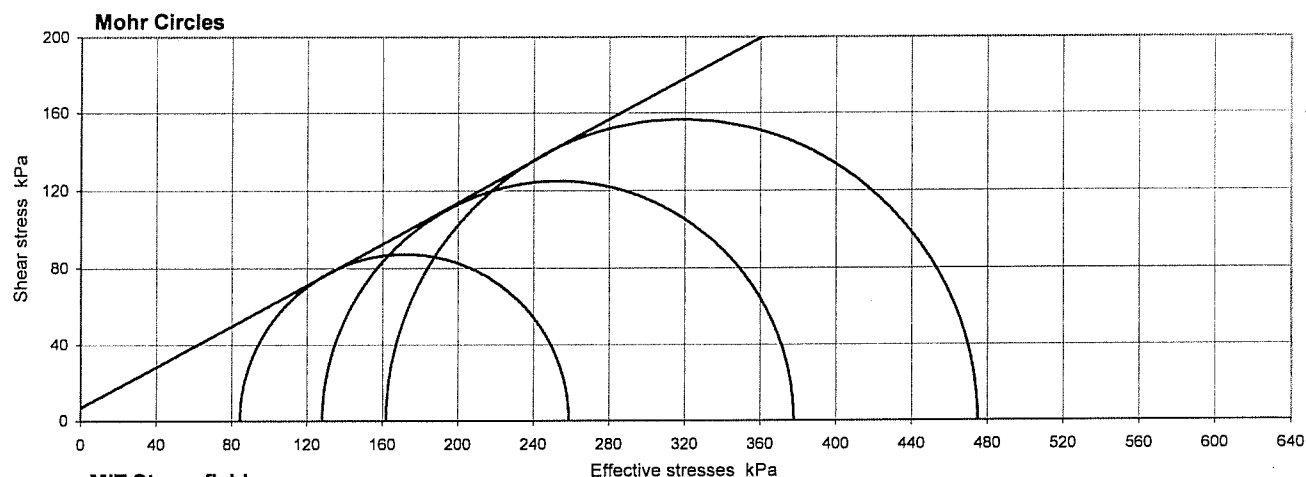
Figure

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Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure (BS1377 : Part 8 : 1990) - Multistage test on a single specimen

Project No	A1077-11	Sample Details:	Hole No	BH3
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	10.00-10.45
			No	28
			Type	U
			ID	
			Spec Ref	

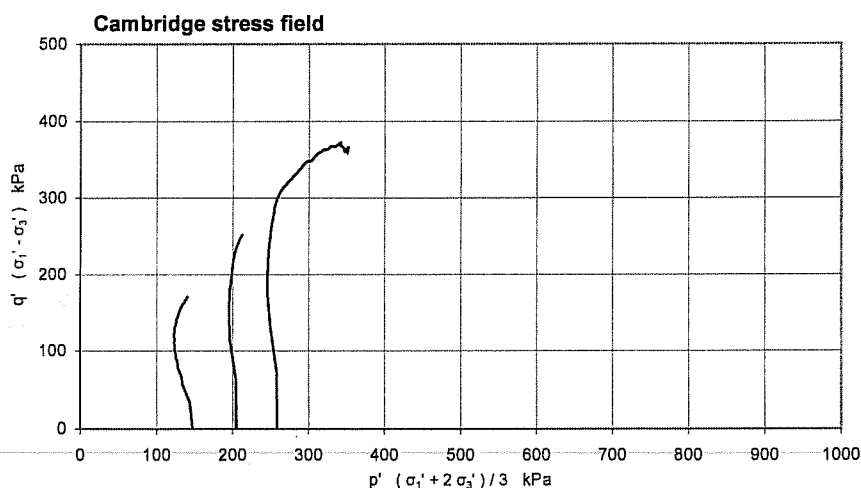


Compression stages

Stage	1	2	3	
Cell pressure	450	510	565	kPa
Initial pwp	303	305	306	kPa
Initial σ_3'	147	205	259	kPa
Rate of strain	0.43	0.43	0.43	%/hr

Failure conditions

Criterion	Maximum effective principal stress ratio			
Axial strain	2.82	3.89	4.80	%
$(\sigma_1' / \sigma_3')_f$	3.066	2.950	2.933	
$(\sigma_1' - \sigma_3')_f$	174.4	249.7	312.9	kPa
u_f	366	382	403	kPa
$\sigma_3'_f$	84	128	162	kPa
$\sigma_1'_f$	259	378	475	kPa
A_f	0.36	0.31	0.31	
Time to failure	6.6	9.1	11.2	hrs



Shear Strength Parameters

at peak stress ratio

		Linear regression	
c'	kPa	6.9	
ϕ'	degrees	28.1	
		Manual re-assessment	
c'	kPa	-	
ϕ'	degrees	-	

Notes : Deviator stresses corrected for area change, vertical side drains and 0.9 mm thick rubber membrane(s)

Mode of failure



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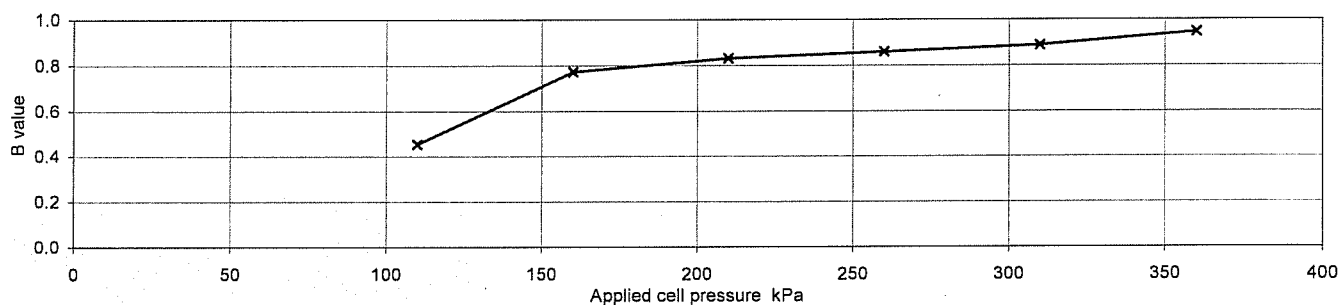
**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure
(BS1377 : Part 8 : 1990) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH4
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	3.00-3.45
			No	16
			Type	U
			ID	
			Spec Ref	

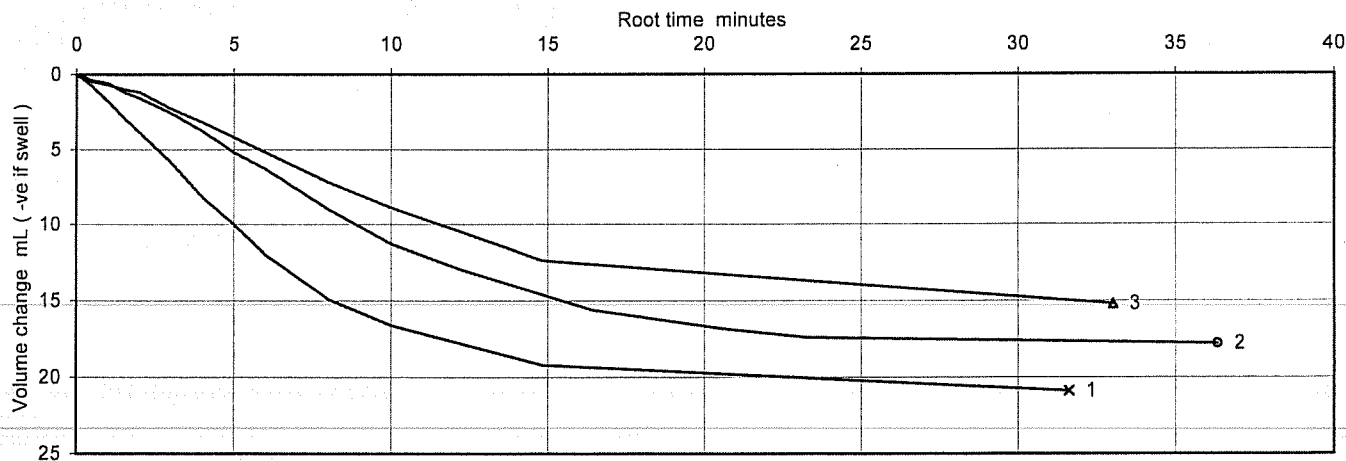
Specimen Details		
Initial		
Length	mm	204.09
Diameter	mm	102.86
Bulk Density	Mg/m ³	1.96
Water Content	%	23
Dry density	Mg/m ³	1.60
After test		
Bulk Density	Mg/m ³	2.02
Water Content	%	23
Dry density	Mg/m ³	1.65

Soil Description	Firm greyish brown slightly sandy slightly gravelly CLAY.
Specimen Type /Preparation	UNDISTURBED

Saturation Details		Method of Saturation
		Increments of cell and back pressure
Cell pressure increments	kPa	50
Differential Pressure	kPa	10
Final Cell Pressure	kPa	360
Final pore water pressure	kPa	300
Final B Value		0.95



Consolidation Details		Drainage Conditions			
		Stage No.	1	2	3
	Cell Pressure applied		400	420	440
	Back Pressure applied		350	350	350
	Effective Pressure		50	70	90
	Pore pressure at start of consolidation		385	393	401
	Pore pressure at end of consolidation		351	350	352
	Pore pressure dissipation at end of consolidation		99	99	97
					%
Consolidation parameters (see note to BS1377 : pt 8, clause 6.3.4)	Coefficient of Consolidation	C _{vi}	2.01	0.98	0.83
	Coefficient of Compressibility	M _{vi}	0.36	0.25	0.19
	Coefficient of Permeability (calculated)	k _{vi}	2.2E-10	7.5E-11	4.8E-11



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Figure

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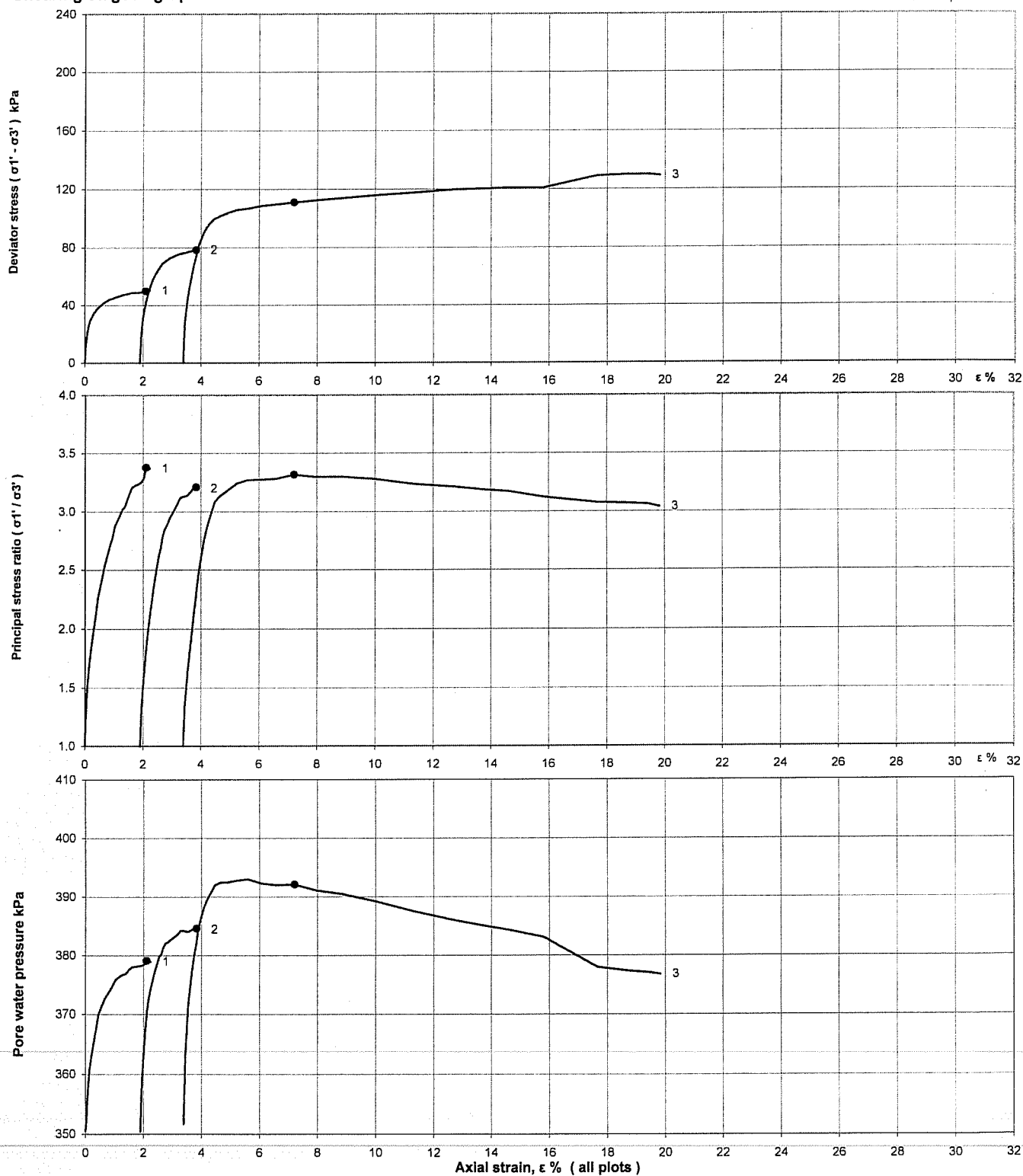
sheet 1 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure
(BS1377 : Part 8 : 1990) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No		BH4		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		3.00-3.45		
			No	16	Type	U	
			ID				
			Spec Ref				

Shearing stages - graphical data

o failure points



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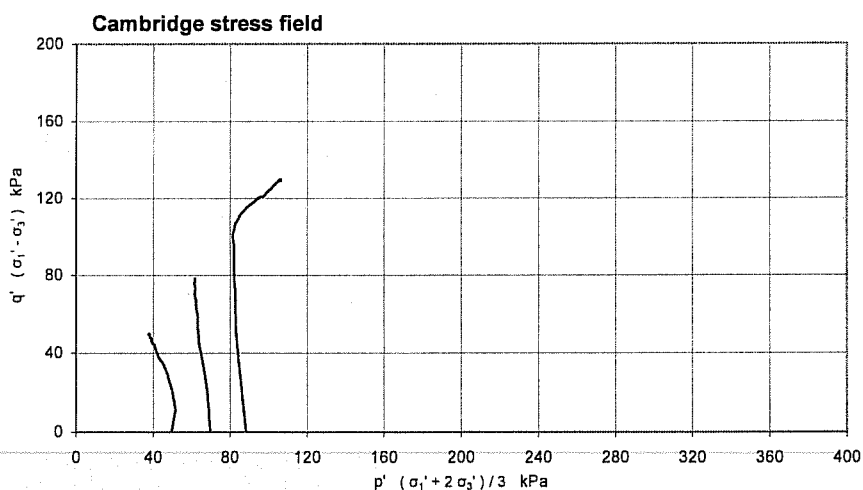
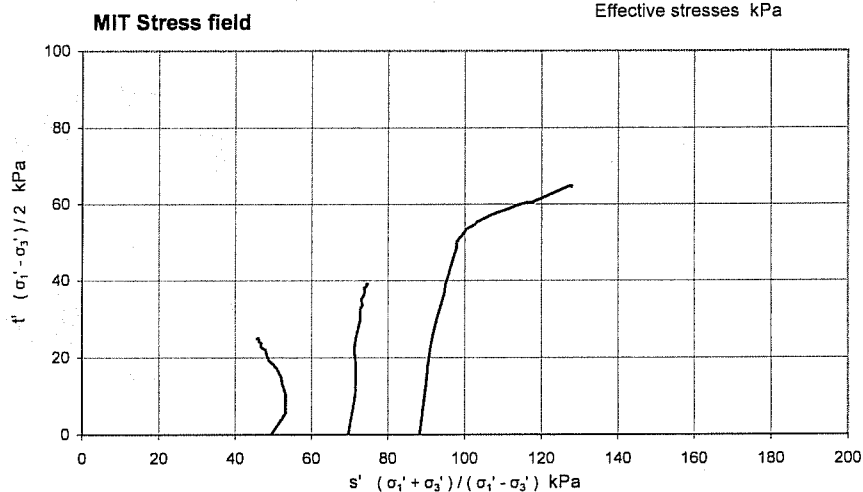
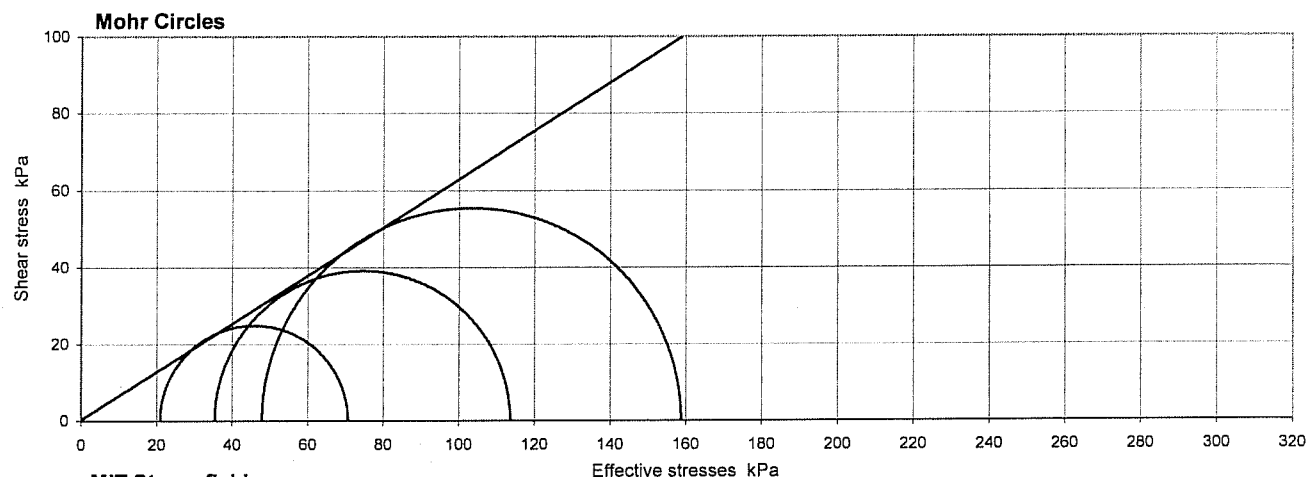
Figure

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Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure (BS1377 : Part 8 : 1990) - Multistage test on a single specimen

Project No	A1077-11	Sample Details:	Hole No	BH4
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	3.00-3.45
			No	16
			Type	U
			ID	
			Spec Ref	



Compression stages

Stage	1	2	3	
Cell pressure	400	420	440	kPa
Initial pwp	351	350	352	kPa
Initial σ_3'	50	70	88	kPa
Rate of strain	1.51	1.51	1.51	%/hr

Failure conditions

Criterion	Maximum effective principal stress ratio			
Axial strain	2.10	3.84	7.22	%
$(\sigma_1' / \sigma_3')_f$	3.378	3.209	3.315	
$(\sigma_1' - \sigma_3')_f$	49.7	78.2	110.9	kPa
u_f	379	385	392	kPa
$\sigma_3' f$	21	35	48	kPa
$\sigma_1' f$	71	114	159	kPa
A_f	0.58	0.44	0.36	
Time to failure	1.4	2.5	4.8	hrs

Shear Strength Parameters

at peak stress ratio

		Linear regression	
c'	kPa	0.2	
ϕ'	degrees	32.1	
		Manual re-assessment	
c'	kPa	-	
ϕ'	degrees	-	

Notes : Deviator stresses corrected for area change, vertical side drains and 0.45 mm thick rubber membrane(s)

Mode of failure



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Figure

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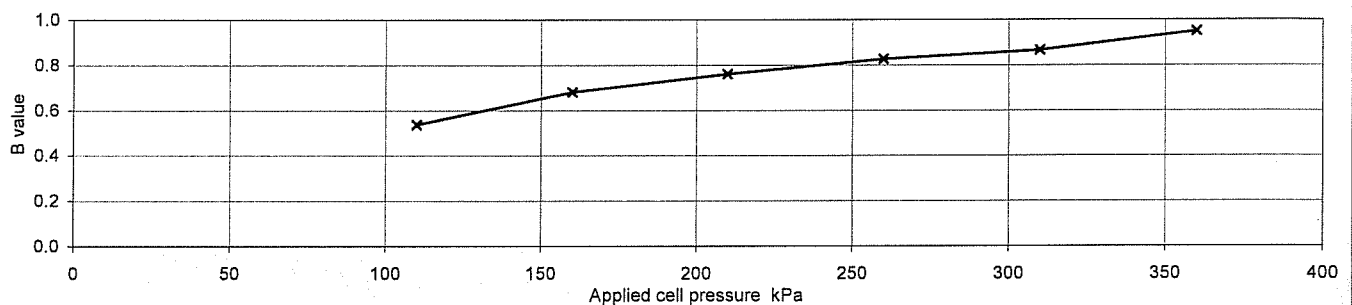
**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure
(BS1377 : Part 8 : 1990) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH5
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	3.00-3.45
			No	14
			Type	U
			ID	
			Spec Ref	

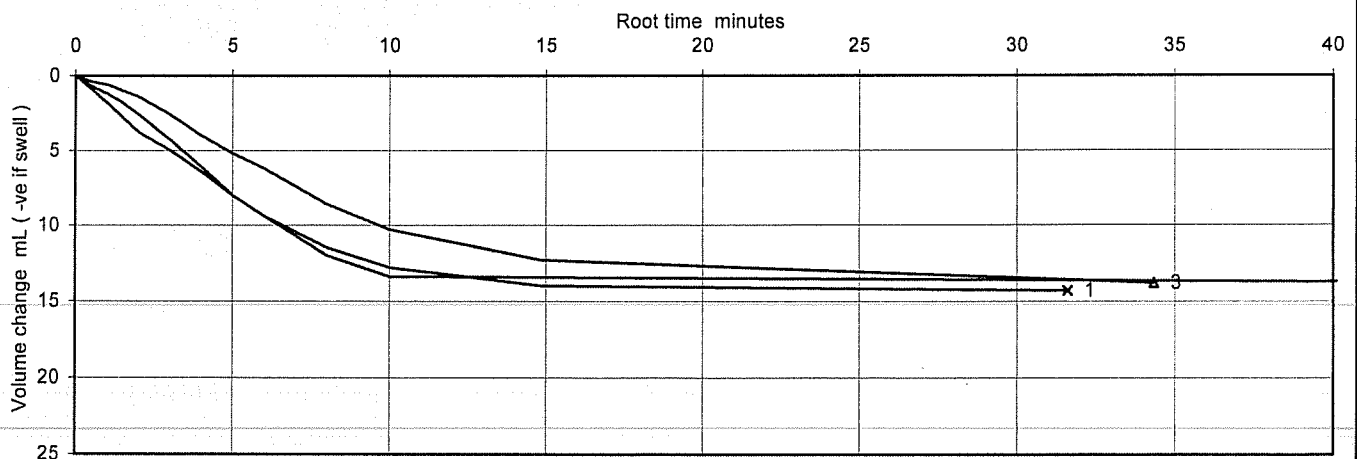
Specimen Details		
Initial		
Length	mm	174.16
Diameter	mm	103.45
Bulk Density	Mg/m ³	1.87
Water Content	%	22
Dry density	Mg/m ³	1.53
After test		
Bulk Density	Mg/m ³	2.02
Water Content	%	23
Dry density	Mg/m ³	1.63

Soil Description	Stiff greyish brown slightly sandy slightly gravelly CLAY.
Specimen Type /Preparation	UNDISTURBED

Saturation Details		Method of Saturation
		Increments of cell and back pressure
Cell pressure increments	kPa	50
Differential Pressure	kPa	10
Final Cell Pressure	kPa	360
Final pore water pressure	kPa	300
Final B Value		0.95



Consolidation Details		Drainage Conditions			
		Stage No.	1	2	3
	Cell Pressure applied		400	420	440
	Back Pressure applied		350	350	350
	Effective Pressure		50	70	90
	Pore pressure at start of consolidation		390	399	409
	Pore pressure at end of consolidation		350	350	350
	Pore pressure dissipation at end of consolidation		100	99	99
					%
Consolidation parameters (see note to BS1377 : pt 8, clause 6.3.4)	Coefficient of Consolidation	C _{vi}	3.27	3.88	1.75
	Coefficient of Compressibility	M _{vi}	0.25	0.21	0.17
	Coefficient of Permeability (calculated)	k _{vi}	2.5E-10	2.5E-10	9.3E-11



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Figure

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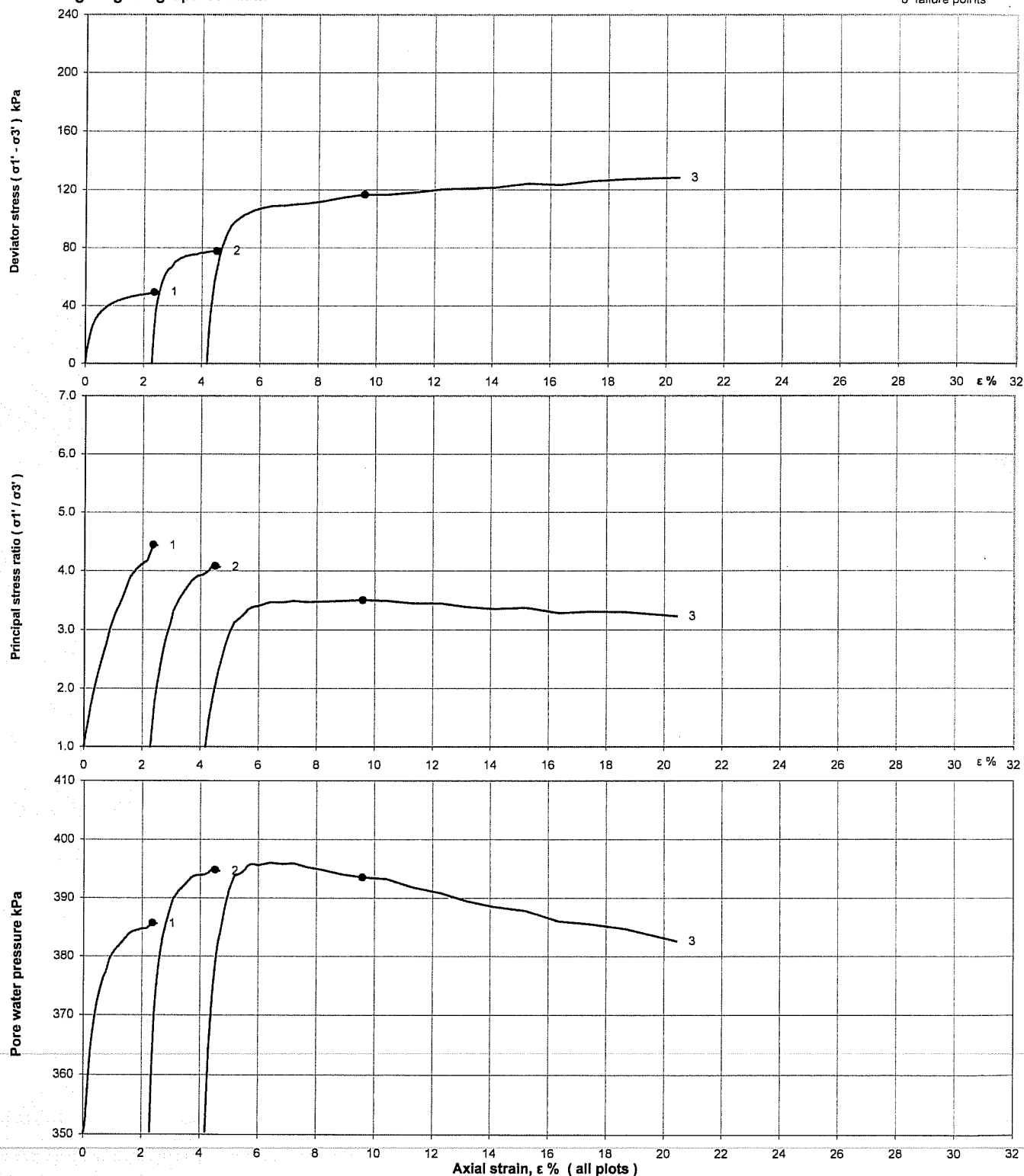
sheet 1 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure
(BS1377 : Part 8 : 1990) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No		BH5	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		3.00-3.45	
			No	14	Type	U
			ID			
			Spec Ref			

Shearing stages - graphical data

o failure points



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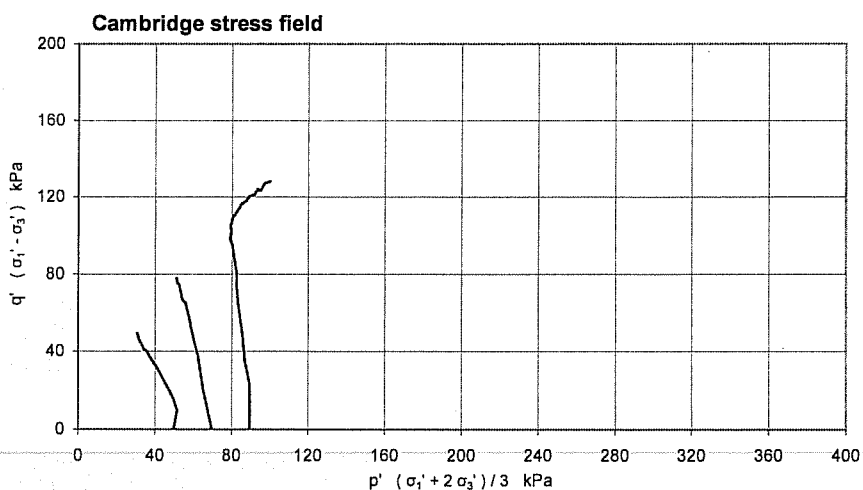
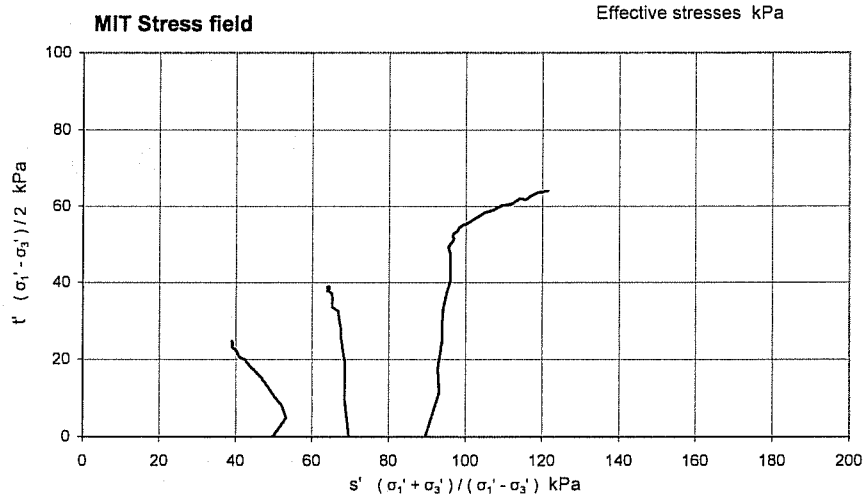
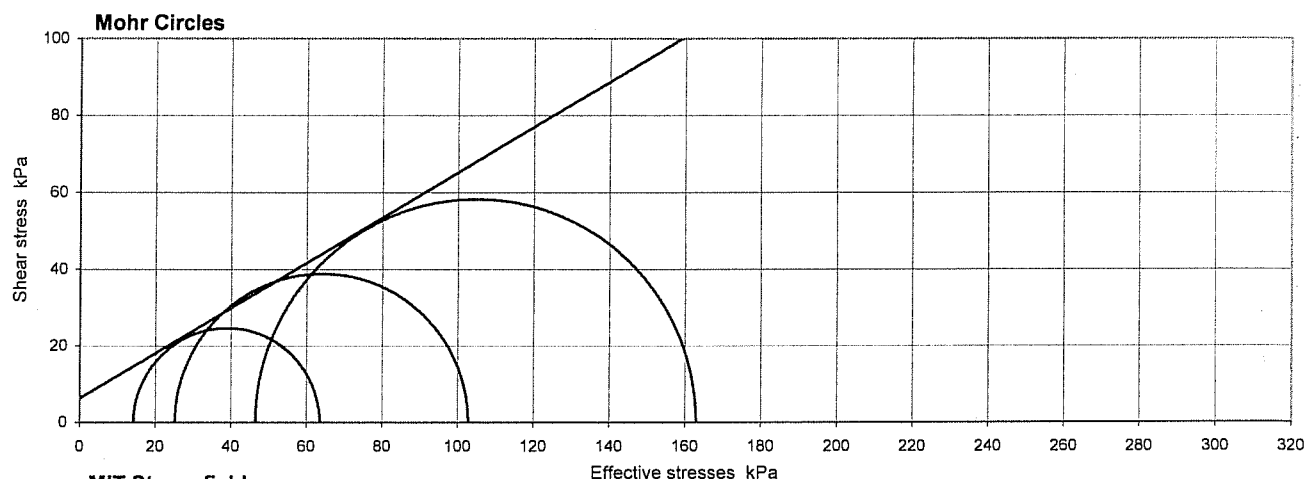
Figure

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Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure (BS1377 : Part 8 : 1990) - Multistage test on a single specimen

Project No	A1077-11	Sample Details:	Hole No	BH5
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	3.00-3.45
			No	14
			Type	U
			ID	
			Spec Ref	



Compression stages

Stage	1	2	3	
Cell pressure	400	420	440	kPa
Initial pwp	350	350	350	kPa
Initial σ_3'	50	70	90	kPa
Rate of strain	1.93	1.93	1.93	%/hr

Failure conditions

Criterion	Maximum effective principal stress ratio			
Axial strain	2.37	4.51	9.59	%
$(\sigma_1' / \sigma_3')_f$	4.442	4.078	3.505	
$(\sigma_1' - \sigma_3')_f$	49.2	77.6	116.5	kPa
u_f	386	395	394	kPa
$\sigma_3' f$	14	25	47	kPa
$\sigma_1' f$	64	103	163	kPa
A_f	0.72	0.57	0.37	
Time to failure	1.2	2.3	5.0	hrs

Shear Strength Parameters

at peak stress ratio

		Linear regression	
c'	kPa	6.3	
ϕ'	degrees	30.5	
		Manual re-assessment	
c'	kPa	-	
ϕ'	degrees	-	

Notes : Deviator stresses corrected for area change, vertical side drains and 0.45 mm thick rubber membrane(s)
Test specimen contains natural break.

Mode of failure



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Figure

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**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure
(BS1377 : Part 8 : 1990) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH6
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	3.00-3.45
			No	14
			Type	U
			ID	
			Spec Ref	

Specimen Details

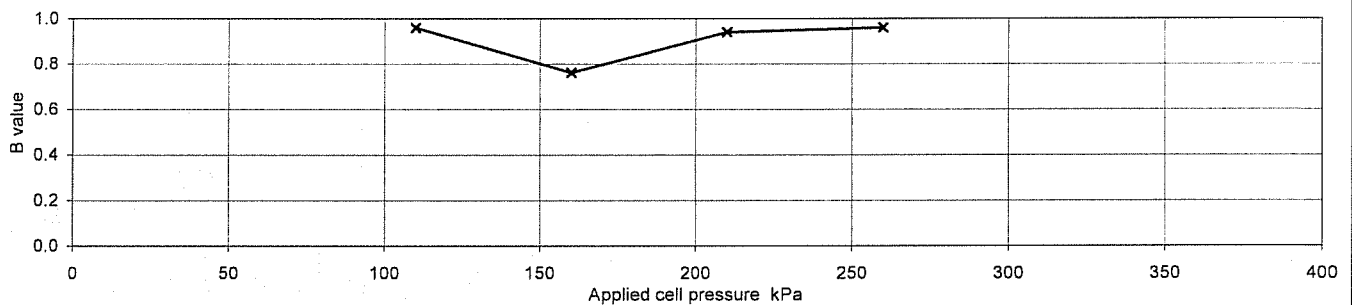
Initial		
Length	mm	202.63
Diameter	mm	103.04
Bulk Density	Mg/m ³	2.08
Water Content	%	21
Dry density	Mg/m ³	1.72
After test		
Bulk Density	Mg/m ³	2.13
Water Content	%	20
Dry density	Mg/m ³	1.78

Soil Description Firm brownish grey slightly sandy slightly gravelly CLAY.

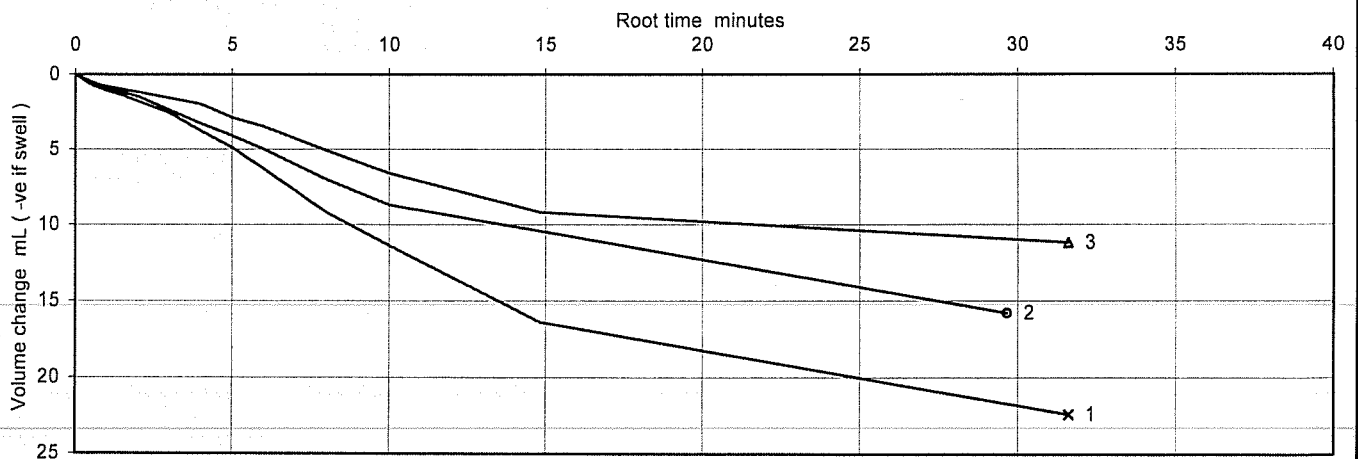
Specimen Type /Preparation UNDISTURBED

Saturation Details

		Method of Saturation
		Increments of cell and back pressure
Cell pressure increments	kPa	50
Differential Pressure	kPa	10
Final Cell Pressure	kPa	260
Final pore water pressure	kPa	200
Final B Value		0.96



Consolidation Details		Drainage Conditions			
		Stage No.	1	2	3
		Cell Pressure applied	350	370	390
		Back Pressure applied	300	300	300
		Effective Pressure	50	70	90
		Pore pressure at start of consolidation	334	348	350
		Pore pressure at end of consolidation	300	300	300
		Pore pressure dissipation at end of consolidation	100	100	100
Consolidation parameters (see note to BS1377 : pt 8, clause 6.3.4)	Coefficient of Consolidation	C _{vi}	0.70	0.72	0.86
	Coefficient of Compressibility	M _{vi}	0.39	0.20	0.14
	Coefficient of Permeability (calculated)	k _{vi}	8.5E-11	4.4E-11	3.6E-11



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Figure

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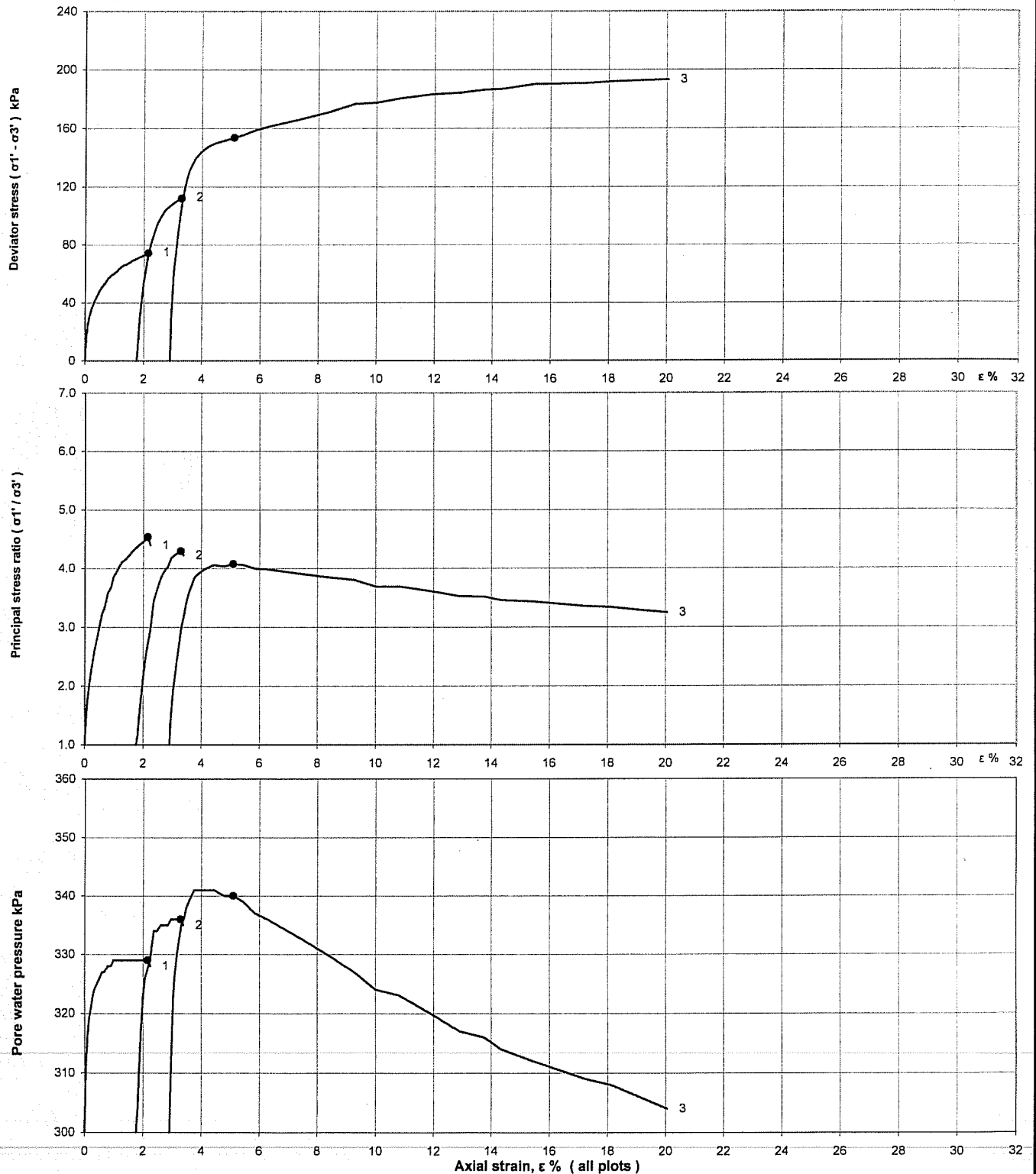
sheet 1 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure
(BS1377 : Part 8 : 1990) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No		BH6	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		3.00-3.45	
			No	14	Type	U
			ID			
			Spec Ref			

Shearing stages - graphical data

o failure points



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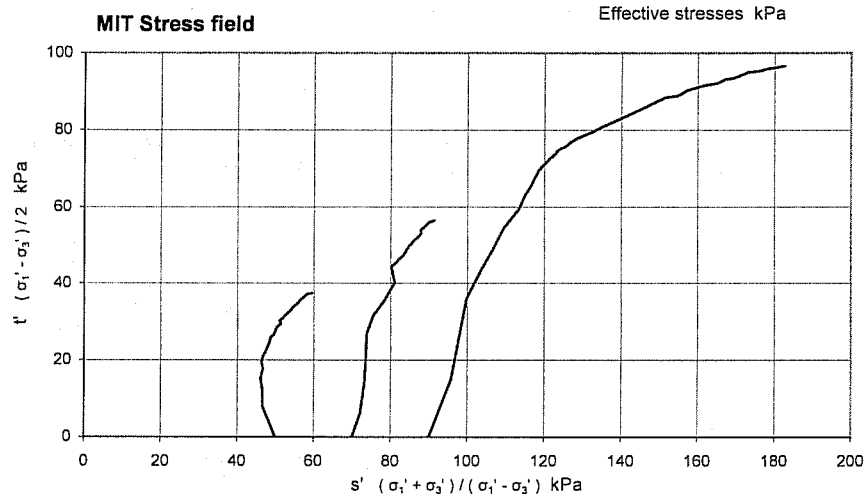
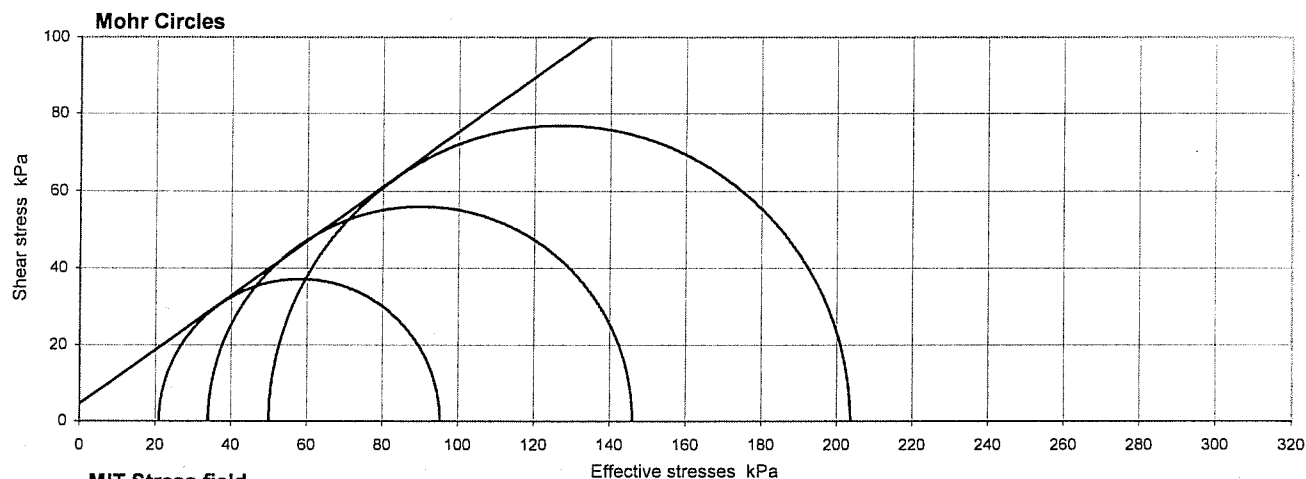
Figure

CUM 10

sheet 2 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure
(BS1377 : Part 8 : 1990) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH6
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	3.00-3.45
			No	14
			ID	
			Spec Ref	
			Type	U

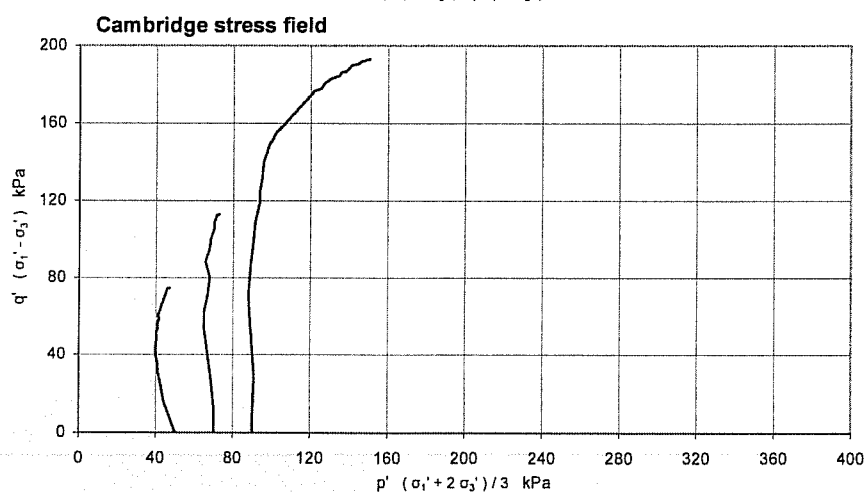


Compression stages

Stage	1	2	3	
Cell pressure	350	370	390	kPa
Initial pwp	300	300	300	kPa
Initial σ_3'	50	70	90	kPa
Rate of strain	0.52	0.52	0.52	%/hr

Failure conditions

Criterion	Maximum effective principal stress ratio			
Axial strain	2.16	3.30	5.10	%
$(\sigma_1' / \sigma_3')_f$	4.535	4.295	4.073	
$(\sigma_1' - \sigma_3')_f$	74.2	112.0	153.6	kPa
u_f	329	336	340	kPa
$\sigma_3' f$	21	34	50	kPa
$\sigma_1' f$	95	146	204	kPa
A_f	0.39	0.32	0.26	
Time to failure	4.1	6.3	9.8	hrs



Shear Strength Parameters

at peak stress ratio

		Linear regression	
c'	kPa	4.6	
ϕ'	degrees	35.3	
		Manual re-assessment	
c'	kPa	-	
ϕ'	degrees	-	

Mode of failure



Notes : Deviator stresses corrected for area change, vertical side drains and 0.9 mm thick rubber membrane(s)

Ref

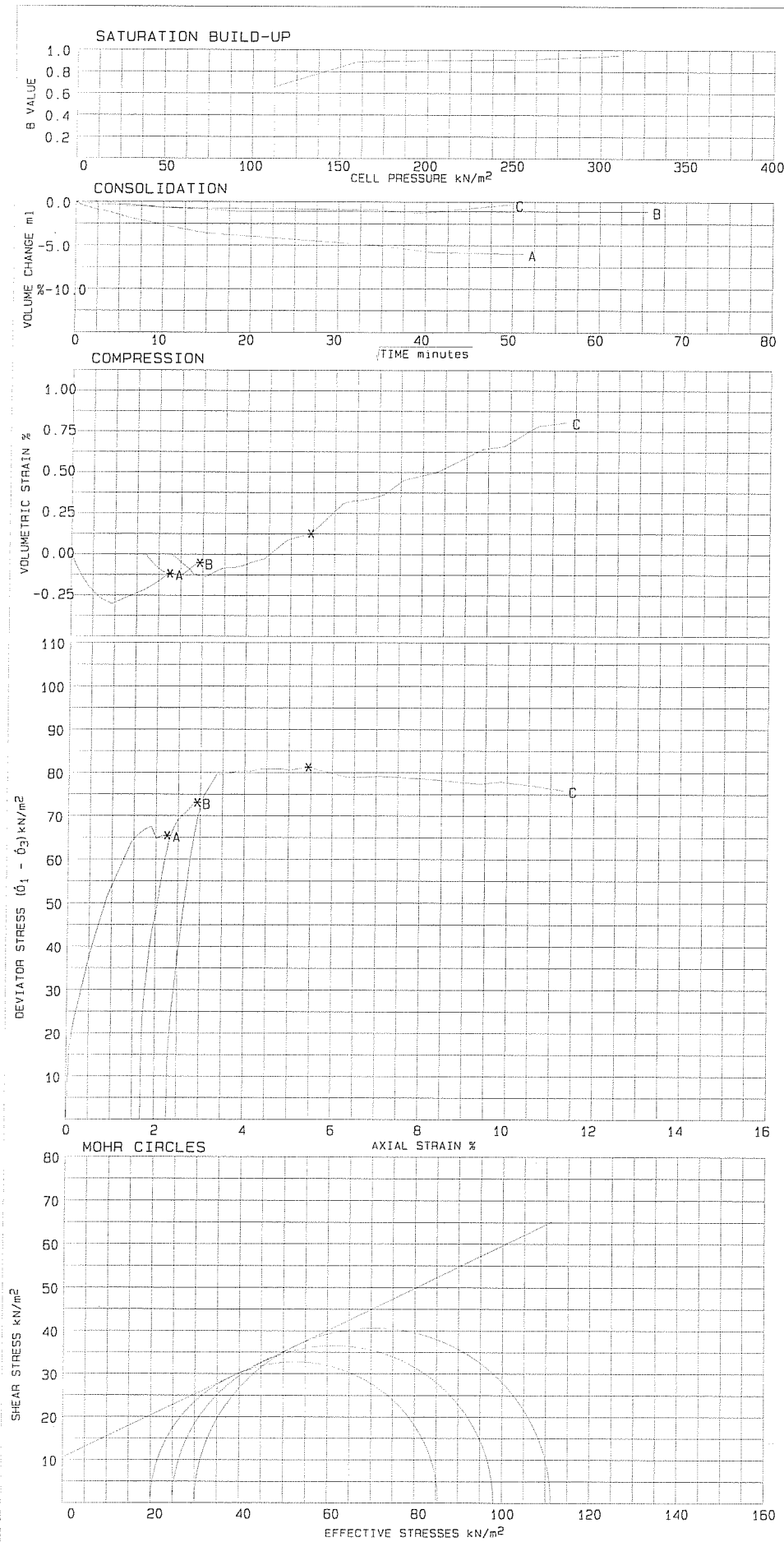
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Figure

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SPECIMEN		A	B	C
INITIAL	Density Mg/m ³	2.11	2.10	2.10
	Moisture %	19	18	18
	Dry Density Mg/m ³	1.78	1.77	1.77
AFTER CONSOLIDATION	Density Mg/m ³	2.10	2.10	2.09
	Moisture %	18	18	18
	Dry Density Mg/m ³	1.77	1.77	1.77
AFTER TEST	Density Mg/m ³	2.10	2.10	2.08
	Moisture %	18	18	19
	Dry Density Mg/m ³	1.77	1.77	1.75
SATURATION	Initial pwp	0	0	0
	Saturated pwp	249	249	249
	Final pwp pressure	260	260	260
	B value	0.95	0.95	0.95
CONSOLIDATION STAGE	Cell Pressure	320	325	330
	Back Pressure	300	300	300
	Initial pwp	309	303	305
	Final pwp	300	300	300
CONSOLIDATION PARAMETERS	Cv ₃ m ² /year	0.43	0.64	0.67
	mv ₃ m ² /MN	0.42	0.21	0.03
COMPRESSION STAGE	Cell Pressure	320	325	330
	Back pressure	300	300	300
	σ_3'	20	25	30
	Rate of % per strain hour	0.13	0.13	0.13
FAILURE CONDITIONS	Axial Strain %	2.3	2.9	5.5
	Volumetric Strain %	-0.1	-0.1	0.1
	($\sigma_1 - \sigma_3$) _f	65	73	81
	σ_1'	85	98	111
	Time to failure hours	17.4	22.5	41.9
	MODE OF FAILURE			

SHEAR STRENGTH PARAMETERS		CD	ØD
BY LINEAR REGRESSION		11 kN/m ²	26 °

NOTES
Saturation by application of cell/back pressure increments of 50 kN/m² with a differential of 10 kN/m².
Drainage during consolidation and shearing to top with Vertical side drains fitted
Stress/strain curves corrected for area change, side drains and .45 mm thick membrane

SOIL DESCRIPTION	
Firm brown slightly sandy, slightly gravelly CLAY.	
TYPE OF SPECIMEN	
UNDISTURBED	
SPECIMEN DIMENSIONS	
102.5 mm dia x 203.9 mm long	

Test carried out to BS 1377 : Part 8 : 1990 : Test 8

Consolidated Drained Triaxial Compression Test with measurement of volume change				Borehole No.	BH1	Depth	1.20-1.65m
				sample No.	U5		
SLR	Soil	Location	SANDSEND BOREHOLES, NORTH YORKSHIRE	Loc. No.		Fig.	
8.8	Mechanics			A1077-11			

**Determination of shear strength by direct shear (Small shearbox apparatus)
(BS1377 : Part 7 : clause 4 : 1990)**

Project No	A1077-11	Sample Details:	Hole No.		BH1	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		12.50-12.95	
			Sample No	33	Type	U
			ID			
			Spec Ref			

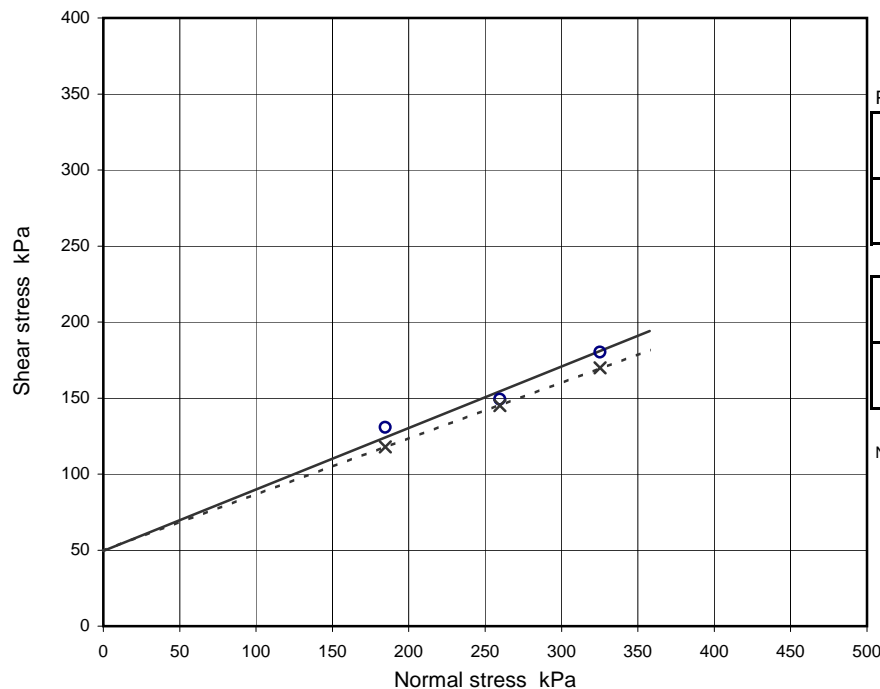
Soil Description	Firm to stiff brown slightly sandy slightly gravelly CLAY.
Specimen Type /Preparation	Undisturbed

Specimen(s) nominally 60mm x 60mm square
 Test(s) carried out in submerged condition
 Particle density, assumed 2.65 Mg/m³

Specimen Details		No.	1	2	3	4	5	6
Initial	Height	mm	23.4	23.4	23.4			
	Bulk Density	Mg/m ³	2.20	2.18	2.20			
	Water Content	%	14.5	14.6	14.9			
	Dry density	Mg/m ³	1.92	1.90	1.92			
	Voids ratio		0.378	0.395	0.383			
Consol ⁿ	Degree of Saturation	%	102	98	103			
	Consolidation / Normal Stress applied	kPa	185	260	325			
	Change in height during consolidation	mm	-0.872	-1.200	-1.334			
Shear see note 1	Voids ratio after consolidation		0.326	0.324	0.304			
	Voids ratio at end of test		0.246	0.252	0.220			
	Moisture content at end of test	%	9.3	9.5	8.3			
	Saturation at end of test	%	100	100	100			

Shearing stage

Rate of displacement	Peak	mm/min	0.005	0.005	0.005			
	Residual	mm/min	0.023	0.023	0.023			
Peak values, (o)	Relative displacement	mm	6.04	9.50	6.30			
	Shear stress	kPa	130.7	149.2	180.2			
Residual values, (x)	No. of reversals		2	2	2			
	Relative displacement	mm	20.00	30.00	10.00			
	Shear stress	kPa	118.0	145.2	169.8			



Shear Strength Parameters

Peak strength, (o)		Regression	Manual
c'	kPa	(64)	50
Ø'	degrees	(19½)	22

Residual strength, (x)			
c' _R	kPa	50	-
Ø' _R	degrees	20	-

Notes :

1 After shear values based on BS1377. Pt 7 cl. 4.6.1.6
 using δH calculated from consolidation and shear stages

Ref

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Figure

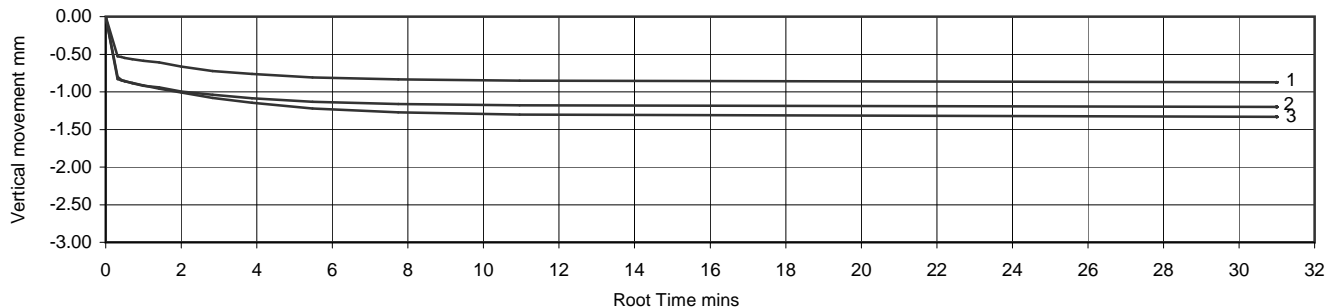
SSB 1

sheet 1 of 2

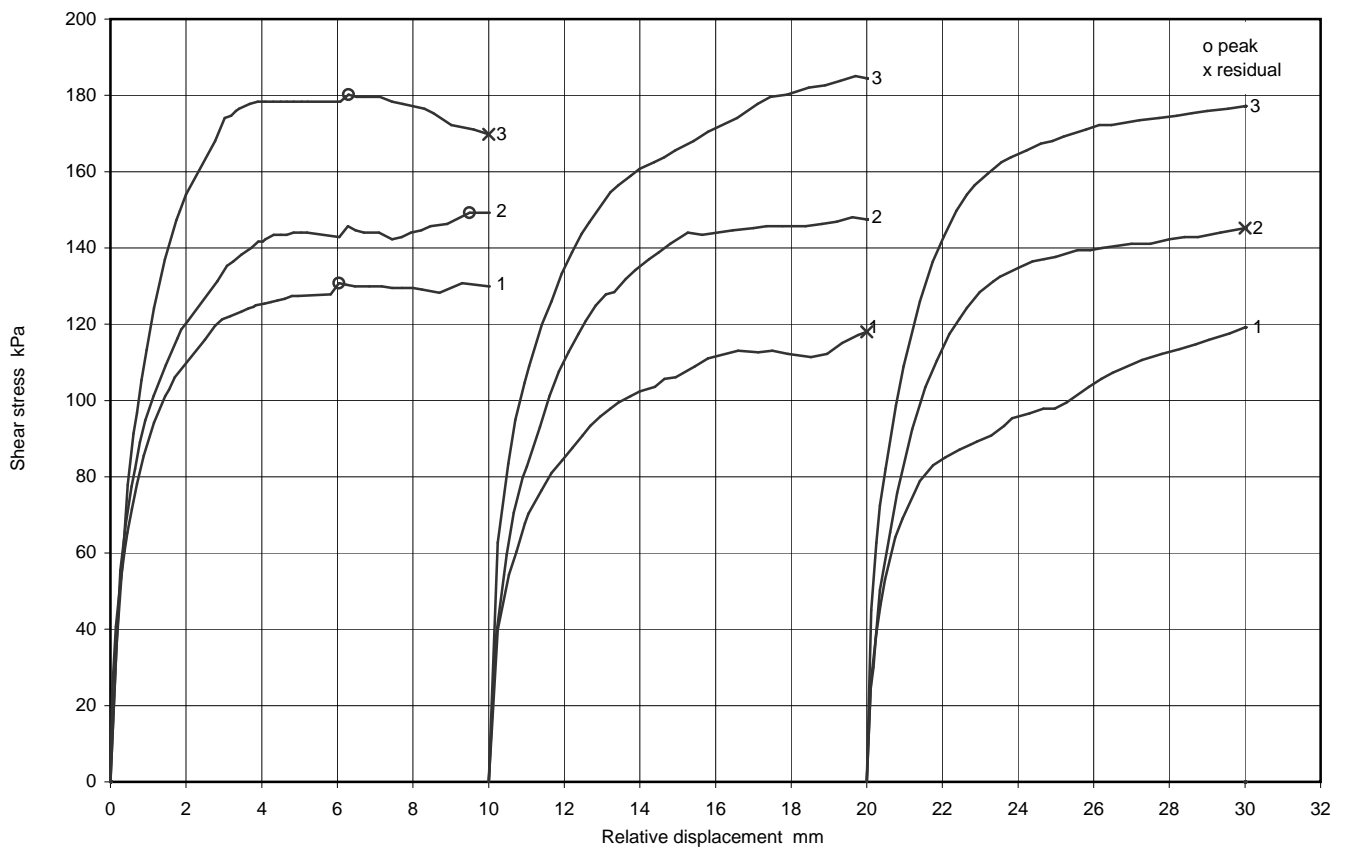
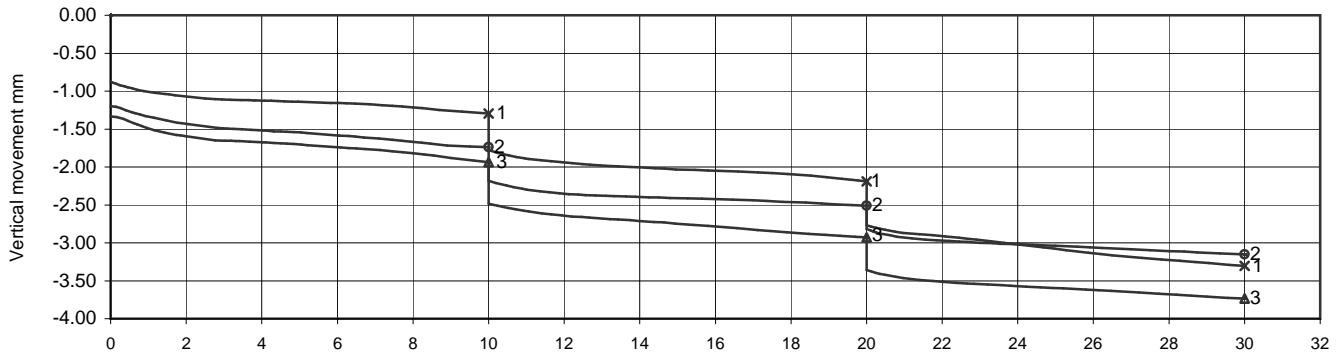
**Determination of shear strength by direct shear (Small shearbox apparatus)
(BS1377 : Part 7 : clause 4 : 1990)**

Project No	A1077-11	Sample Details:	Hole No.		BH1	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		12.50-12.95	
			Sample No	33	Type	U
			ID			
			Spec Ref			

Consolidation stage(s)



Shearing stage(s)



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Figure

SSB 1

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**Determination of shear strength by direct shear (Small shearbox apparatus)
(BS1377 : Part 7 : clause 4 : 1990)**

Project No	A1077-11	Sample Details:	Hole No.		BH2	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		5.00-5.45	
			Sample No	12	Type	U
			ID			
			Spec Ref			

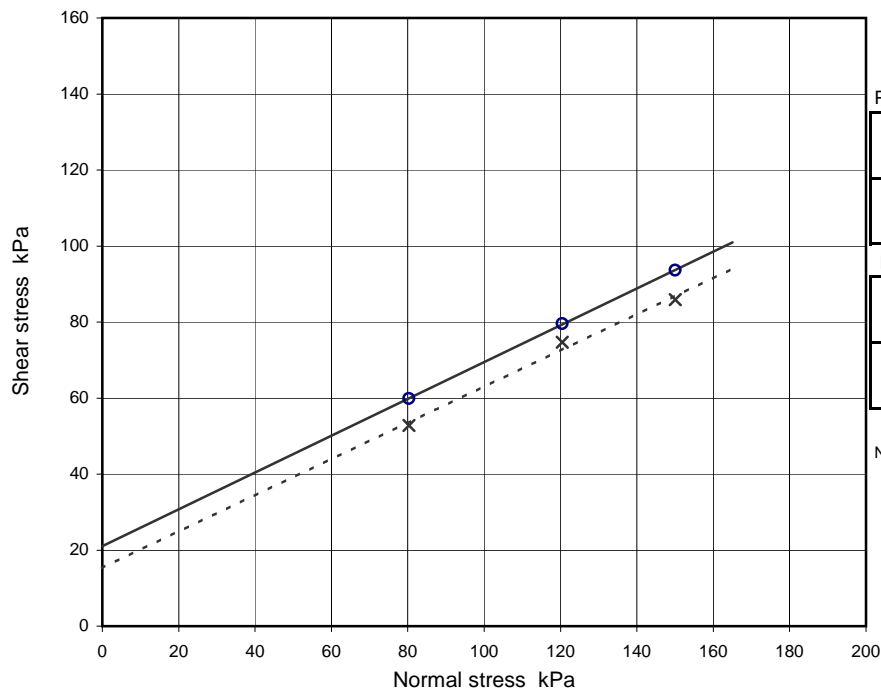
Soil Description	Firm brown slightly sandy slightly gravelly CLAY.
Specimen Type /Preparation	Undisturbed

Specimen(s) nominally 60mm x 60mm square
 Test(s) carried out in submerged condition
 Particle density, assumed 2.65 Mg/m³

Specimen Details		No.	1	2	3	4	5	6
Initial	Height	mm	23.4	23.4	23.4			
	Bulk Density	Mg/m ³	2.17	2.17	2.19			
	Water Content	%	14.2	14.2	13.9			
	Dry density	Mg/m ³	1.90	1.90	1.92			
	Voids ratio		0.395	0.396	0.377			
	Degree of Saturation	%	95	95	98			
Consol ⁿ	Consolidation / Normal Stress applied	kPa	80	120	150			
	Change in height during consolidation	mm	-0.436	-0.470	-0.582			
	Voids ratio after consolidation		0.369	0.368	0.343			
Shear see note 1	Voids ratio at end of test		0.303	0.354	0.309			
	Moisture content at end of test	%	11.4	13.4	11.7			
	Saturation at end of test	%	100	100	100			

Shearing stage

Rate of displacement	Peak	mm/min	0.009	0.009	0.009			
	Residual	mm/min	0.046	0.046	0.046			
Peak values, (o)	Relative displacement	mm	10.00	6.05	5.85			
	Shear stress	kPa	59.9	79.6	93.6			
Residual values, (x)	No. of reversals		2	2	2			
	Relative displacement	mm	30.00	28.43	28.75			
	Shear stress	kPa	52.8	74.7	85.9			



Shear Strength Parameters

Peak strength, (o)		Regression	Manual
c'	kPa	21	-
Ø'	degrees	26	-

Residual strength, (x)		Regression	Manual
c' _R	kPa	15	-
Ø' _R	degrees	25½	-

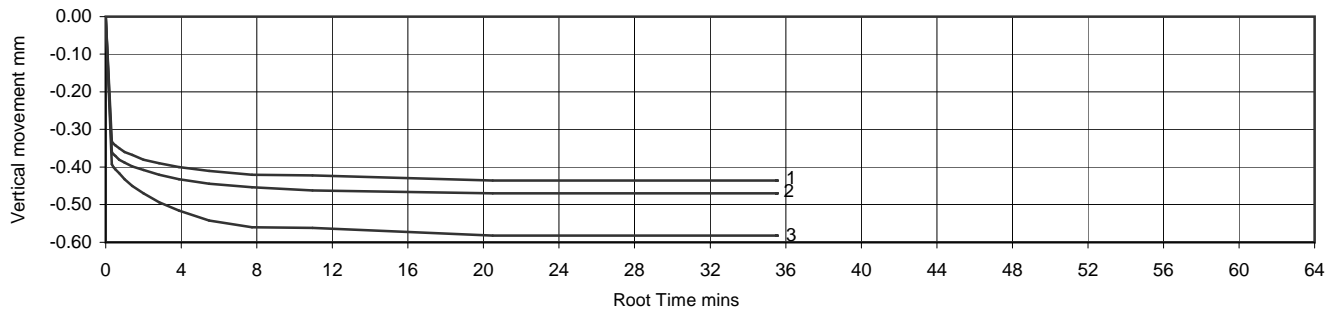
Notes :

1 After shear values based on BS1377. Pt 7 cl. 4.6.1.6
 using δH calculated from consolidation and shear stages

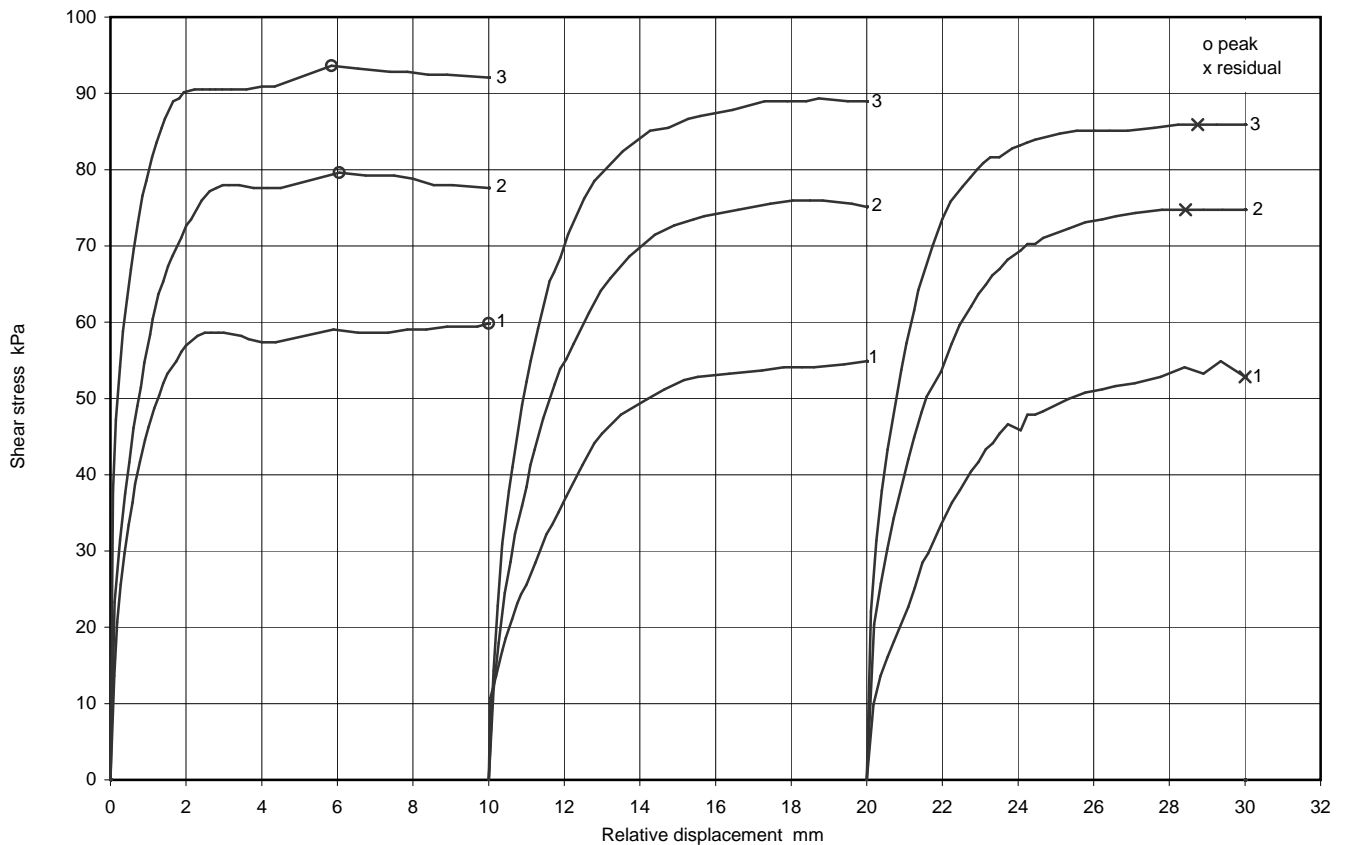
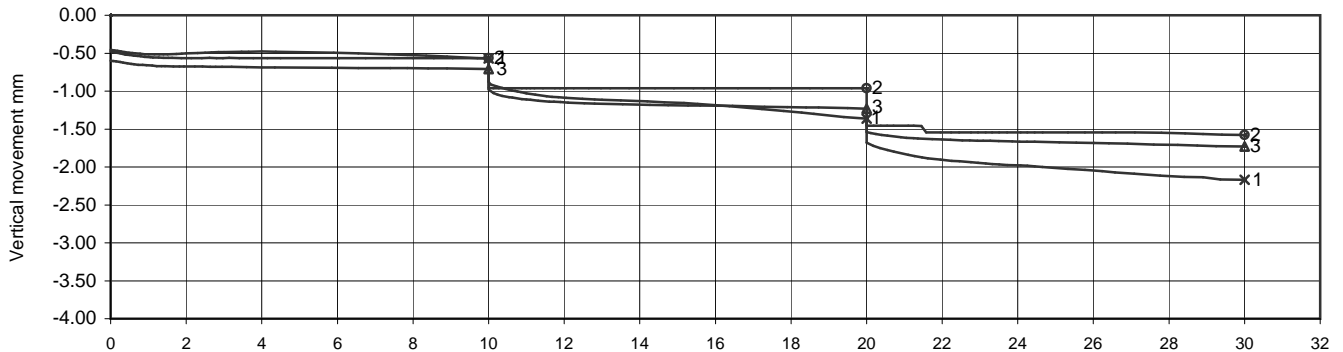
**Determination of shear strength by direct shear (Small shearbox apparatus)
(BS1377 : Part 7 : clause 4 : 1990)**

Project No	A1077-11	Sample Details:	Hole No.		BH2	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		5.00-5.45	
			Sample No	12	Type	U
			ID			
			Spec Ref			

Consolidation stage(s)



Shearing stage(s)



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Figure

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**Determination of shear strength by direct shear (Small shearbox apparatus)
(BS1377 : Part 7 : clause 4 : 1990)**

Project No	A1077-11	Sample Details:	Hole No.		BH2	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		19.00-19.45	
			Sample No	43	Type	U
			ID			
			Spec Ref			

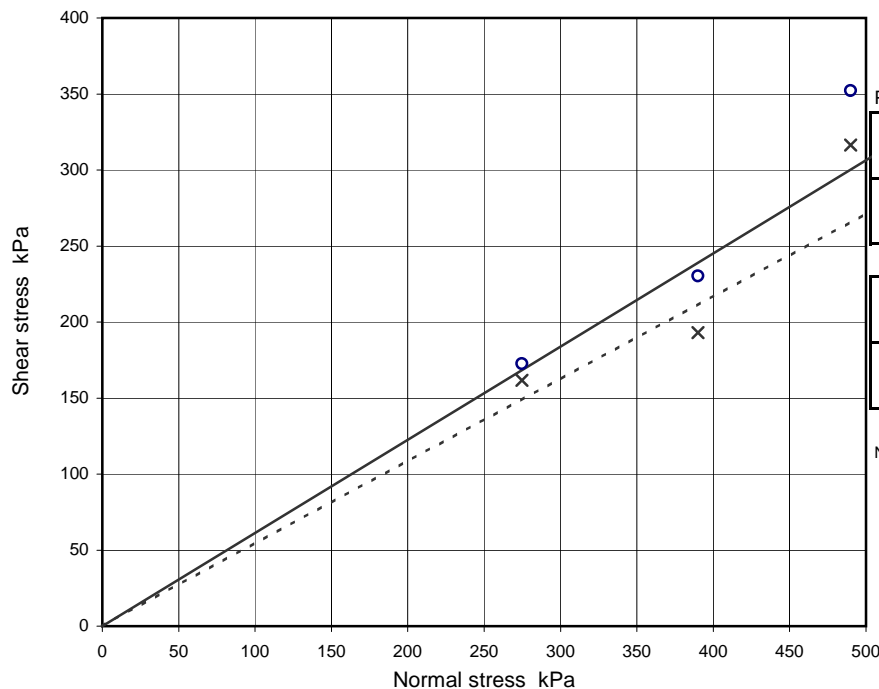
Soil Description	Firm brown slightly sandy slightly gravelly CLAY.
Specimen Type /Preparation	Undisturbed

Specimen(s) nominally 60mm x 60mm square
 Test(s) carried out in submerged condition
 Particle density, assumed 2.65 Mg/m³

Specimen Details		No.	1	2	3	4	5	6
Initial	Height	mm	23.4	23.4	23.4			
	Bulk Density	Mg/m ³	2.18	2.20	2.18			
	Water Content	%	14.5	15.4	16.1			
	Dry density	Mg/m ³	1.91	1.90	1.88			
	Voids ratio		0.389	0.391	0.411			
Consol ⁿ	Degree of Saturation	%	99	104	104			
	Consolidation / Normal Stress applied	kPa	275	390	490			
	Change in height during consolidation	mm	-1.028	-1.748	-1.808			
Shear see note 1	Voids ratio after consolidation		0.328	0.287	0.302			
	Voids ratio at end of test		0.272	0.183	0.175			
	Moisture content at end of test	%	10.3	6.9	6.6			
	Saturation at end of test	%	100	100	100			

Shearing stage

Rate of displacement	Peak	mm/min	0.003	0.003	0.003			
	Residual	mm/min	0.013	0.013	0.013			
Peak values, (o)	Relative displacement	mm	5.93	5.68	6.30			
	Shear stress	kPa	172.7	230.4	352.3			
Residual values, (x)	No. of reversals		2	2	2			
	Relative displacement	mm	30.60	20.00	10.00			
	Shear stress	kPa	161.6	193.1	316.4			



Shear Strength Parameters

Peak strength, (o)		Regression	Manual
c'	kPa	(-66.1)	0.0
Ø'	degrees	(39½)	31½

Residual strength, (x)			
c' _R	kPa	(-48.7)	0.0
Ø' _R	degrees	(35½)	28½

Notes :

- 1 After shear values based on BS1377. Pt 7 cl. 4.6.1.6 using δH calculated from consolidation and shear stages
- 2 2 pieces of medium gravel found in specimen 3 after test.
- 3 Manual interpretation carried out for results for specimens 1 and 2 only, assuming c' and c'_R = 0

Ref

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Figure

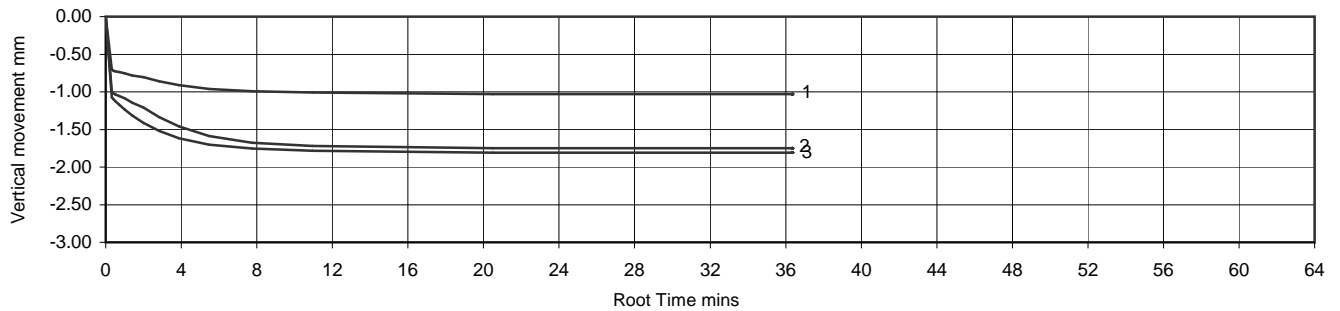
SSB 3

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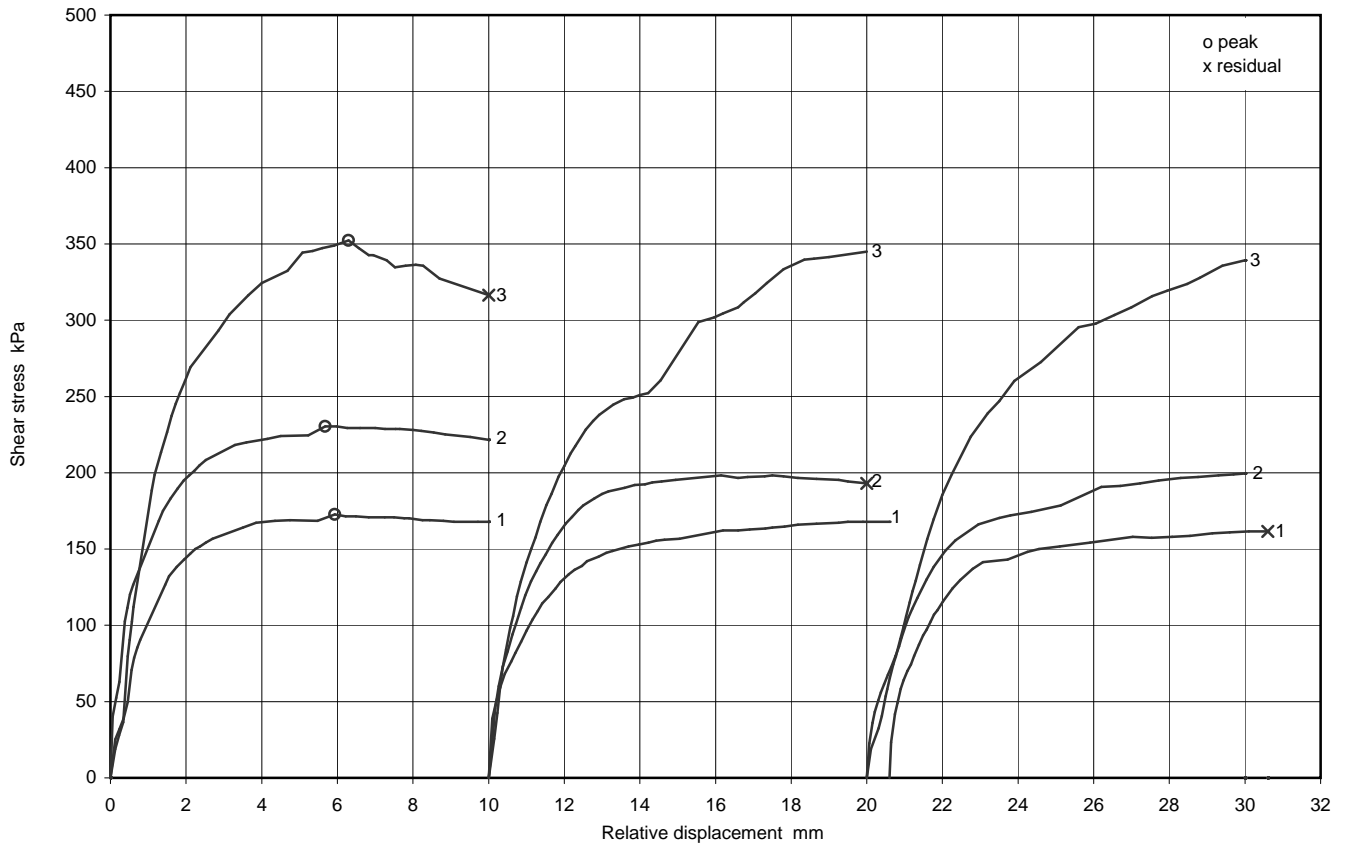
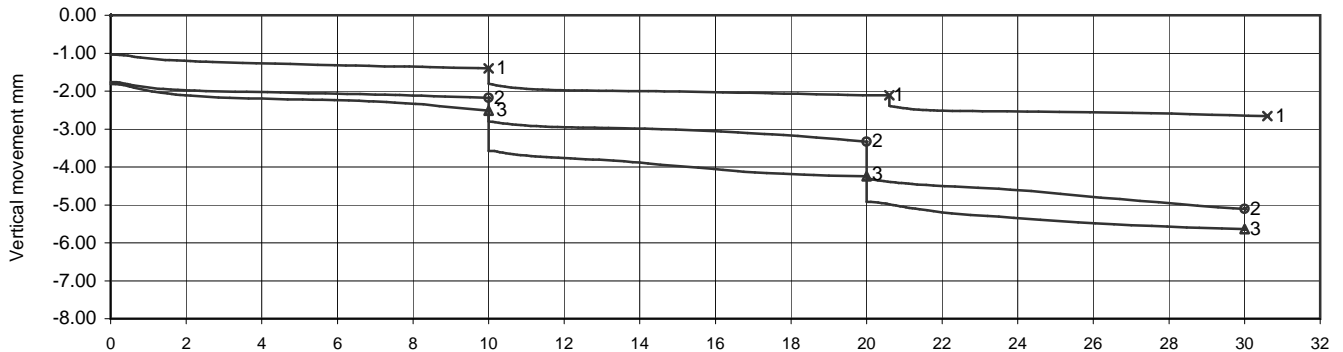
**Determination of shear strength by direct shear (Small shearbox apparatus)
(BS1377 : Part 7 : clause 4 : 1990)**

Project No	A1077-11	Sample Details:	Hole No.		BH2	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		19.00-19.45	
			Sample No	43	Type	U
			ID			
			Spec Ref			

Consolidation stage(s)



Shearing stage(s)



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Figure

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**Determination of shear strength by direct shear (Small shearbox apparatus)
(BS1377 : Part 7 : clause 4 : 1990)**

Project No	A1077-11	Sample Details:	Hole No.		BH2	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		23.00-23.45	
			Sample No	51	Type	U
			ID			
			Spec Ref			

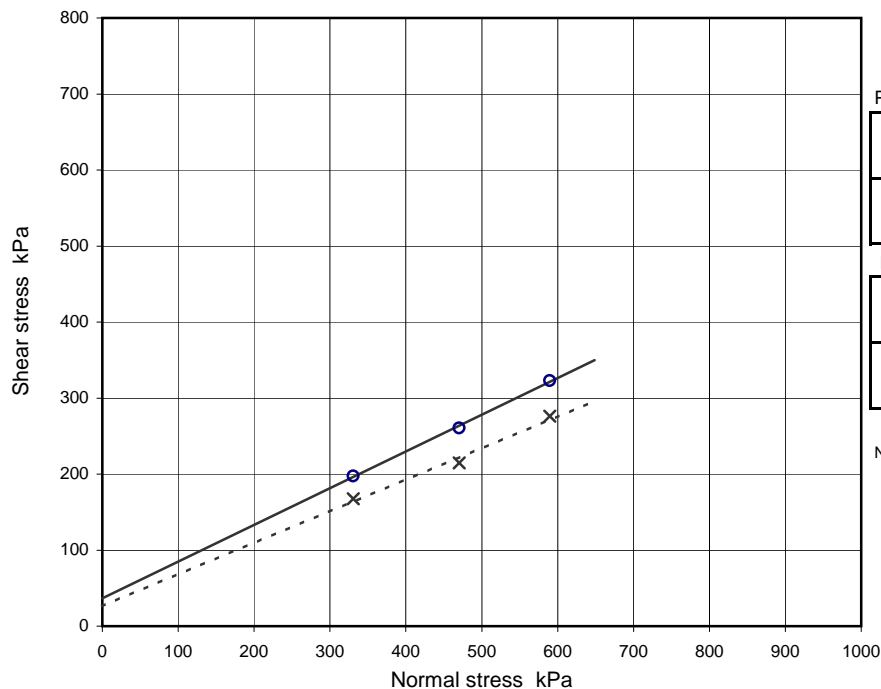
Soil Description	Firm to stiff brown slightly sandy slightly gravelly CLAY.
Specimen Type /Preparation	Undisturbed

Specimen(s) nominally 60mm x 60mm square
 Test(s) carried out in submerged condition
 Particle density, assumed 2.65 Mg/m³

Specimen Details		No.	1	2	3	4	5	6
Initial	Height	mm	23.4	23.4	23.4			
	Bulk Density	Mg/m ³	2.19	2.20	2.21			
	Water Content	%	13.6	13.6	13.6			
	Dry density	Mg/m ³	1.93	1.94	1.95			
	Voids ratio		0.372	0.365	0.360			
Consol ⁿ	Degree of Saturation	%	96	98	100			
	Consolidation / Normal Stress applied	kPa	330	470	590			
	Change in height during consolidation	mm	-0.906	-0.972	-1.280			
Shear see note 1	Voids ratio after consolidation		0.319	0.308	0.286			
	Voids ratio at end of test		0.282	0.211	0.126			
	Moisture content at end of test	%	10.6	8.0	4.7			
	Saturation at end of test	%	100	100	100			

Shearing stage

Rate of displacement	Peak	mm/min	0.003	0.003	0.003			
	Residual	mm/min	0.016	0.016	0.016			
Peak values, (o)	Relative displacement	mm	4.35	4.15	3.94			
	Shear stress	kPa	197.5	260.5	322.9			
Residual values, (x)	No. of reversals		2	2	3			
	Relative displacement	mm	28.95	30.00	40.00			
	Shear stress	kPa	167.5	214.5	276.1			



Shear Strength Parameters

Peak strength, (o)		Regression	Manual
c'	kPa	36	-
Ø'	degrees	26	-

Residual strength, (x)		Regression	Manual
c' _R	kPa	26	-
Ø' _R	degrees	22½	-

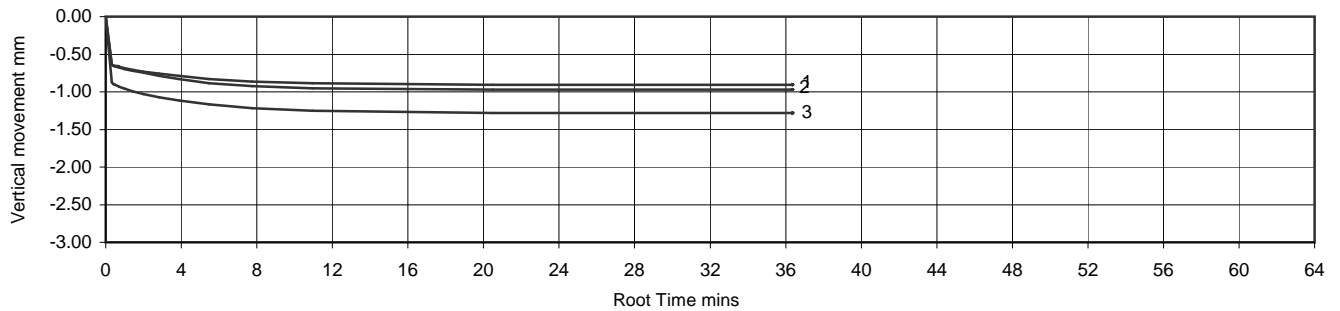
Notes :

1 After shear values based on BS1377. Pt 7 cl. 4.6.1.6
 using δH calculated from consolidation and shear stages

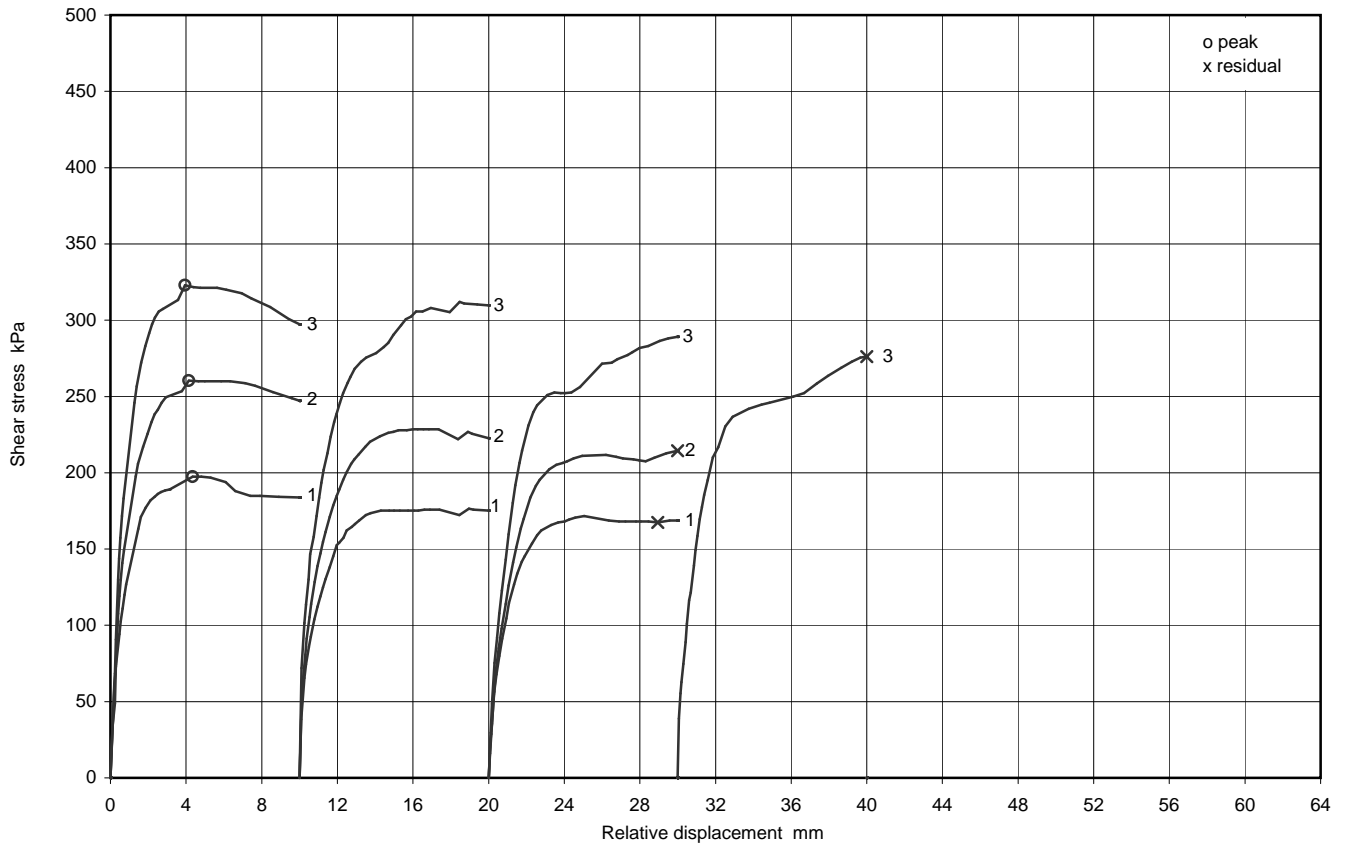
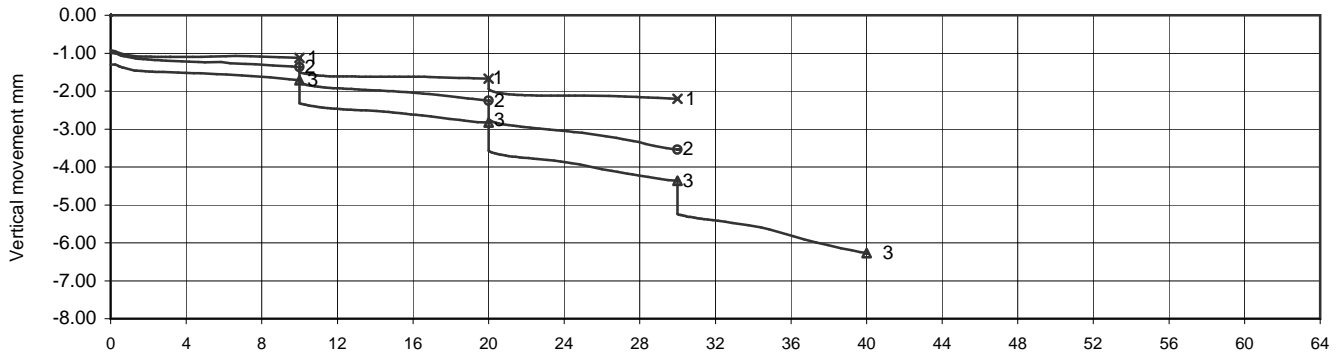
**Determination of shear strength by direct shear (Small shearbox apparatus)
(BS1377 : Part 7 : clause 4 : 1990)**

Project No	A1077-11	Sample Details:	Hole No.		BH2	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		23.00-23.45	
			Sample No	51	Type	U
			ID			
			Spec Ref			

Consolidation stage(s)



Shearing stage(s)



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Figure

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**Determination of residual shear strength using the small ring shear apparatus
(BS1377 : Part 7 : clause 6 : 1990)**

Project No	A1077-11	Sample Details:	Hole No.		BH2	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		15.00-15.45	
			Sample No	31	Type	U
			ID			
			Spec Ref			

Soil Description	Firm to stiff brown slightly sandy slightly gravelly CLAY.	
Specimen Type /Preparation	-1.18mm material. Recompactd at as received moisture content.	

Specimen Details

Initial height	mm	5.0
Initial Bulk Density	Mg/m ³	2.38
Initial Moisture Content (trimmings)	%	13.1
Initial Dry Density	Mg/m ³	2.10
Moisture content at end of test	%	16.3

Nominal dimensions

Outer diameter 100 mm
Inner diameter 70 mm

Test carried out in submerged condition

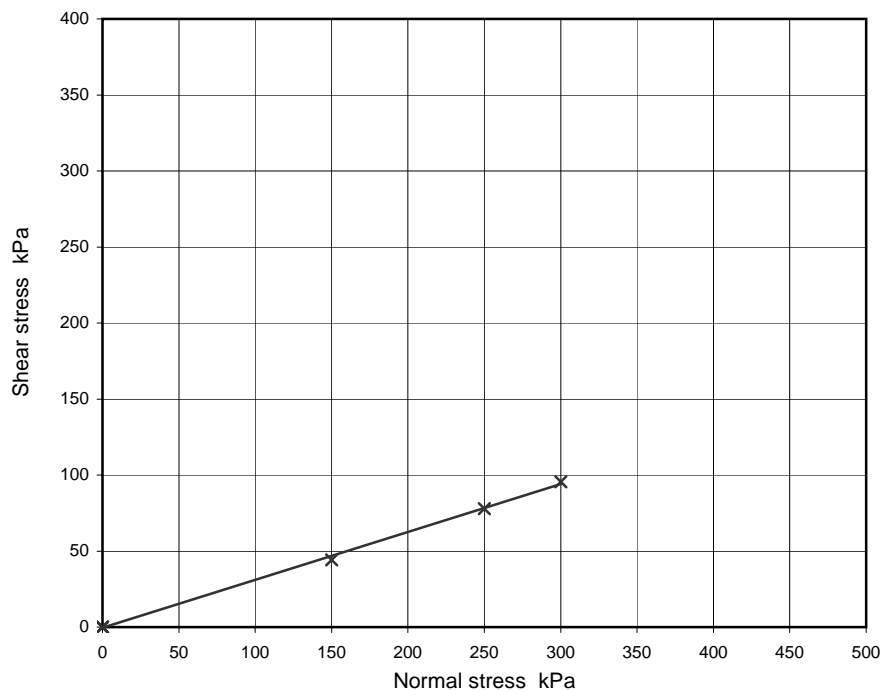
STAGE No.	1	2	3			
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Consolidation stage(s)

Consolidation / Normal Stress applied	kPa	150	250	300			
Change in height during stage	mm	-0.178	-0.138	-0.060			

Shearing stage(s)

Torque arm diameter	mm	150	150	150			
Rate of displacement	Angular	°/min	0.120	0.120	0.120		
	Average linear	mm/min	0.089	0.089	0.089		
At residual state	Normal stress	kPa	150	250	300		
	Mean linear displacement	mm	14.5	32.5	65.9		
	Mean Shear stress	kPa	44.2	77.9	95.6		



Residual Shear Strength Parameters

c'_R is assumed to be zero, BS 1377

ϕ'_R **17.5** degrees

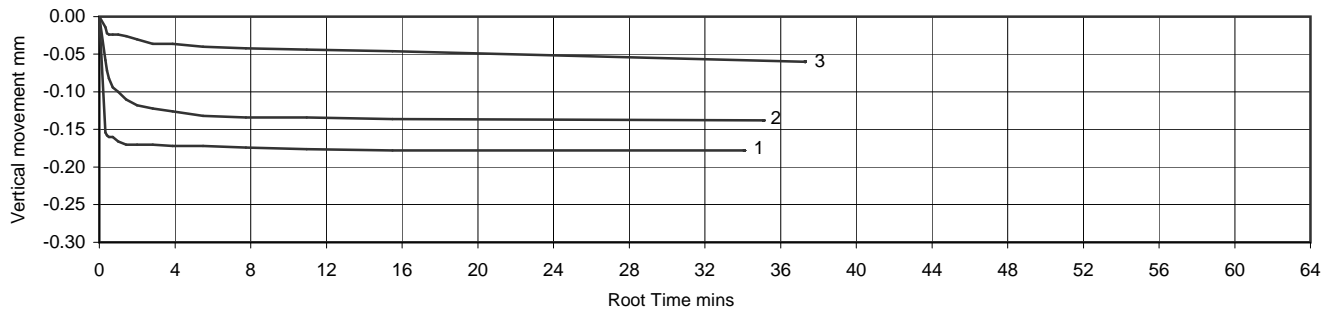
Notes :

1 Bulk and dry density values are based on nominal dimensions and are therefore indicative only.

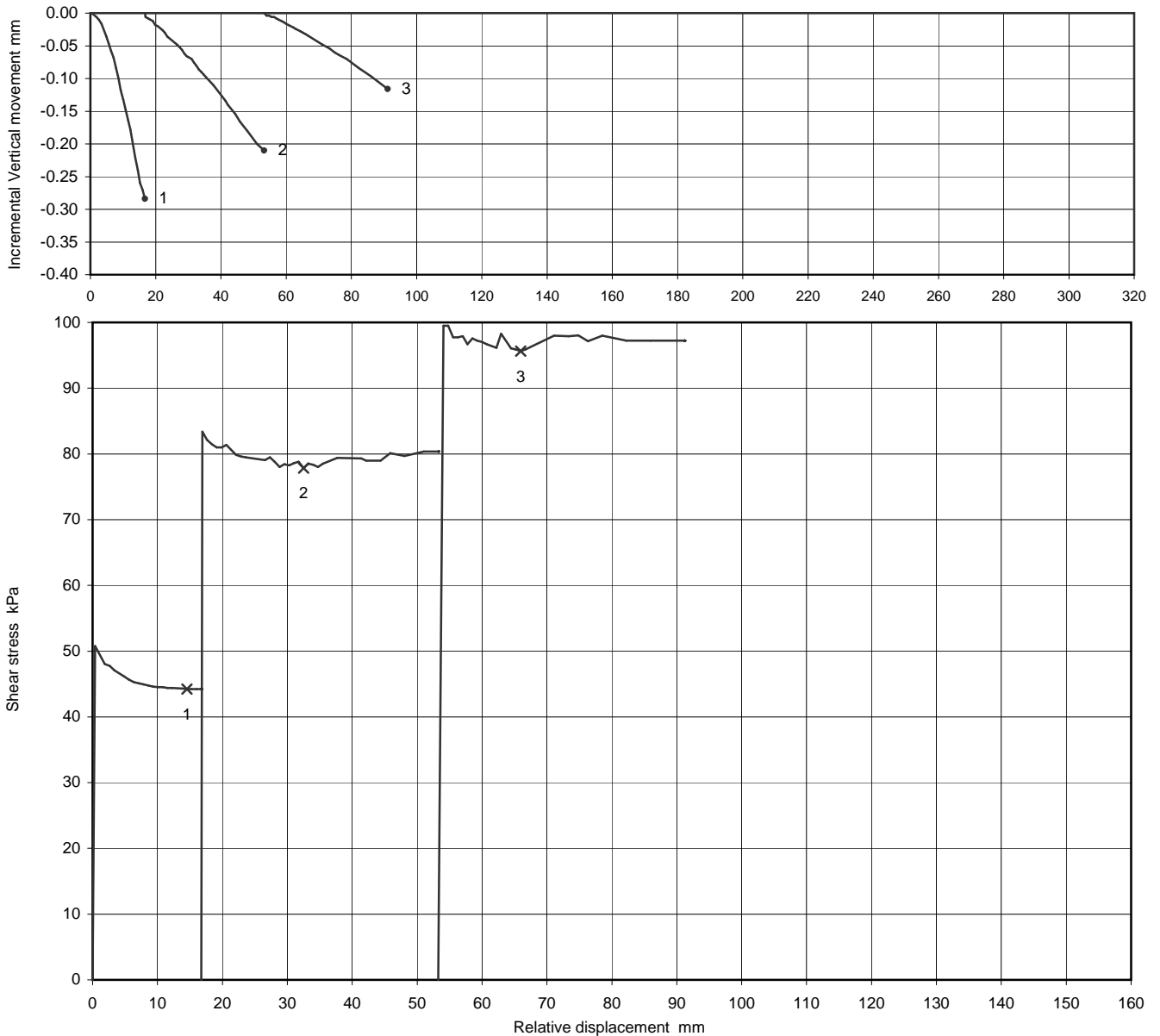
**Determination of residual shear strength using the small ring shear apparatus
(BS1377 : Part 7 : clause 6 : 1990)**

Project No	A1077-11	Sample Details:	Hole No.		BH2	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		15.00-15.45	
			Sample No	31	Type	U
			ID			
			Spec Ref			

Consolidation stage(s)



Shearing stage(s)



Ref

SLR7.6
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Figure

RS 1

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**Determination of residual shear strength using the small ring shear apparatus
(BS1377 : Part 7 : clause 6 : 1990)**

Project No	A1077-11	Sample Details:	Hole No.		BH3	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		7.00-7.45	
			Sample No	20	Type	U
			ID			
			Spec Ref			

Soil Description	Firm brown slightly sandy slightly gravelly CLAY.				
Specimen Type /Preparation	-1.18mm material. Recompactd at as received moisture content.				

Specimen Details

Initial height	mm	5.0
Initial Bulk Density	Mg/m ³	2.33
Initial Moisture Content (trimmings)	%	16.2
Initial Dry Density	Mg/m ³	2.01
Moisture content at end of test	%	15.4

Nominal dimensions

Outer diameter 100 mm

Inner diameter 70 mm

Test carried out in submerged condition

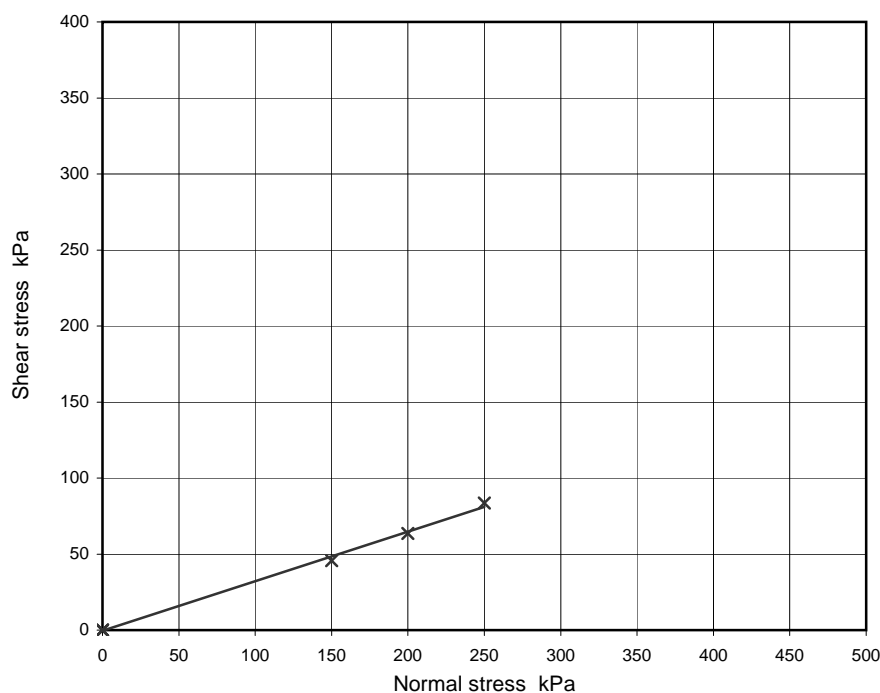
STAGE No.	1	2	3			
------------------	----------	----------	----------	--	--	--

Consolidation stage(s)

Consolidation / Normal Stress applied	kPa	150	200	250			
Change in height during stage	mm	-0.204	-0.050	-0.036			

Shearing stage(s)

Torque arm diameter	mm	150	150	150			
Rate of displacement	Angular	°/min	0.120	0.120	0.120		
	Average linear	mm/min	0.089	0.089	0.089		
At residual state	Normal stress	kPa	150	200	250		
	Mean linear displacement	mm	8.0	39.6	89.0		
	Mean Shear stress	kPa	45.8	63.6	83.5		



Residual Shear Strength Parameters

c'_R is assumed to be zero, BS 1377

ϕ'_R **18.0** degrees

Notes :

1 Bulk and dry density values are based on nominal dimensions and are therefore indicative only.

Ref

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Figure

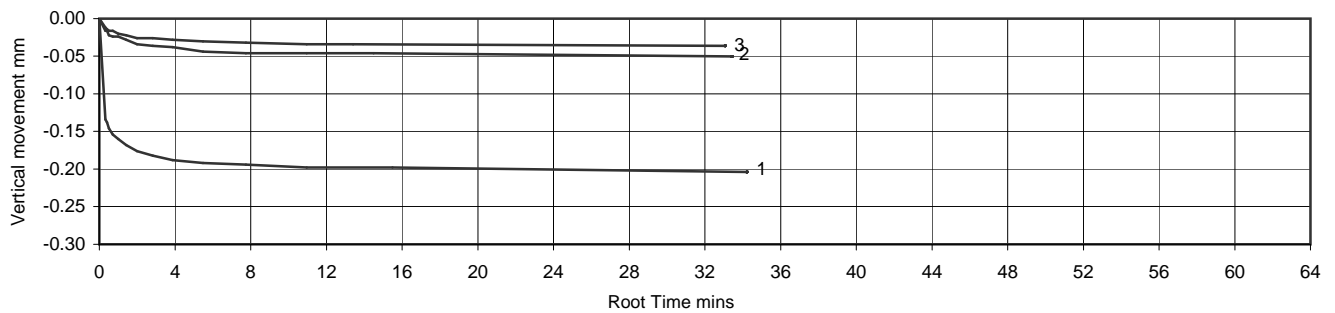
RS 2

sheet 1 of 2

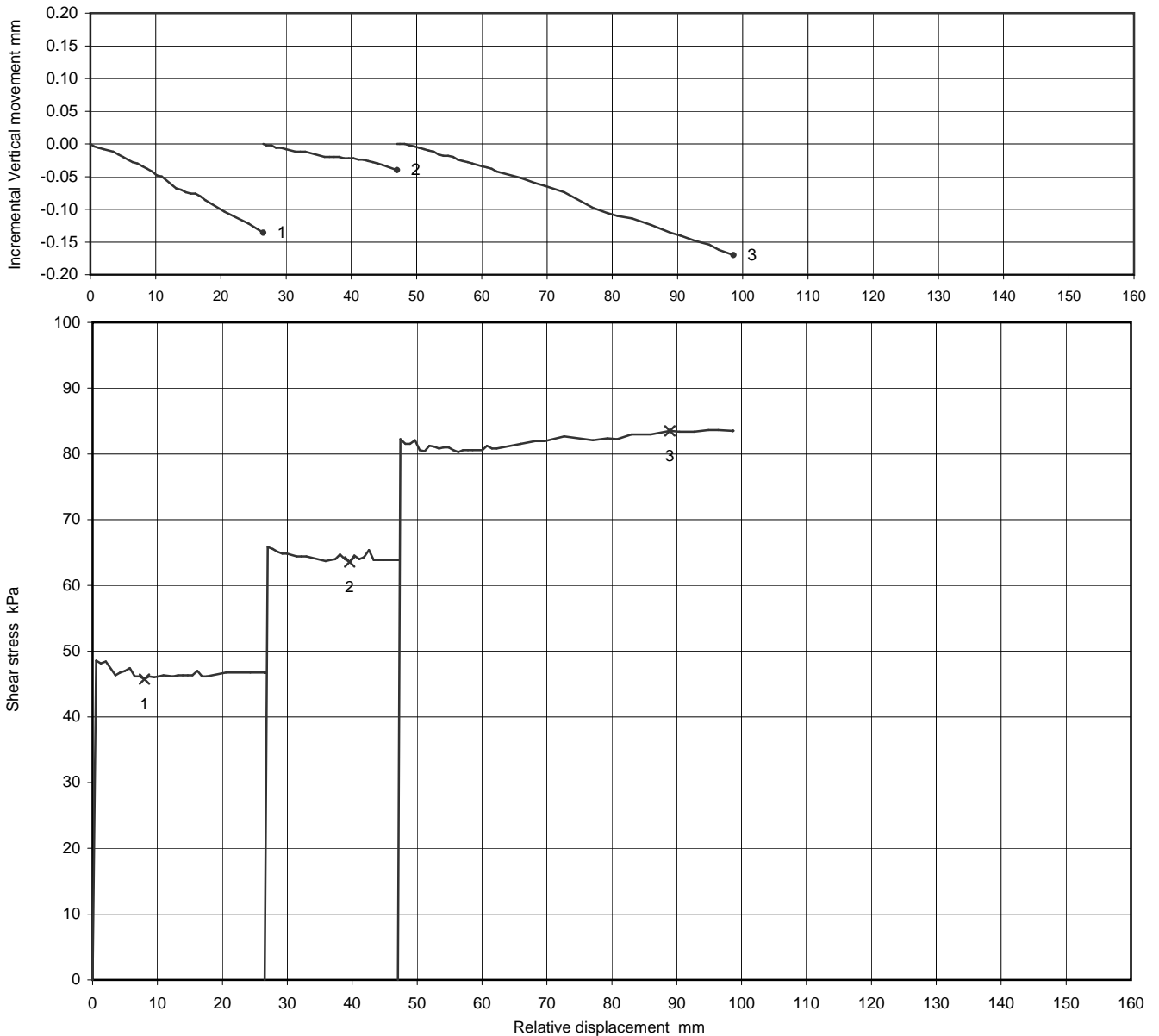
**Determination of residual shear strength using the small ring shear apparatus
(BS1377 : Part 7 : clause 6 : 1990)**

Project No	A1077-11	Sample Details:	Hole No.		BH3	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		7.00-7.45	
			Sample No	20	Type	U
			ID			
			Spec Ref			

Consolidation stage(s)



Shearing stage(s)



Ref

SLR7.6
Rev 85
Jan 11



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Figure

RS 2

sheet 2 of 2

Shear Strength by Laboratory Vane method - Summary of Results

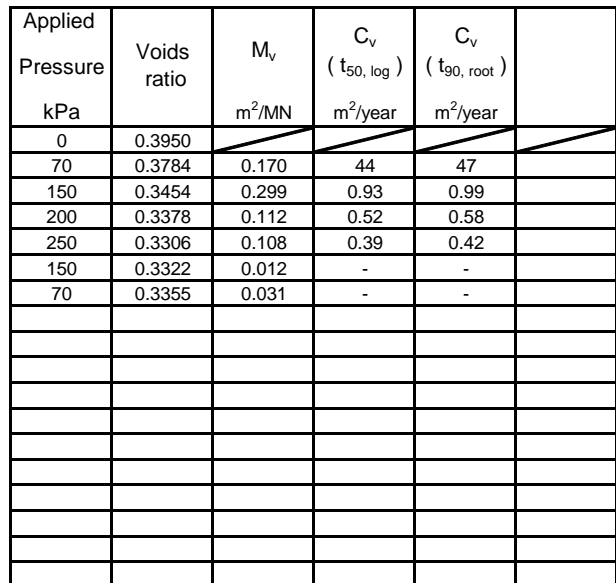
[illegible]

Notes :

- 1 Tests carried out in accordance with BS1377 : part 7 : 1990, clause 3
2 Shear strengths given are average of 3 tests unless otherwise stated
3 Tests carried out in 100mm diameter plastic sample tube using 12.7mm x 12.7mm vane

Ref SLR Lvane Rev 1 Aug 07		Sheet printed : 18/11/2011 17:41	Table LVANE 1
--	---	-------------------------------------	------------------------------------

Project No	A1077-11	Sample Details:	Hole No		BH1		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		3.00		
			Samp No	9	Type	U	
			ID	ESGA1077-11201110100000000010			
			Spec Ref				

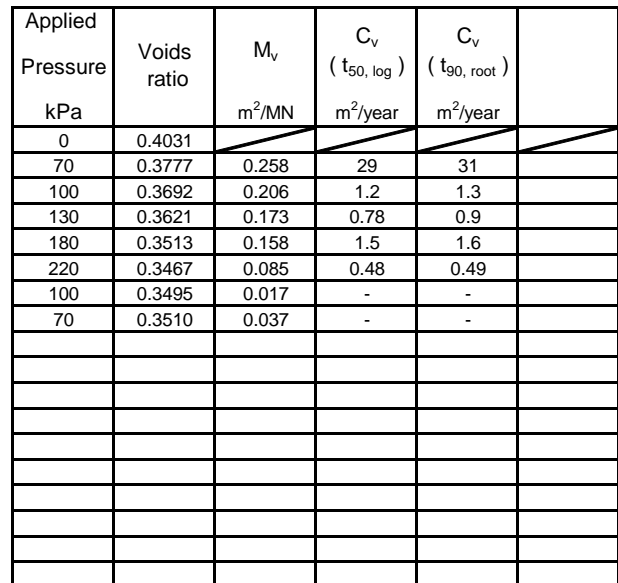


OED 1

ONE DIMENSIONAL CONSOLIDATION TEST							
BS 1377 : Part 5 : 1990 : clause 3							
Project No	A1077-11	Sample Details:	Hole No	BH1			
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	6.50			
			Samp No	17	Type	U	
			ID	ESGA1077-11201110100000000018			
			Spec Ref				
<div><div>Voids ratio</div><div>Applied pressure kPa</div><div>Log pressure / voids ratio</div></div> <div><div>Cv m²/year (log t)</div><div>Applied pressure kPa</div><div>Log pressure / Cv</div></div>							
Soil description		Firm brown slightly sandy slightly gravelly CLAY.					
Preparation		Recompacted					
Index properties (if available)		Liquid limit % 33	Plastic limit % 15				
Specimen details		Initial 2.65	Final assumed	Mg/m³			
Particle density		74.59	mm				
Diameter		18.76	17.99	mm			
Height		0.402	0.344				
Voids ratio		15	14	%			
Moisture content		2.18	2.24	Mg/m³			
Bulk density		1.89	1.97	Mg/m³			
Dry density		101	105	%			
Saturation		21		°C			
Average temperature for test							
Swelling pressure		not measured kPa					
Notes :		Recompacted using 2.5kg effort at as received moisture content.					
Specimen taken		20 mm from base of sample					
QA Ref		SLR 5.3 Rev 122 Aug 11				Figure OED 2	
Environmental Scientifics Group		UKAS TESTING 1157				Printed:18/11/2011 17:47	

[illegible]

Project No	A1077-11	Sample Details:	Hole No		BH2		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		3.00		
			Samp No	8	Type	U	
			ID	ESGA1077-11201110100000000074			
			Spec Ref				



OED 4

ONE DIMENSIONAL CONSOLIDATION TEST							
BS 1377 : Part 5 : 1990 : clause 3							
Project No	A1077-11	Sample Details:	Hole No	BH2			
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	5.00			
			Samp No	12	Type	U	
			ID	ESGA1077-11201110100000000078			
			Spec Ref				
<div><div><div>Voids ratio</div><div>Applied pressure kPa</div><div>Log pressure / voids ratio</div></div><div><div>Cv m²/year (log t)</div><div>Applied pressure kPa</div><div>Log pressure / Cv</div></div></div>							
Soil description		Brown slightly sandy slightly gravelly CLAY.					
Preparation		Recompacted					
Index properties (if available)		Liquid limit %	31	Plastic limit %	15		
Specimen details		Initial	Final				
Particle density		2.65	assumed	Mg/m³			
Diameter		75.00	mm				
Height		18.96	18.46	mm			
Voids ratio		0.375	0.339				
Moisture content		14	13	%			
Bulk density		2.21	2.25	Mg/m³			
Dry density		1.93	1.98	Mg/m³			
Saturation		102	105	%			
Average temperature for test		21		°C			
Swelling pressure		not measured		kPa			
Notes :		Recompacted using 2.5kg effort at as received moisture content.					
Specimen taken		10 mm from base of sample					
QA Ref		SLR 5.3 Rev 122 Aug 11				Figure OED 5	

ONE DIMENSIONAL CONSOLIDATION TEST						
BS 1377 : Part 5 : 1990 : clause 3						
Project No	A1077-11	Sample Details:	Hole No	BH3		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	2.50		
			Samp No	8	Type	U
			ID	ESGA1077-11201110110000000133		
		Spec Ref				

Voids ratio

Applied pressure kPa

Log pressure / voids ratio

Cv m²/year (log t)

Applied pressure kPa

Log pressure / Cv

Soil description	Firm to very stiff brown slightly sandy slightly gravelly CLAY.																																		
Preparation	Recompacted																																		
Index properties (if available)	Liquid limit % <table border="1"><tr><td>37</td><td>Plastic limit %</td><td>18</td></tr></table>	37	Plastic limit %	18																															
37	Plastic limit %	18																																	
Specimen details	<table border="1"><thead><tr><th>Initial</th><th>Final</th></tr></thead><tbody><tr><td>Particle density</td><td>assumed</td><td>Mg/m³</td></tr><tr><td>Diameter</td><td>74.96</td><td>mm</td></tr><tr><td>Height</td><td>19.02</td><td>18.56</td><td>mm</td></tr><tr><td>Voids ratio</td><td>0.432</td><td>0.397</td></tr><tr><td>Moisture content</td><td>16</td><td>16</td><td>%</td></tr><tr><td>Bulk density</td><td>2.15</td><td>2.20</td><td>Mg/m³</td></tr><tr><td>Dry density</td><td>1.85</td><td>1.90</td><td>Mg/m³</td></tr><tr><td>Saturation</td><td>98</td><td>105</td><td>%</td></tr><tr><td>Average temperature for test</td><td colspan="2">21 °C</td></tr></tbody></table>	Initial	Final	Particle density	assumed	Mg/m³	Diameter	74.96	mm	Height	19.02	18.56	mm	Voids ratio	0.432	0.397	Moisture content	16	16	%	Bulk density	2.15	2.20	Mg/m³	Dry density	1.85	1.90	Mg/m³	Saturation	98	105	%	Average temperature for test	21 °C	
Initial	Final																																		
Particle density	assumed	Mg/m³																																	
Diameter	74.96	mm																																	
Height	19.02	18.56	mm																																
Voids ratio	0.432	0.397																																	
Moisture content	16	16	%																																
Bulk density	2.15	2.20	Mg/m³																																
Dry density	1.85	1.90	Mg/m³																																
Saturation	98	105	%																																
Average temperature for test	21 °C																																		
Swelling pressure	not measured kPa																																		
Notes :	Recompacted using 2.5kg effort at as received moisture content.																																		
Specimen taken	10 mm from base of sample																																		

Applied Pressure kPa	Voids ratio	M _v m²/MN	C _v (t _{50, log}) m²/year	C _v (t _{90, root}) m²/year
0	0.4322	-	-	-
60	0.4175	0.171	45	54
90	0.4095	0.189	1.9	1.8
120	0.4016	0.185	1.3	1.3
150	0.3943	0.174	0.79	0.88
180	0.3885	0.139	0.61	0.66
90	0.3917	0.025	-	-
60	0.3973	0.135	-	-

QA Ref

SLR 5.3 Rev 122 Aug 11

ESG Environmental Scientifics Group

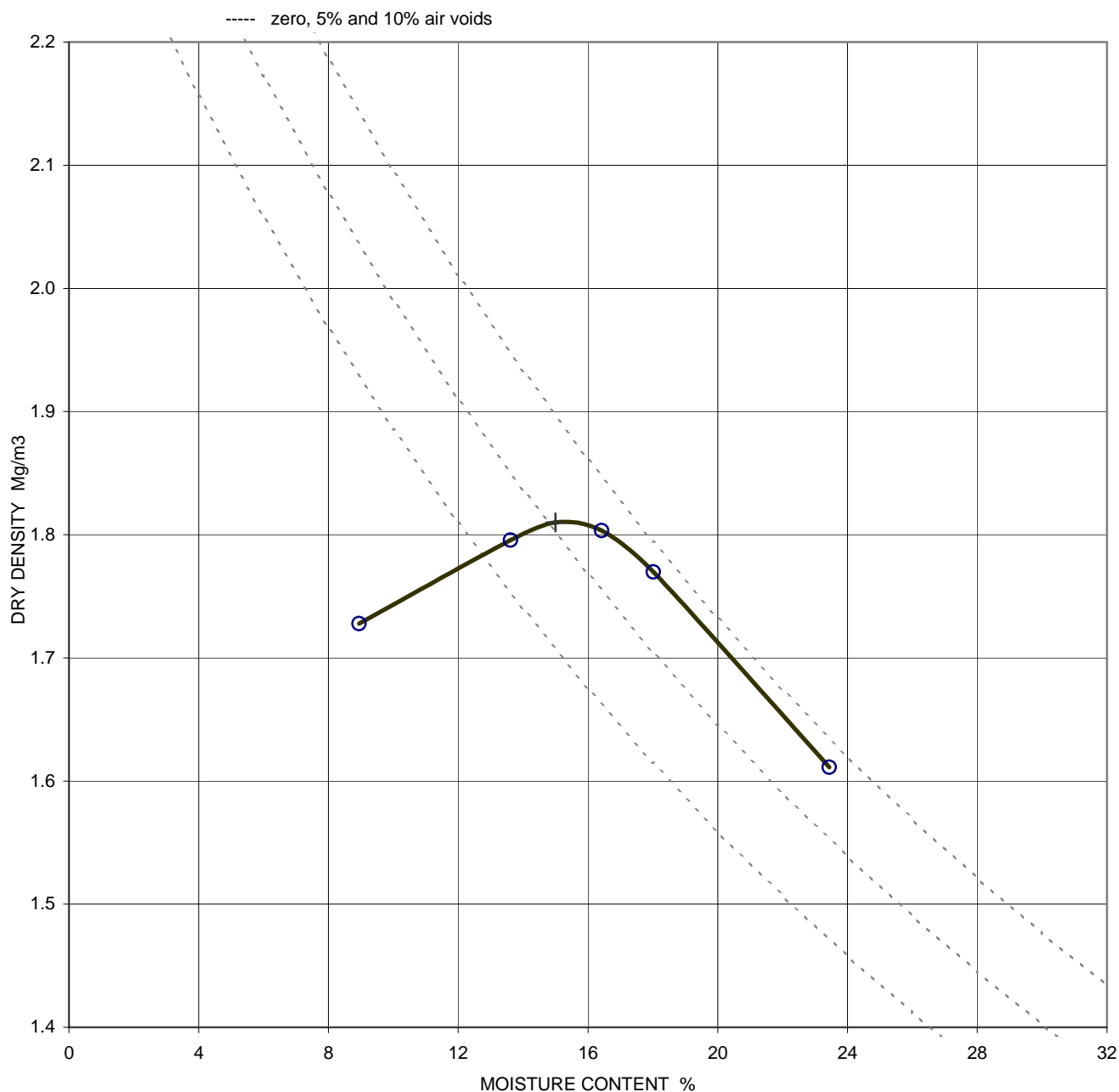
UKAS TESTING 1157

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Figure OED 6

DRY DENSITY / MOISTURE CONTENT RELATIONSHIP
BS1377 : PART 4 : 1990 : HEAVY COMPACTION, 4.5 kg rammer

Project No	A1077-11	Sample Details:	Hole No		BH1		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		0.50		
			Samp No	4	Type	B	
			ID	ESGA1077-11201110100000000004			
			Spec Ref				



Soil description Dark brown slightly gravelly sandy CLAY with occasional rootlets.

Test method BS 1377:part 4:1990: clause 3.5, 4.5 kg rammer in a 1 litre mould

Preparation Original material was natural, separate specimens tested

Material > 37.5mm 0 %

Material < 37.5mm > 20mm 0 %

Particle density 2.65 assumed

Remarks

Derived Parameters +

Maximum dry density, Mg/m³
1.81

Optimum moisture content, %
15

QA Ref
SLD 4, 3.5/6
Rev 66
Aug 11

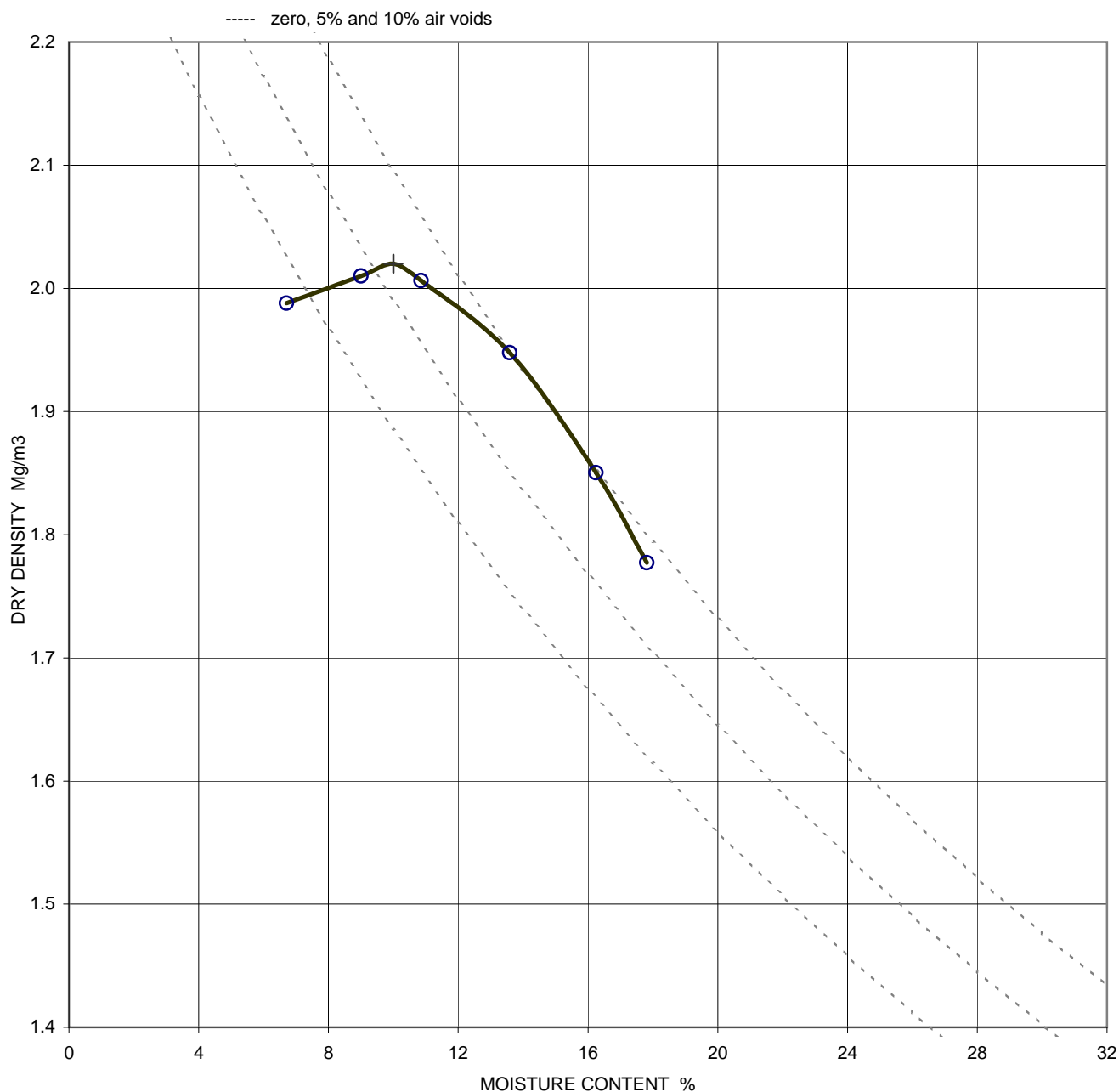


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Figure
COMPH 1

DRY DENSITY / MOISTURE CONTENT RELATIONSHIP
BS1377 : PART 4 : 1990 : HEAVY COMPACTION, 4.5 kg rammer

Project No	A1077-11	Sample Details:	Hole No	BH1
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	2.00
			Samp No	8
			Type	B
			ID	ESGA1077-11201110100000000009
			Spec Ref	



Soil description Brown slightly sandy slightly gravelly CLAY.

Test method BS 1377:part 4:1990: clause 3.5, 4.5 kg rammer in a 1 litre mould

Preparation Original material was natural, composite specimens tested

Material > 37.5mm 2 %

Material < 37.5mm > 20mm 2 %

Particle density 2.65 assumed

Remarks

Derived Parameters +

Maximum dry density, Mg/m3
2.02

Optimum moisture content, %
10

QA Ref
SLD 4, 3.5/6
Rev 66
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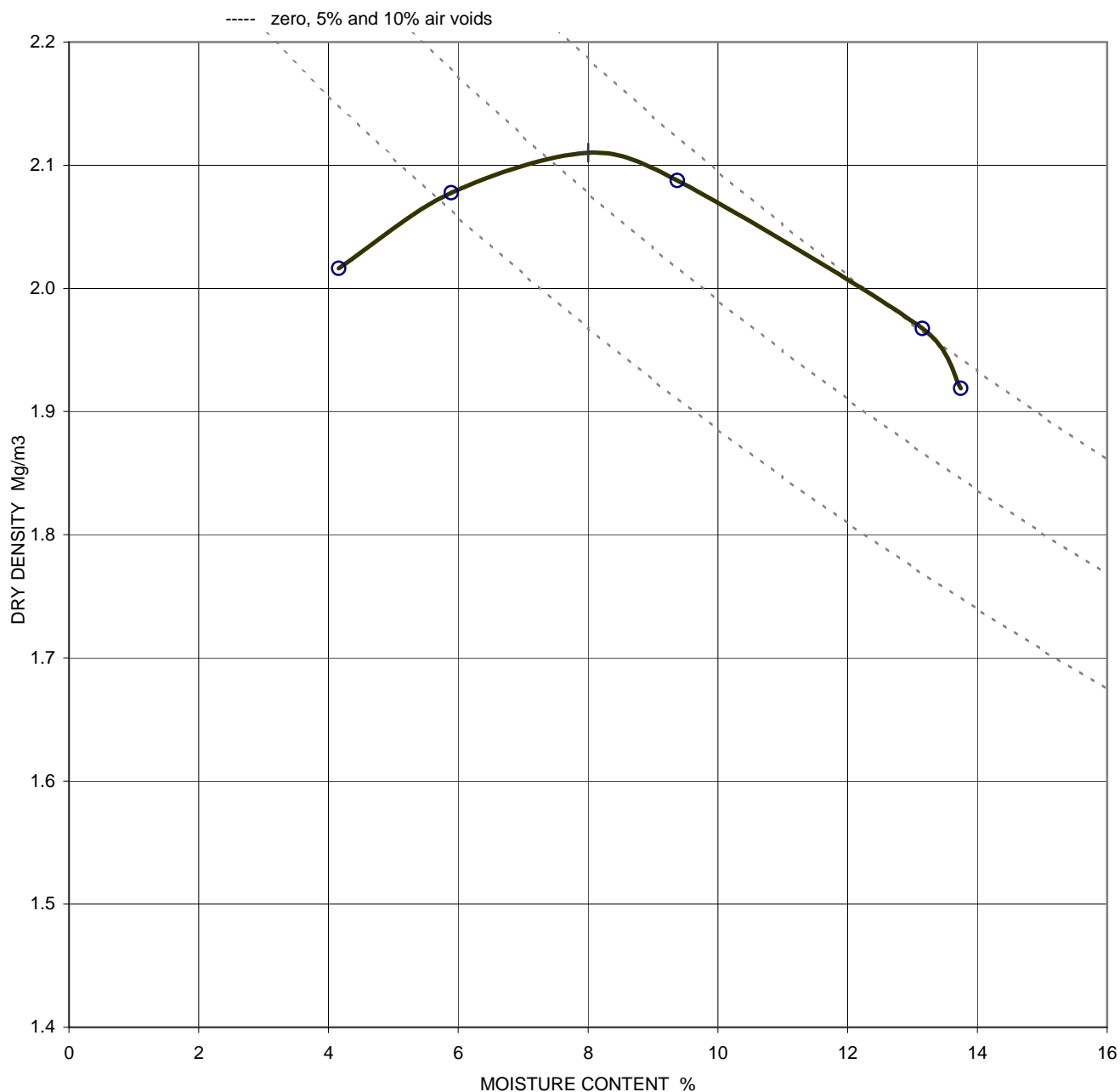


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Figure
COMPH 2

DRY DENSITY / MOISTURE CONTENT RELATIONSHIP
BS1377 : PART 4 : 1990 : HEAVY COMPACTION, 4.5 kg rammer

Project No	A1077-11	Sample Details:	Hole No		BH1	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		4.00	
			Samp No	12	Type	B
			ID	ESGA1077-11201110100000000013		
			Spec Ref			



Soil description Brown slightly sandy slightly gravelly CLAY.

Test method BS 1377:part 4:1990: clause 3.6, 4.5 kg rammer in a CBR mould

Preparation Original material was natural, composite specimens tested

Material > 37.5mm 0 %

Material < 37.5mm > 20mm 3 %

Particle density 2.65 assumed

Remarks

Derived Parameters +

Maximum dry density, Mg/m3
2.11

Optimum moisture content, %
8.0

QA Ref
SLD 4, 3.5/6
Rev 66
Aug 11

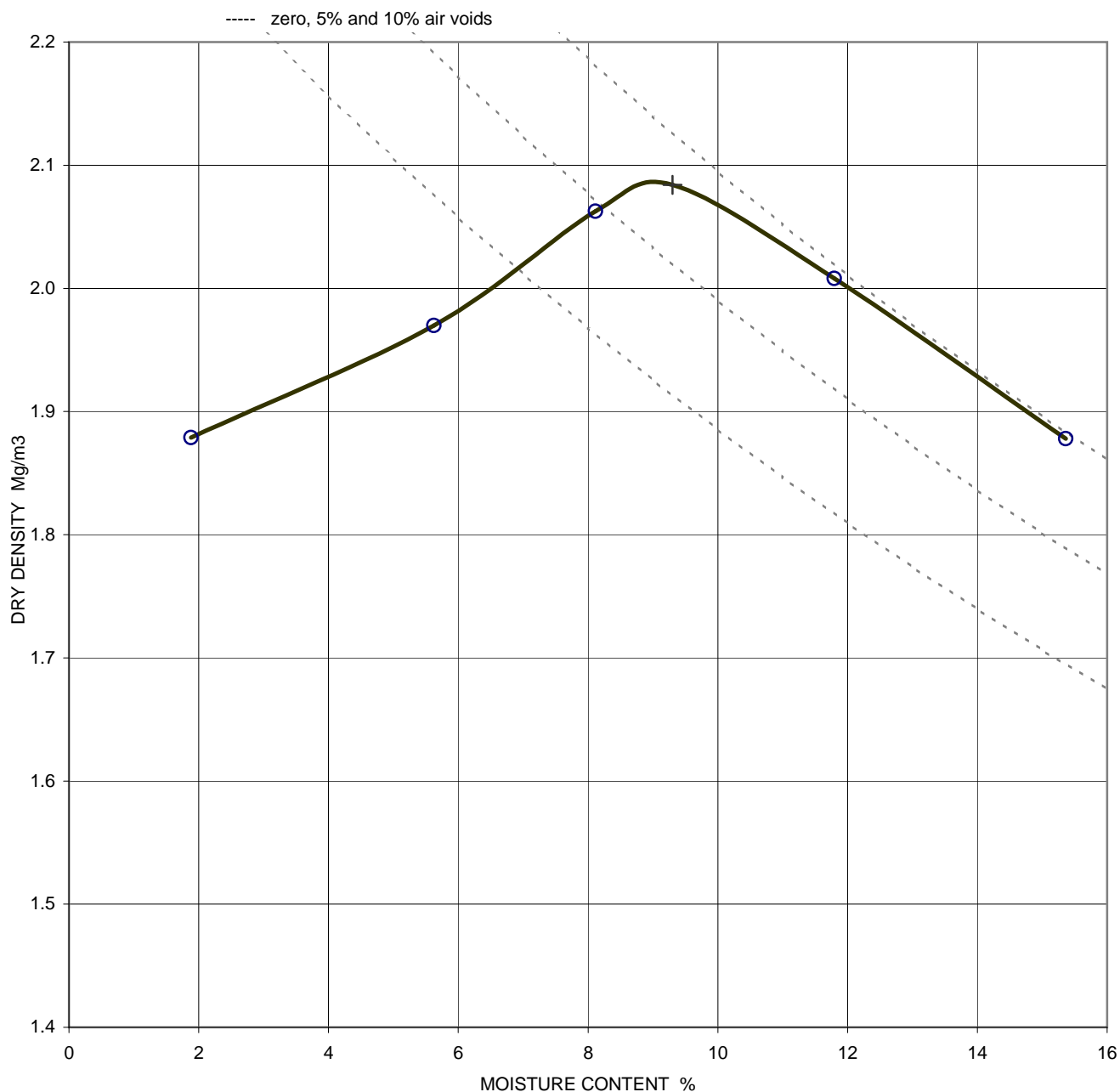


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Figure
COMPH 3

DRY DENSITY / MOISTURE CONTENT RELATIONSHIP
BS1377 : PART 4 : 1990 : HEAVY COMPACTION, 4.5 kg rammer

Project No	A1077-11	Sample Details:	Hole No		BH1	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		5.65	
			Samp No	16	Type	B
			ID	ESGA1077-11201110100000000017		
			Spec Ref			



Soil description Brown slightly sandy slightly gravelly CLAY.

Test method BS 1377:part 4:1990: clause 3.5, 4.5 kg rammer in a 1 litre mould

Preparation Original material was natural, separate specimens tested

Material > 37.5mm 0 %

Material < 37.5mm > 20mm 4 %

Particle density 2.65 assumed

Remarks

Derived Parameters +

Maximum dry density, Mg/m³
2.08

Optimum moisture content, %
9.3

QA Ref
SLD 4, 3.5/6
Rev 66
Aug 11

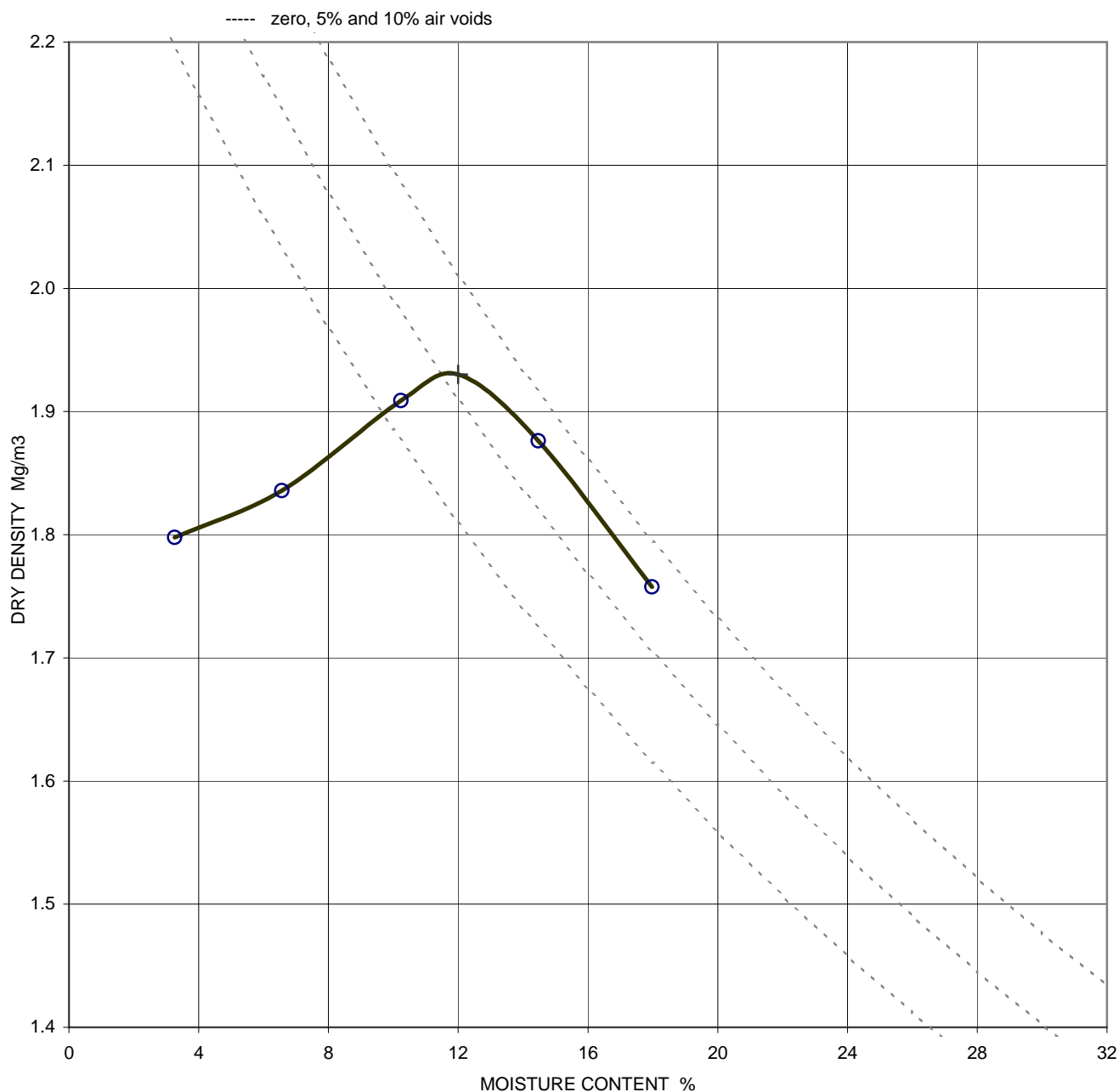


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Figure
COMPH 4

DRY DENSITY / MOISTURE CONTENT RELATIONSHIP
BS1377 : PART 4 : 1990 : HEAVY COMPACTION, 4.5 kg rammer

Project No	A1077-11	Sample Details:	Hole No		BH2		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		0.50		
			Samp No	3	Type	B	
			ID	ESGA1077-11201110100000000069			
			Spec Ref				



Soil description Brown slightly gravelly sandy silty CLAY.

Test method BS 1377:part 4:1990: clause 3.5, 4.5 kg rammer in a 1 litre mould

Preparation Original material was natural, separate specimens tested

Material > 37.5mm 0 %

Material < 37.5mm > 20mm 0 %

Particle density 2.65 assumed

Remarks

Derived Parameters +

Maximum dry density, Mg/m3
1.93

Optimum moisture content, %
12

QA Ref
 SLD 4, 3.5/6
 Rev 66
 Aug 11

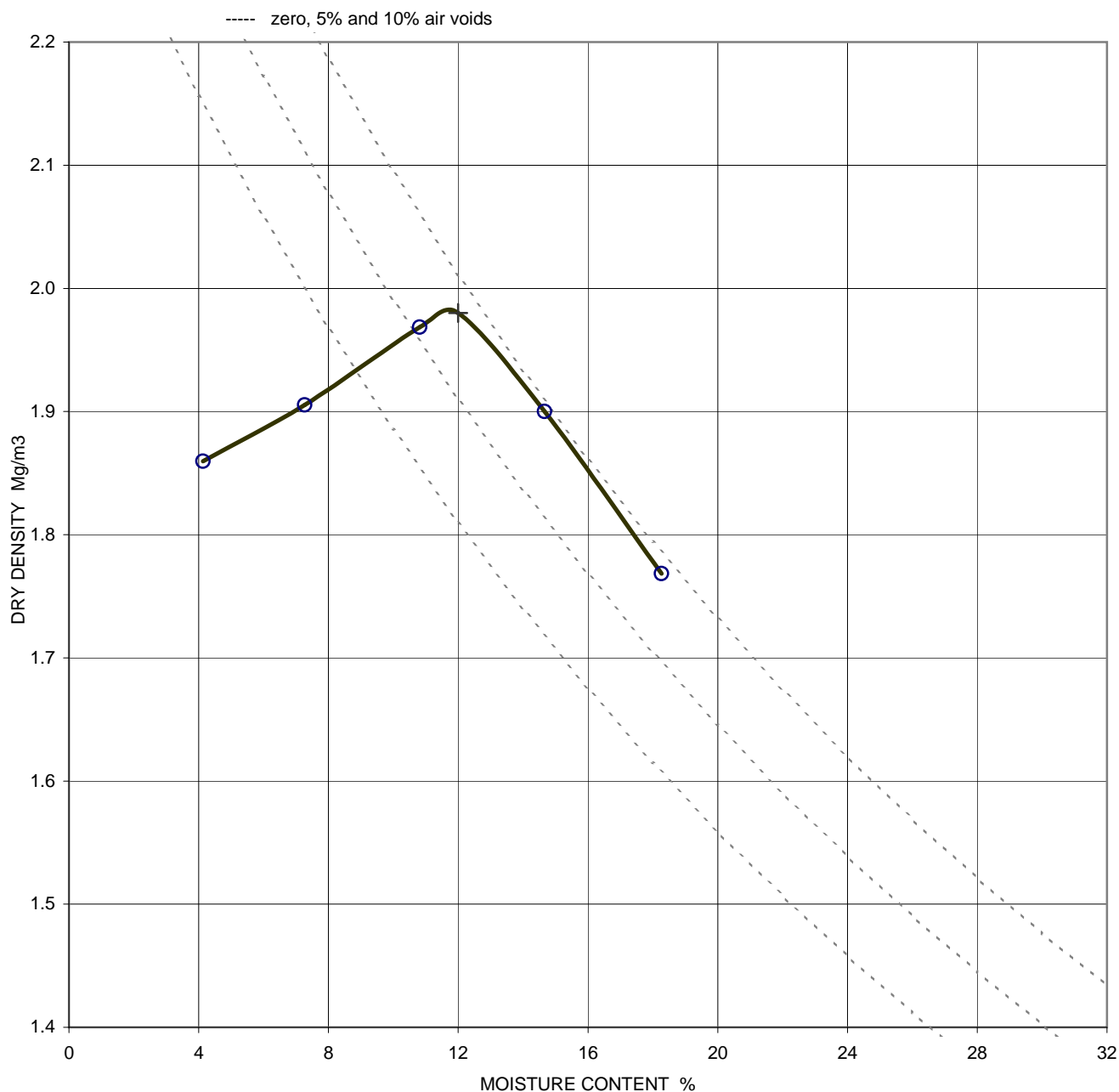


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Figure
COMPH 5

DRY DENSITY / MOISTURE CONTENT RELATIONSHIP
BS1377 : PART 4 : 1990 : HEAVY COMPACTION, 4.5 kg rammer

Project No	A1077-11	Sample Details:	Hole No		BH2	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		2.00	
			Samp No	7	Type	B
			ID	ESGA1077-11201110100000000073		
			Spec Ref			



Soil description Brown slightly sandy slightly gravelly CLAY.

Test method BS 1377:part 4:1990: clause 3.5, 4.5 kg rammer in a 1 litre mould

Preparation Original material was natural, separate specimens tested

Material > 37.5mm 0 %

Material < 37.5mm > 20mm 2 %

Particle density 2.65 assumed

Remarks

Derived Parameters +

Maximum dry density, Mg/m3
1.98

Optimum moisture content, %
12

QA Ref
 SLD 4, 3.5/6
 Rev 66
 Aug 11

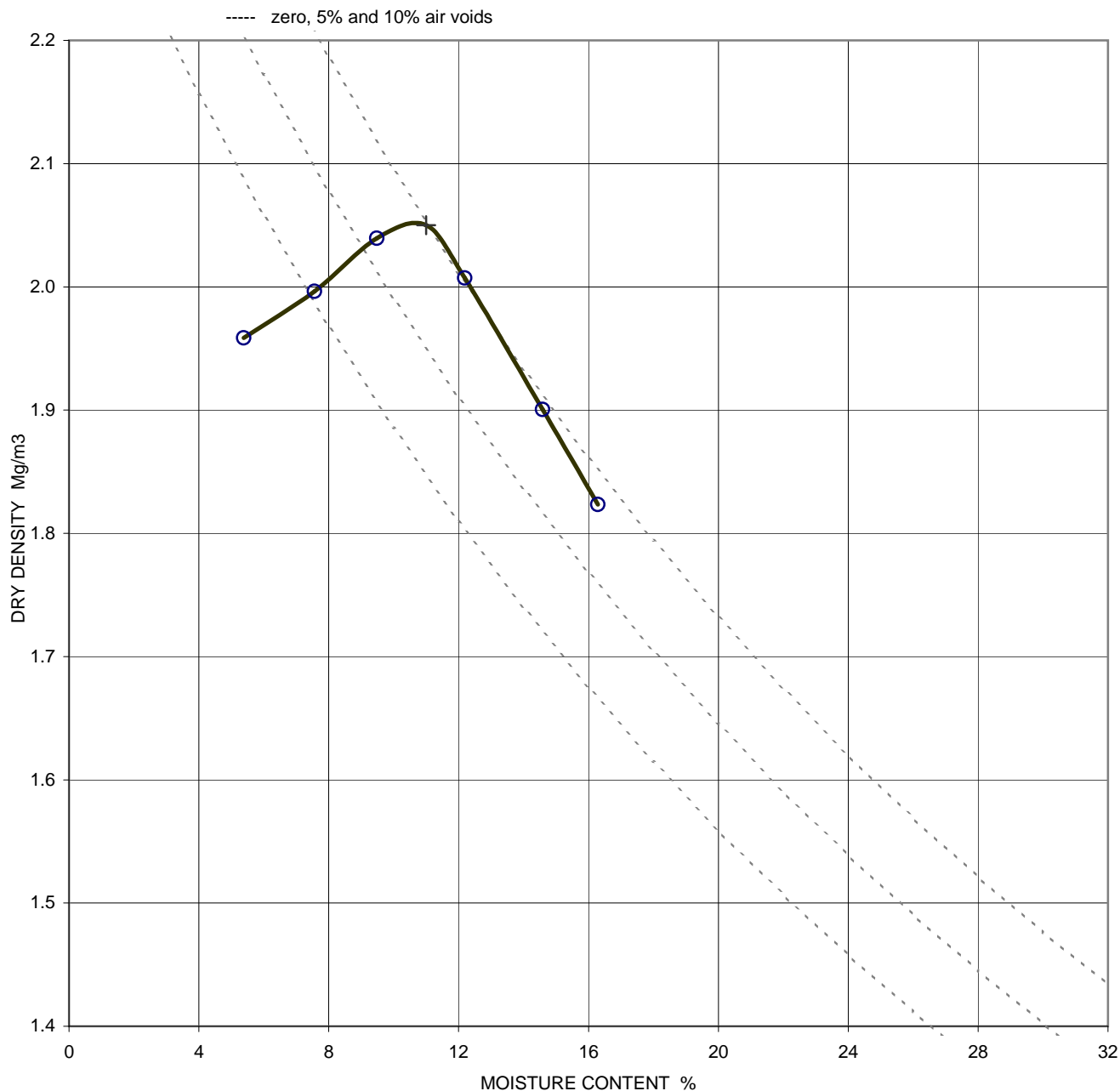


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Figure
COMPH 6

DRY DENSITY / MOISTURE CONTENT RELATIONSHIP
BS1377 : PART 4 : 1990 : HEAVY COMPACTION, 4.5 kg rammer

Project No	A1077-11	Sample Details:	Hole No	BH2
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	4.00
			Samp No	11
			Type	B
			ID	ESGA1077-11201110100000000077
			Spec Ref	



Soil description Gryeish brown slightly sandy slightly gravelly CLAY.

Test method BS 1377:part 4:1990: clause 3.5, 4.5 kg rammer in a 1 litre mould

Preparation Original material was natural, composite specimens tested

Material > 37.5mm 0 %

Material < 37.5mm > 20mm 2 %

Particle density 2.65 assumed

Remarks

Derived Parameters +

Maximum dry density, Mg/m3
2.05

Optimum moisture content, %
11

QA Ref
SLD 4, 3.5/6
Rev 66
Aug 11

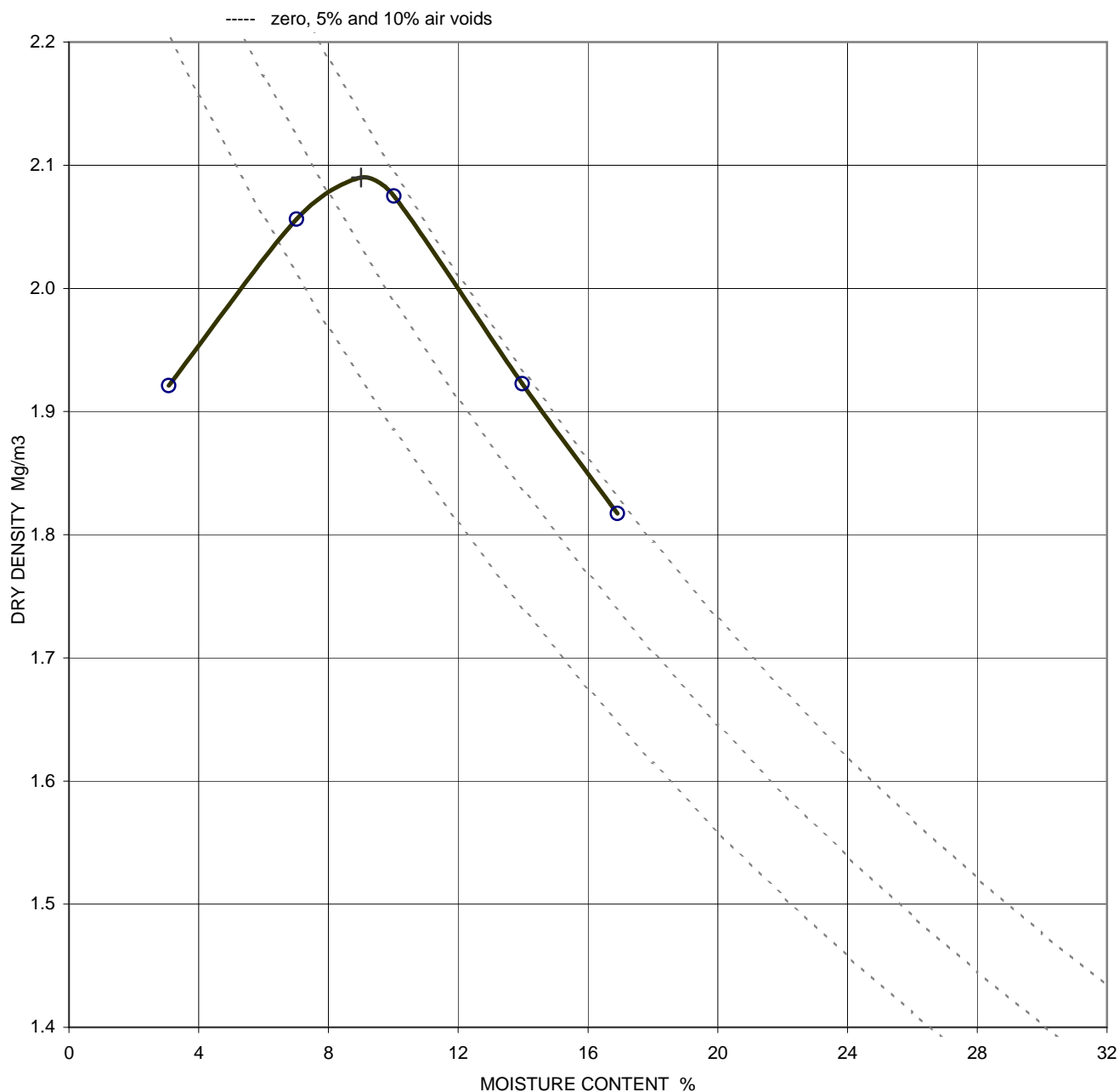


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Figure
COMPH 7

DRY DENSITY / MOISTURE CONTENT RELATIONSHIP
BS1377 : PART 4 : 1990 : HEAVY COMPACTION, 4.5 kg rammer

Project No	A1077-11	Sample Details:	Hole No		BH2	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		6.00	
			Samp No	15	Type	B
			ID	ESGA1077-11201110100000000081		
			Spec Ref			



Soil description Brown slightly sandy slightly gravelly CLAY.

Test method BS 1377:part 4:1990: clause 3.5, 4.5 kg rammer in a 1 litre mould

Preparation Original material was natural, separate specimens tested

Material > 37.5mm 0 %

Material < 37.5mm > 20mm 1 %

Particle density 2.65 assumed

Remarks

Derived Parameters +

Maximum dry density, Mg/m3
2.09

Optimum moisture content, %
9.0

QA Ref
 SLD 4, 3.5/6
 Rev 66
 Aug 11

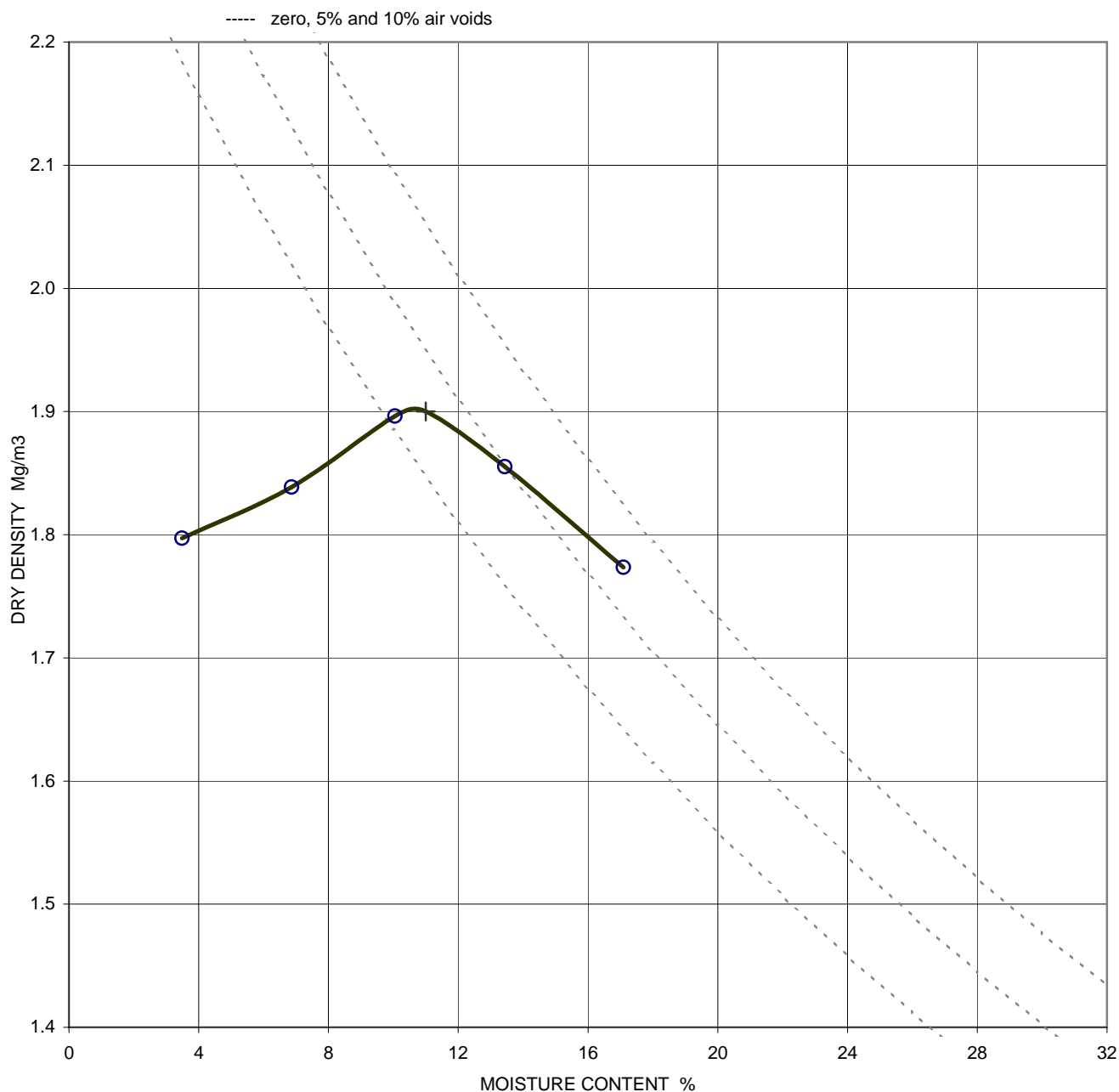


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Figure
COMPH 8

DRY DENSITY / MOISTURE CONTENT RELATIONSHIP
BS1377 : PART 4 : 1990 : HEAVY COMPACTION, 4.5 kg rammer

Project No	A1077-11	Sample Details:	Hole No		BH3	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		0.50	
			Samp No	3	Type	B
			ID	ESGA1077-11201110110000000128		
			Spec Ref			



Soil description Brown slightly gravelly sandy CLAY.

Test method BS 1377:part 4:1990: clause 3.5, 4.5 kg rammer in a 1 litre mould

Preparation Original material was natural, separate specimens tested

Material > 37.5mm 0 %

Material < 37.5mm > 20mm 0 %

Particle density 2.65 assumed

Remarks

Derived Parameters +

Maximum dry density, Mg/m3
1.90

Optimum moisture content, %
11

QA Ref
SLD 4, 3.5/6
Rev 66
Aug 11

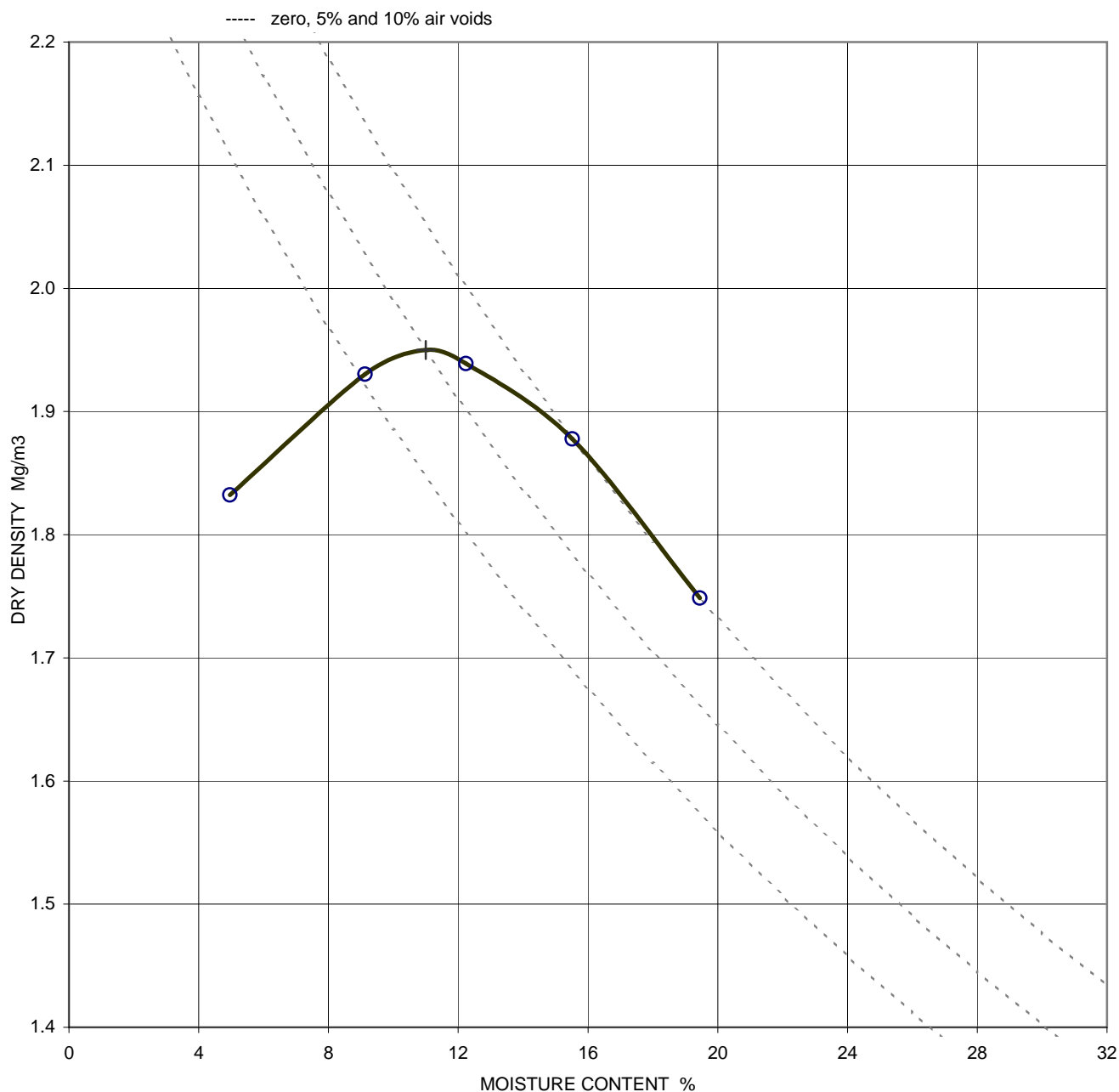


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Figure
COMPH 9

DRY DENSITY / MOISTURE CONTENT RELATIONSHIP
BS1377 : PART 4 : 1990 : HEAVY COMPACTION, 4.5 kg rammer

Project No	A1077-11	Sample Details:	Hole No		BH3	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		1.85	
			Samp No	7	Type	B
			ID	ESGA1077-11201110110000000132		
			Spec Ref			



Soil description Brown slightly sandy slightly gravelly CLAY.

Test method BS 1377:part 4:1990: clause 3.5, 4.5 kg rammer in a 1 litre mould

Preparation Original material was natural, separate specimens tested

Material > 37.5mm 0 %

Material < 37.5mm > 20mm 1 %

Particle density 2.65 assumed

Remarks

Derived Parameters +

Maximum dry density, Mg/m3
1.95

Optimum moisture content, %
11

QA Ref
SLD 4, 3.5/6
Rev 66
Aug 11

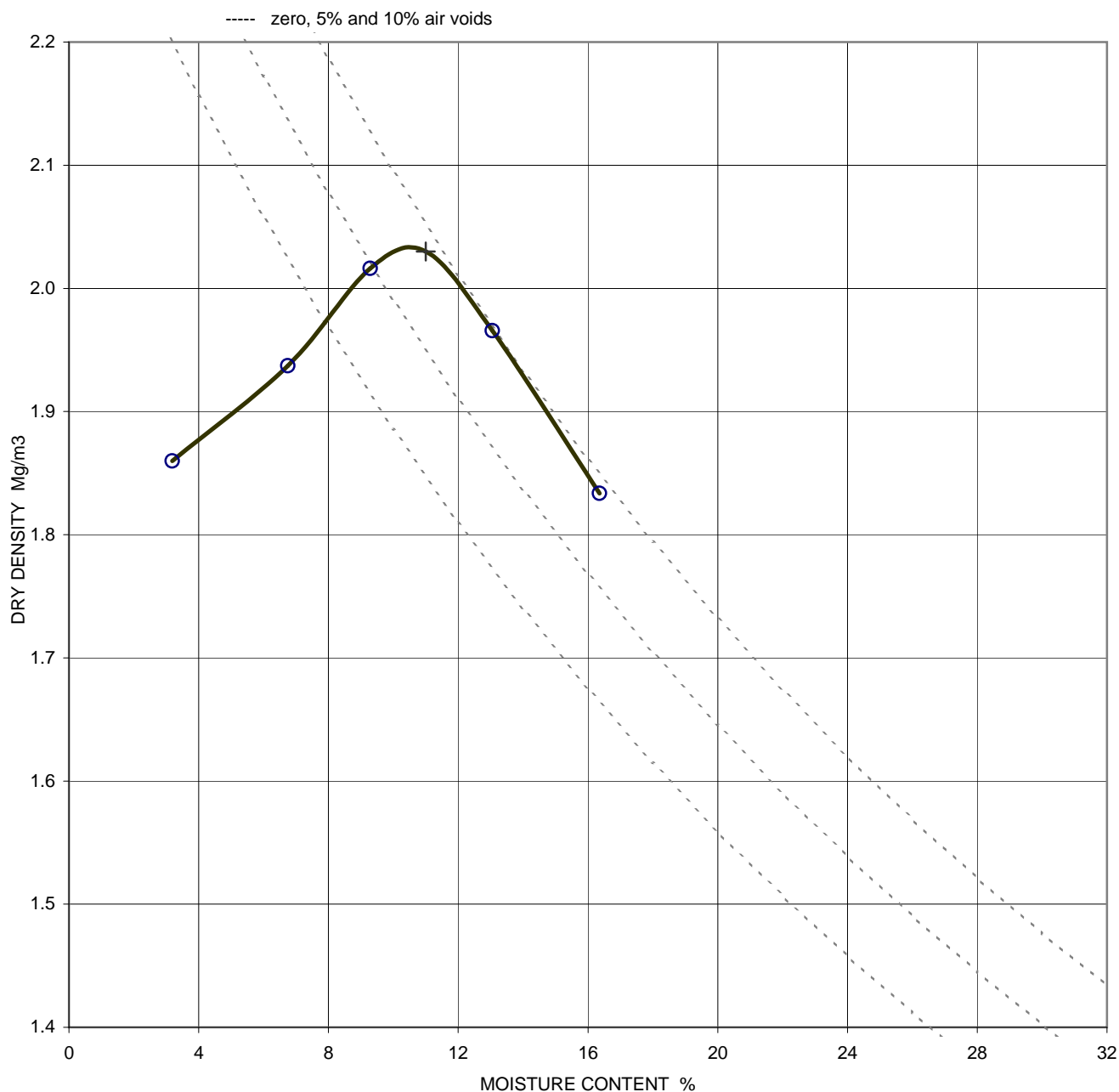


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Figure
COMPH 10

DRY DENSITY / MOISTURE CONTENT RELATIONSHIP
BS1377 : PART 4 : 1990 : HEAVY COMPACTION, 4.5 kg rammer

Project No	A1077-11	Sample Details:	Hole No		BH3	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		3.15	
			Samp No	11	Type	B
			ID	ESGA1077-11201110110000000136		
			Spec Ref			



Soil description Reddish brown slightly sandy slightly gravelly CLAY.

Test method BS 1377:part 4:1990: clause 3.5, 4.5 kg rammer in a 1 litre mould

Preparation Original material was natural, separate specimens tested

Material > 37.5mm 0 %

Material < 37.5mm > 20mm 0 %

Particle density 2.65 assumed

Remarks

Derived Parameters +

Maximum dry density, Mg/m³
2.03

Optimum moisture content, %
11

QA Ref
 SLD 4, 3.5/6
 Rev 66
 Aug 11

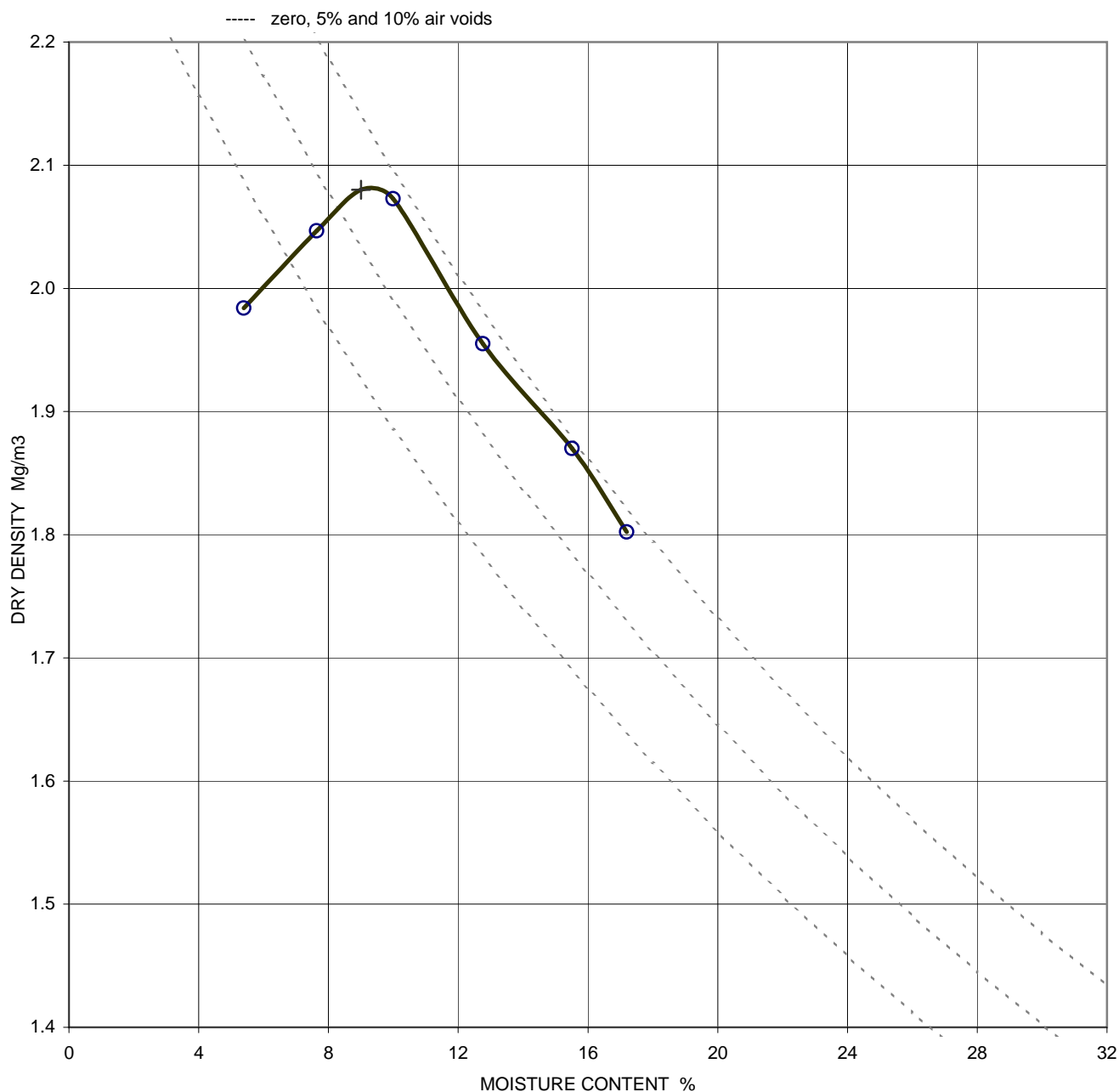


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Figure
COMPH 11

DRY DENSITY / MOISTURE CONTENT RELATIONSHIP
BS1377 : PART 4 : 1990 : HEAVY COMPACTION, 4.5 kg rammer

Project No	A1077-11	Sample Details:	Hole No	BH3
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	4.65
			Samp No	15
			Type	B
			ID	ESGA1077-11201110110000000140
			Spec Ref	



Soil description Brown slightly sandy slightly gravelly CLAY.

Test method BS 1377:part 4:1990: clause 3.5, 4.5 kg rammer in a 1 litre mould

Preparation Original material was natural, composite specimens tested

Material > 37.5mm 0 %

Material < 37.5mm > 20mm 1 %

Particle density 2.65 assumed

Remarks

Derived Parameters +

Maximum dry density, Mg/m3
2.08

Optimum moisture content, %
9.0

QA Ref
 SLD 4, 3.5/6
 Rev 66
 Aug 11

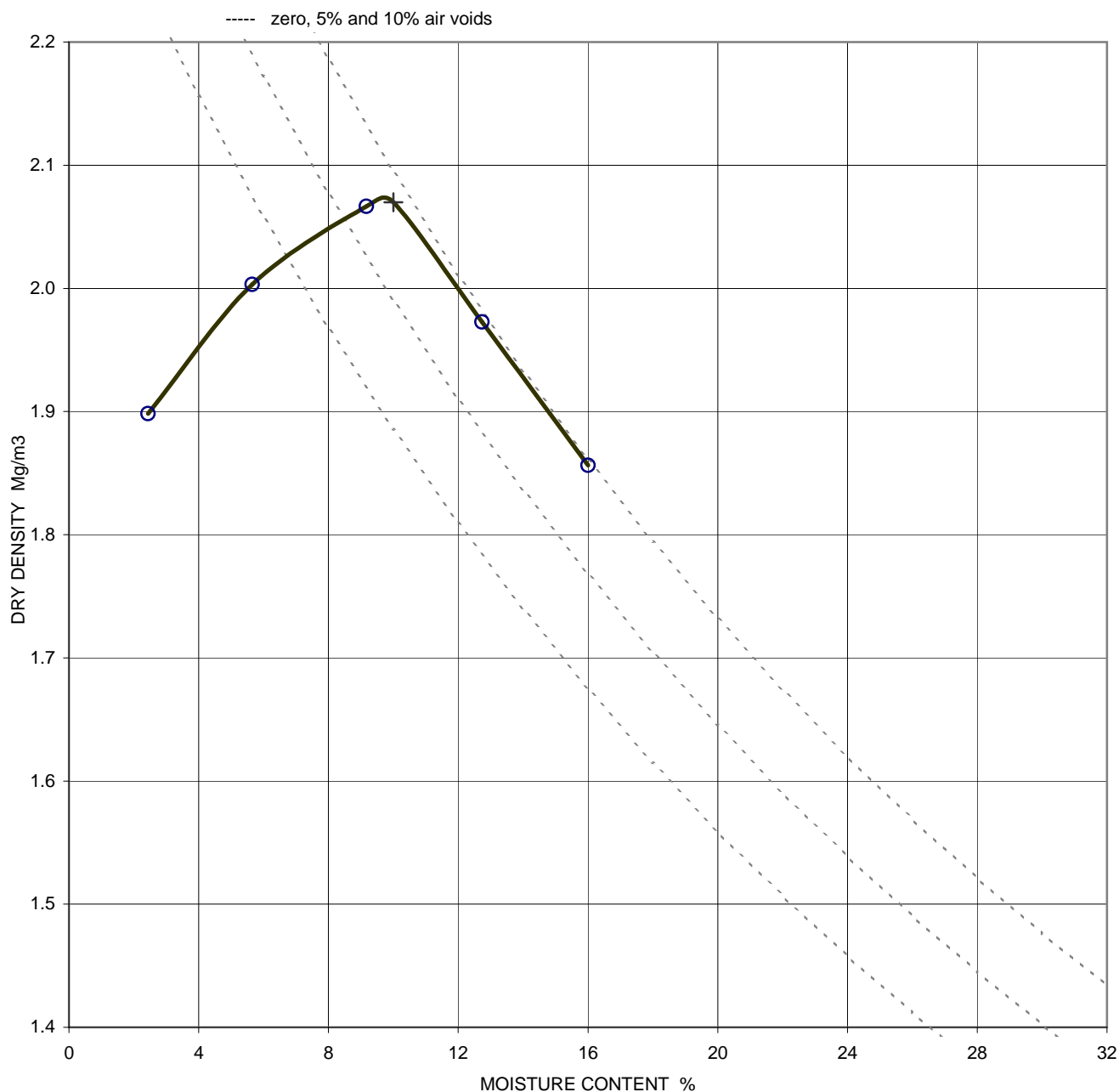


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Figure
COMPH 12

DRY DENSITY / MOISTURE CONTENT RELATIONSHIP
BS1377 : PART 4 : 1990 : HEAVY COMPACTION, 4.5 kg rammer

Project No	A1077-11	Sample Details:	Hole No		BH3	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		6.15	
			Samp No	19	Type	B
			ID	ESGA1077-11201110110000000144		
			Spec Ref			



Soil description Brown slightly sandy slightly gravelly CLAY.

Test method BS 1377:part 4:1990: clause 3.5, 4.5 kg rammer in a 1 litre mould

Preparation Original material was natural, separate specimens tested

Material > 37.5mm 0 %

Material < 37.5mm > 20mm 1 %

Particle density 2.65 assumed

Remarks

Derived Parameters +

Maximum dry density, Mg/m3
2.07

Optimum moisture content, %
10

QA Ref
 SLD 4, 3.5/6
 Rev 66
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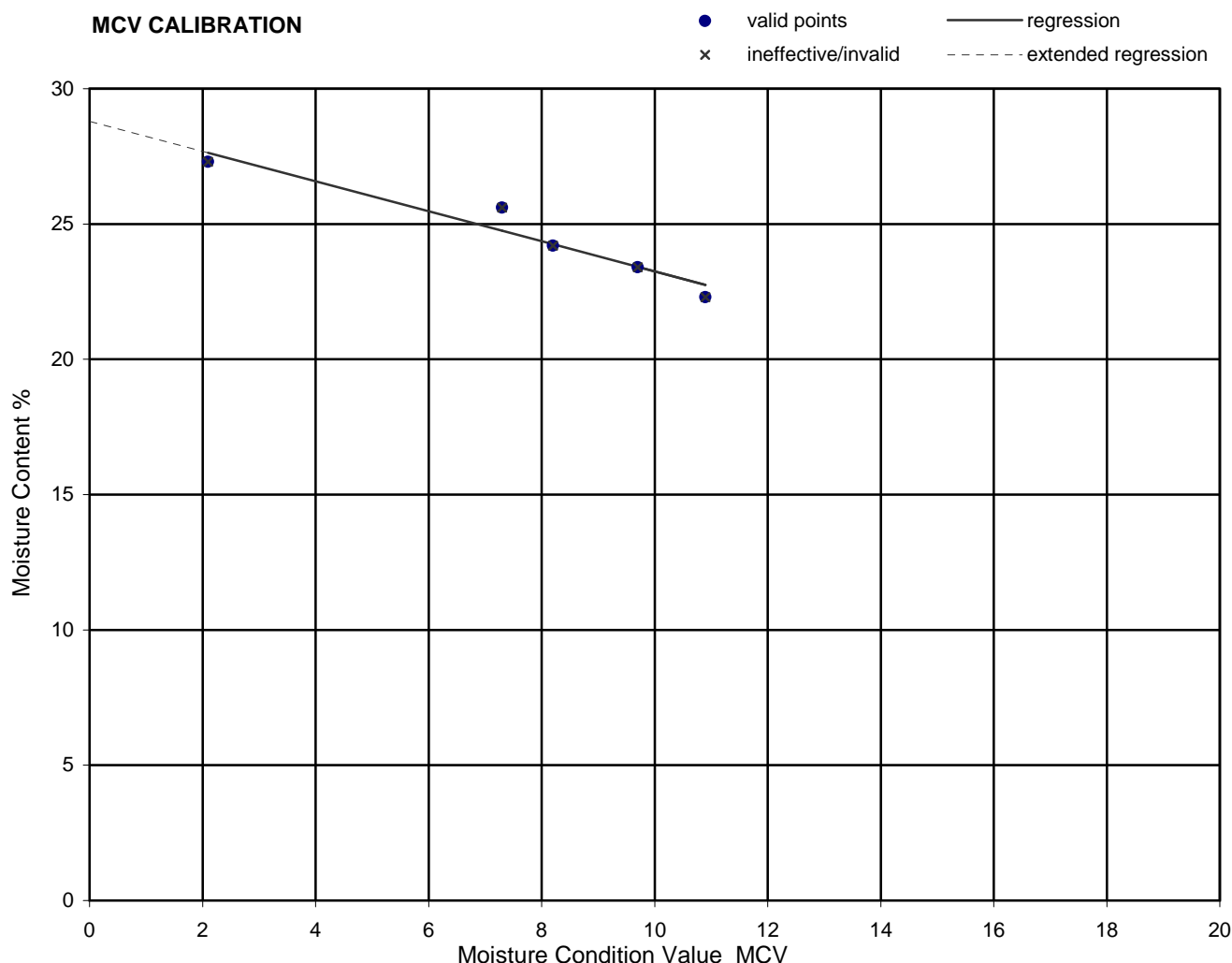
Figure
COMPH 13

MOISTURE CONDITION VALUE (MCV) / MOISTURE CONTENT RELATIONSHIP

BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH1	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.50	
			Samp No	4	Type B
			ID	ESGA1077-11201110100000000004	
			Spec Ref		

MCV CALIBRATION



Characteristics of calibration line (determined using linear regression)

Intercept	28.8
Slope	-0.55
Sensitivity (Change in MCV per 1% moisture content)	1.80
Correlation (proximity of test points to regression line)	-0.97
Method of interpretation of MCV	Steepest straight line

The above characteristics are NOT covered by UKAS accreditation to BS1377

QA Ref

SLD 4, 5.5
Rev 72
Aug 11



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Figure

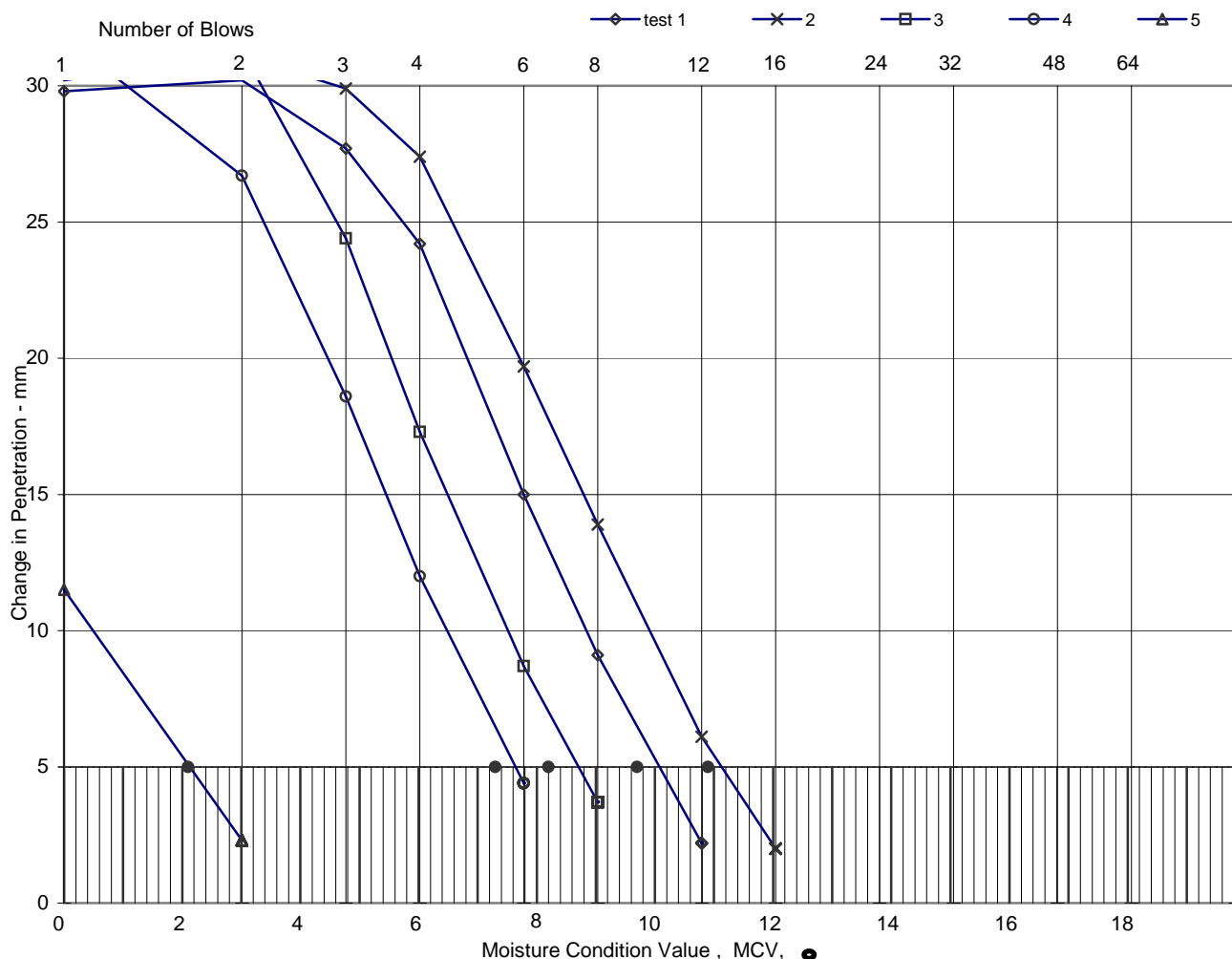
MCVREL 1

sheet 1 of 2

MOISTURE CONDITION VALUE (MCV) / MOISTURE CONTENT RELATIONSHIP

BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH1		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.50		
			Samp No	4	Type	B
			ID	ESGA1077-11201110100000000004		
			Spec Ref			



Test No	* ineffective / invalid point	1	2	3	4	5
Moisture Condition Value		9.7	10.9	8.2	7.3	2.1
Moisture Content	%	23.4	22.3	24.2	25.6	27.3
Bulk density after test	Mg/m ³	2.00	2.04	2.00	1.98	1.91
Dry density after test	Mg/m ³	1.62	1.67	1.61	1.58	1.50

Soil description	Dark brown slightly gravelly sandy CLAY with occasional rootlets.
Procedure / Preparation	BS1377:Part 1 and Part 4, clause 5.5
Remarks	

Initial moisture content <20mm	%	23.4
Material retained on 20mm sieve	%	0

Method of determining MCV
Steepest straight line

QA Ref

SLD 4, 5.5
Rev 72
Aug 11



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Figure

MCVREL 1

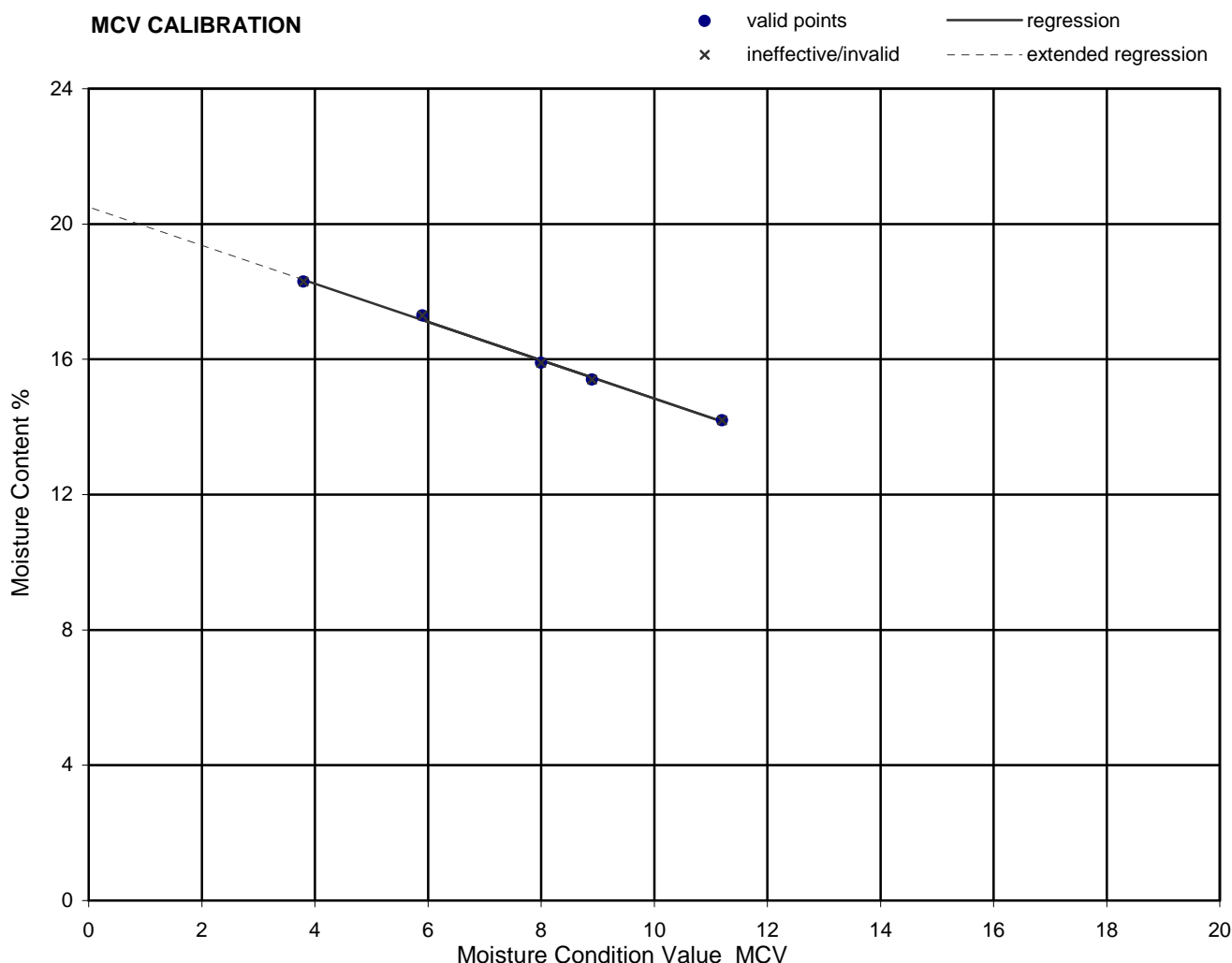
sheet 2 of 2

MOISTURE CONDITION VALUE (MCV) / MOISTURE CONTENT RELATIONSHIP

BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH1	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	2.00	
			Samp No	8	Type B
			ID	ESGA1077-11201110100000000009	
			Spec Ref		

MCV CALIBRATION



Characteristics of calibration line (determined using linear regression)

Intercept	20.5
Slope	-0.57
Sensitivity (Change in MCV per 1% moisture content)	1.76
Correlation (proximity of test points to regression line)	-1.00
Method of interpretation of MCV	Steepest straight line

The above characteristics are NOT covered by UKAS accreditation to BS1377

QA Ref
SLD 4, 5.5
Rev 72
Aug 11



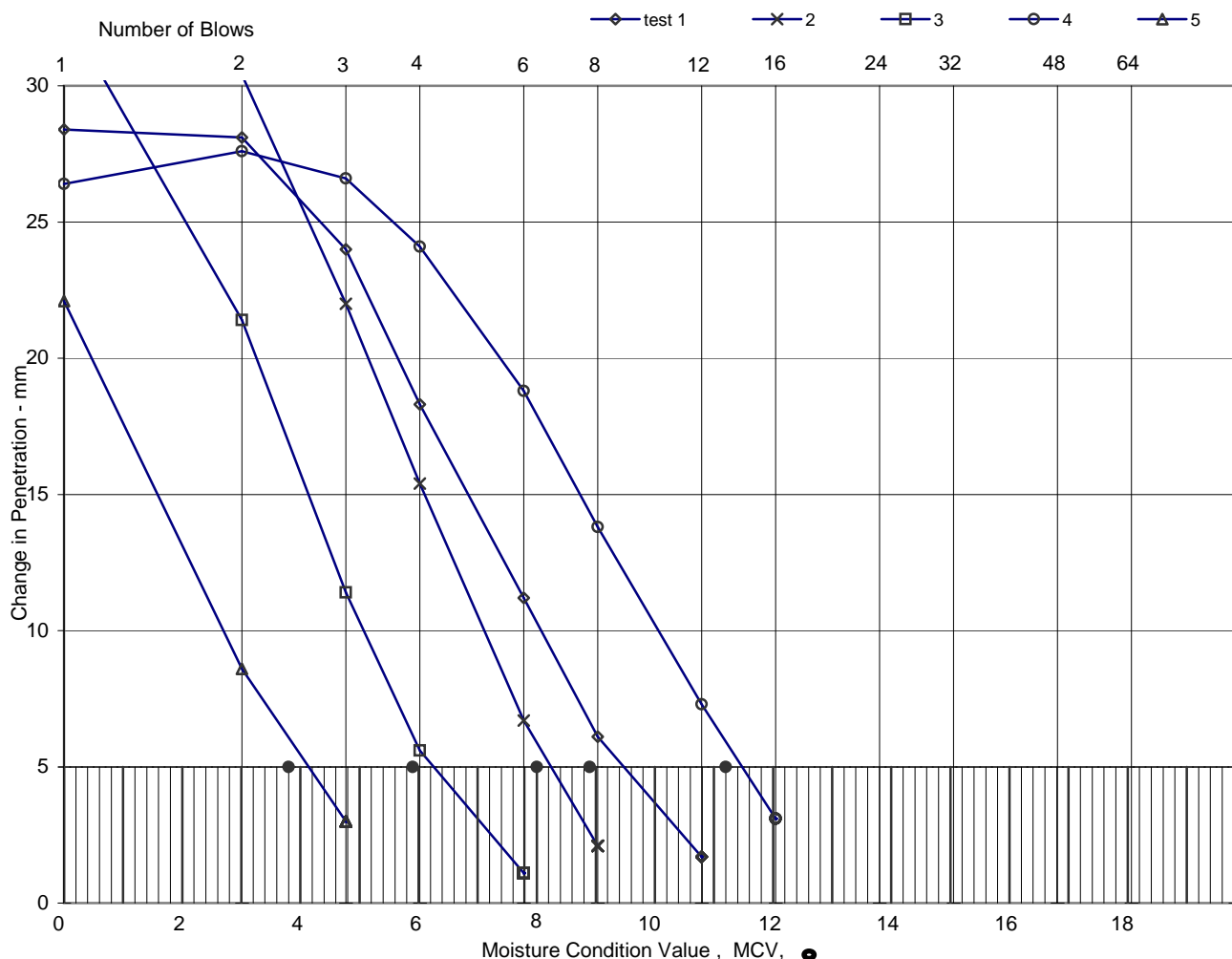
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Figure
MCVREL 2
sheet 1 of 2

MOISTURE CONDITION VALUE (MCV) / MOISTURE CONTENT RELATIONSHIP

BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH1
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	2.00
			Samp No	8
			Type	B
			ID	ESGA1077-11201110100000000009
			Spec Ref	



Test No	* ineffective / invalid point	1	2	3	4	5
Moisture Condition Value		8.9	8.0	5.9	11.2	3.8
Moisture Content	%	15.4	15.9	17.3	14.2	18.3
Bulk density after test	Mg/m ³	2.19	2.16	2.11	2.21	2.09
Dry density after test	Mg/m ³	1.90	1.86	1.80	1.94	1.77

Soil description	Brown slightly sandy slightly gravelly CLAY.
Procedure / Preparation	BS1377:Part 1 and Part 4, clause 5.5
Remarks	

Initial moisture content <20mm	%	15.4
Material retained on 20mm sieve	%	4

Method of determining MCV
Steepest straight line

QA Ref

SLD 4, 5.5
Rev 72
Aug 11



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Figure

MCVREL 2

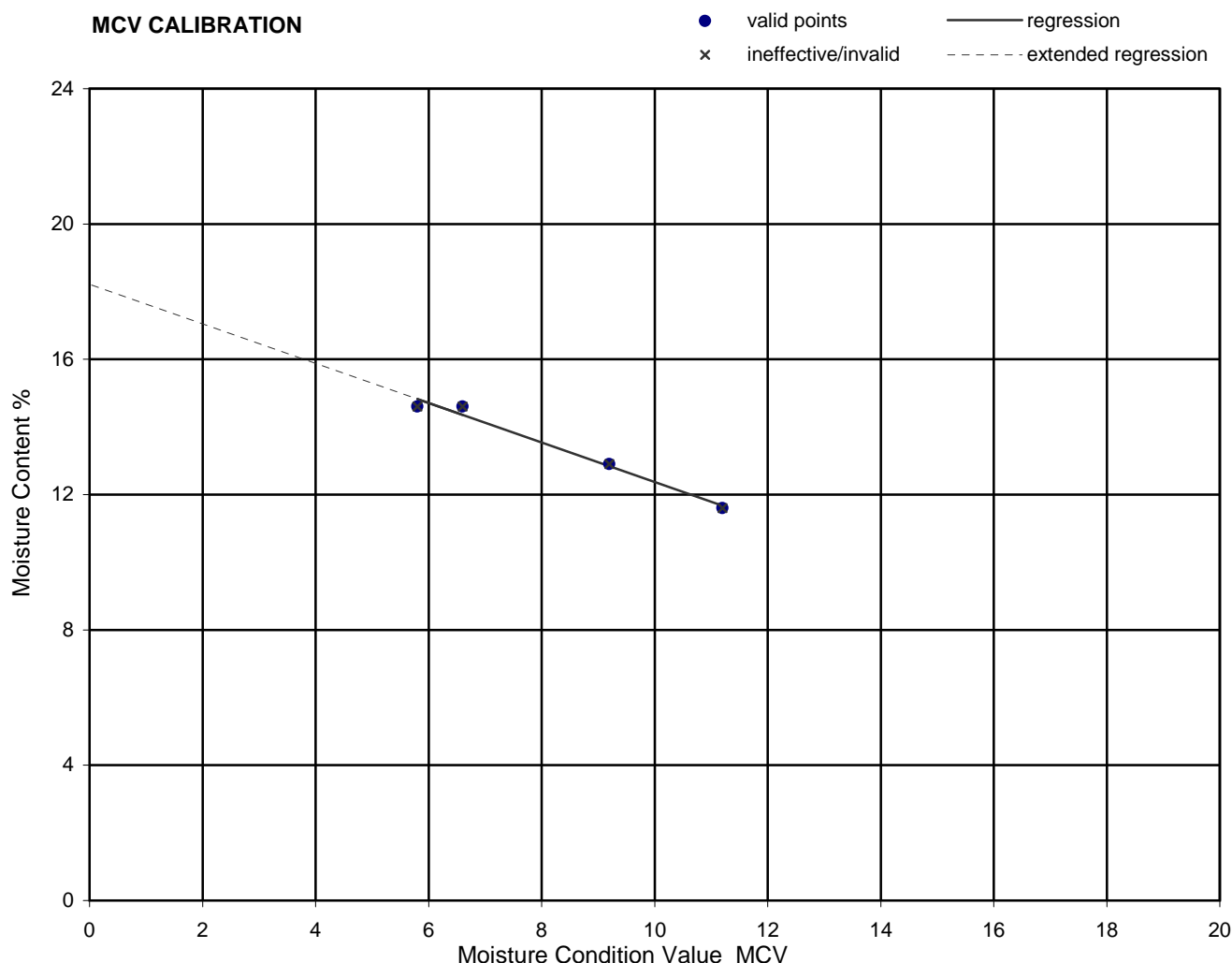
sheet 2 of 2

MOISTURE CONDITION VALUE (MCV) / MOISTURE CONTENT RELATIONSHIP

BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH1	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	4.00	
			Samp No	12	Type B
			ID	ESGA1077-11201110100000000013	
			Spec Ref		

MCV CALIBRATION



Characteristics of calibration line (determined using linear regression)

Intercept	18.2
Slope	-0.58
Sensitivity (Change in MCV per 1% moisture content)	1.71
Correlation (proximity of test points to regression line)	-0.99
Method of interpretation of MCV	Steepest straight line

The above characteristics are NOT covered by UKAS accreditation to BS1377

QA Ref

SLD 4, 5.5
Rev 72
Aug 11



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Figure

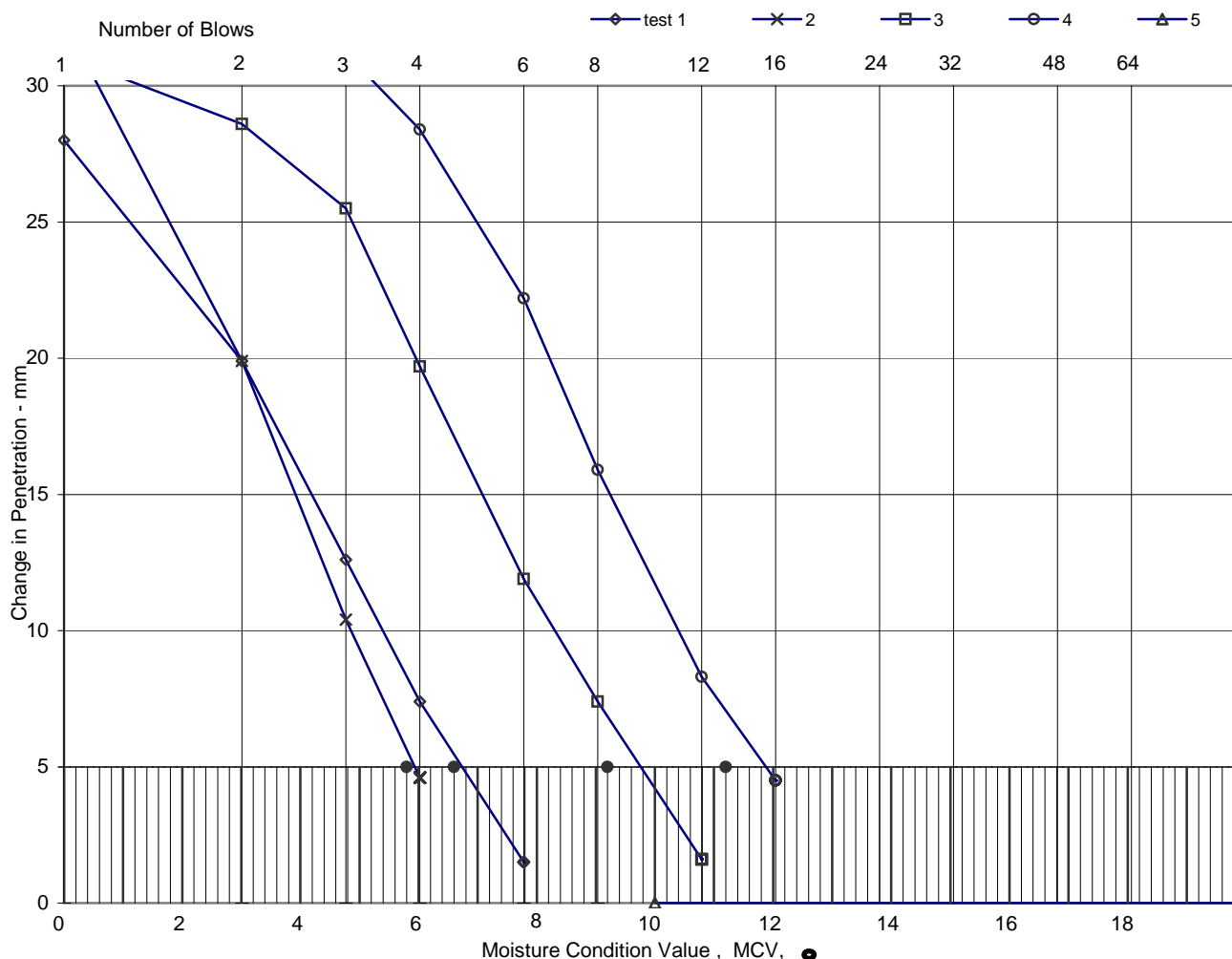
MCVREL 3

sheet 1 of 2

MOISTURE CONDITION VALUE (MCV) / MOISTURE CONTENT RELATIONSHIP

BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No		BH1	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		4.00	
			Samp No	12	Type	B
			ID	ESGA1077-11201110100000000013		
			Spec Ref			



Test No	* ineffective / invalid point	1	2	3	4	
Moisture Condition Value		6.6	5.8	9.2	11.2	
Moisture Content	%	14.6	14.6	12.9	11.6	
Bulk density after test	Mg/m ³	2.20	2.19	2.25	2.29	
Dry density after test	Mg/m ³	1.92	1.91	1.99	2.05	

Soil description	Brown slightly sandy slightly gravelly CLAY.
Procedure / Preparation	BS1377:Part 1 and Part 4, clause 5.5
Remarks	

Initial moisture content <20mm	%	14.6
Material retained on 20mm sieve	%	5

Method of determining MCV
Steepest straight line

QA Ref

SLD 4, 5.5
Rev 72
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Figure

MCVREL 3

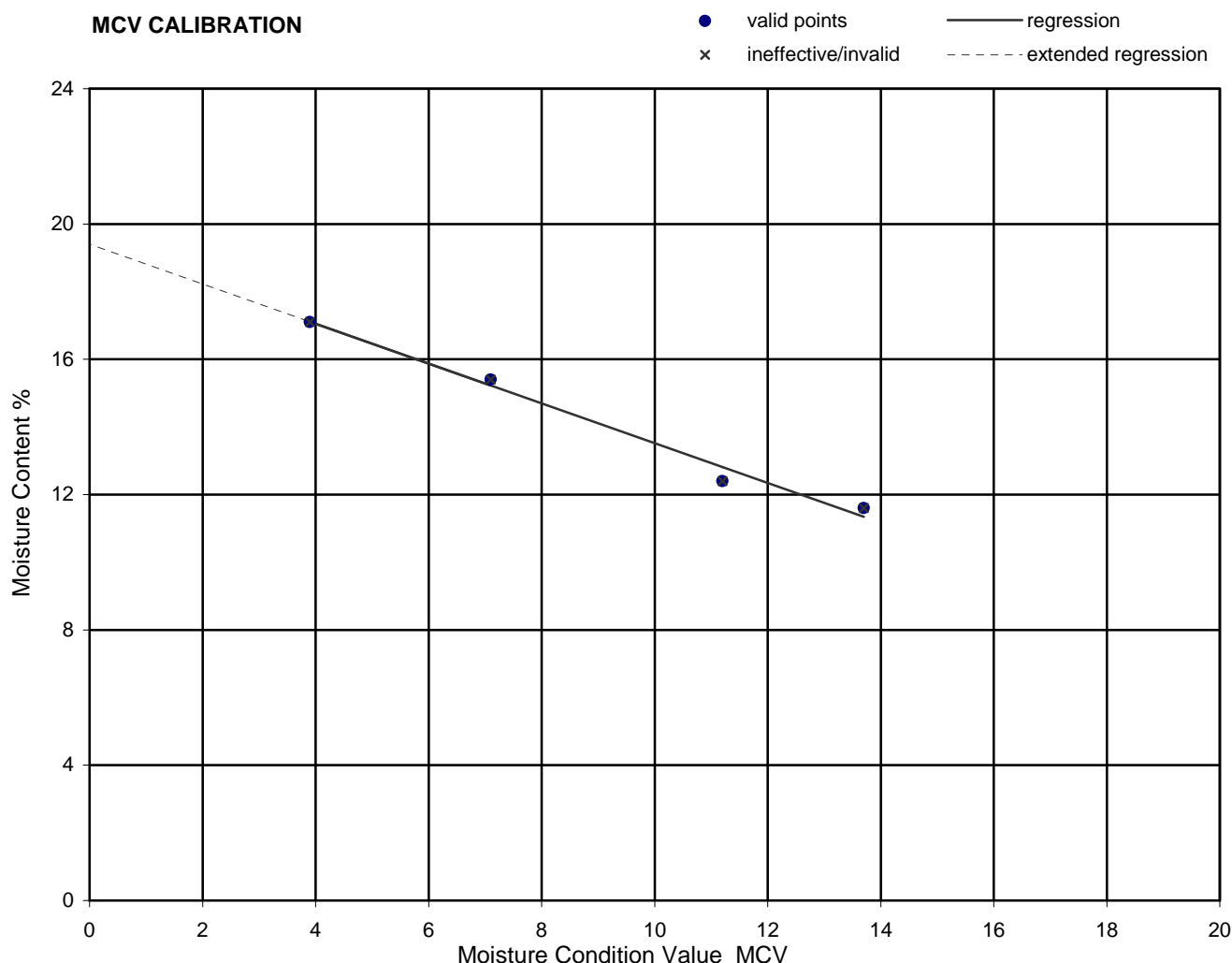
sheet 2 of 2

MOISTURE CONDITION VALUE (MCV) / MOISTURE CONTENT RELATIONSHIP

BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH1	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	5.65	
			Samp No	16	Type B
			ID	ESGA1077-11201110100000000017	
			Spec Ref		

MCV CALIBRATION



Characteristics of calibration line (determined using linear regression)

Intercept	19.4
Slope	-0.59
Sensitivity (Change in MCV per 1% moisture content)	1.70
Correlation (proximity of test points to regression line)	-0.99
Method of interpretation of MCV	Steepest straight line

The above characteristics are NOT covered by UKAS accreditation to BS1377

QA Ref

SLD 4, 5.5
Rev 72
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Figure

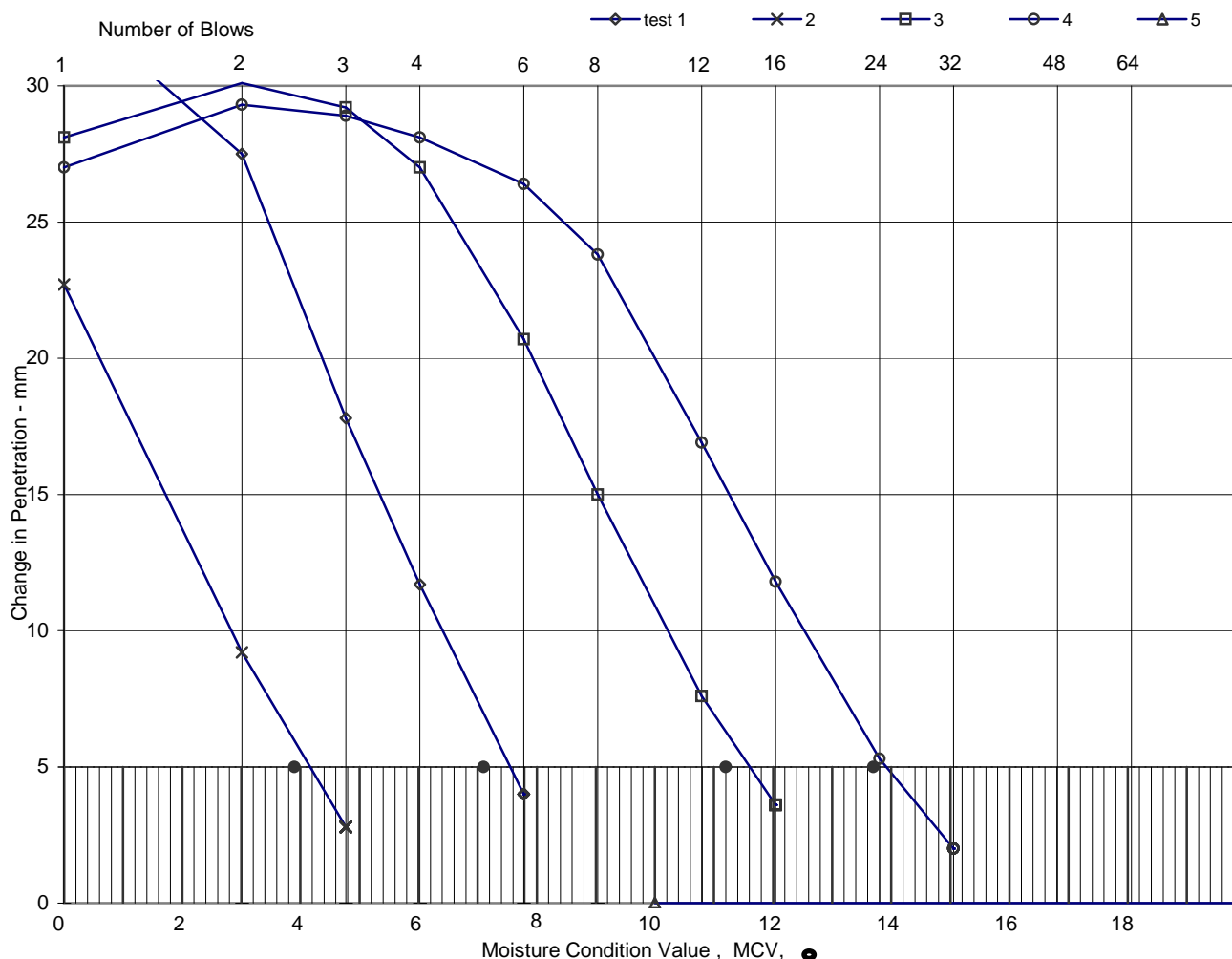
MCVREL 4

sheet 1 of 2

MOISTURE CONDITION VALUE (MCV) / MOISTURE CONTENT RELATIONSHIP

BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH1		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	5.65		
			Samp No	16	Type	B
			ID	ESGA1077-11201110100000000017		
			Spec Ref			



Test No	* ineffective / invalid point	1	2	3	4	
Moisture Condition Value		7.1	3.9	11.2	13.7	
Moisture Content	%	15.4	17.1	12.4	11.6	
Bulk density after test	Mg/m ³	2.21	2.13	2.28	2.30	
Dry density after test	Mg/m ³	1.92	1.82	2.03	2.06	

Soil description	Brown slightly sandy slightly gravelly CLAY.
Procedure / Preparation	BS1377:Part 1 and Part 4, clause 5.5
Remarks	

Initial moisture content <20mm	%	15.4
Material retained on 20mm sieve	%	3.7

Method of determining MCV
Steepest straight line

QA Ref

SLD 4, 5.5
Rev 72
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Figure

MCVREL 4

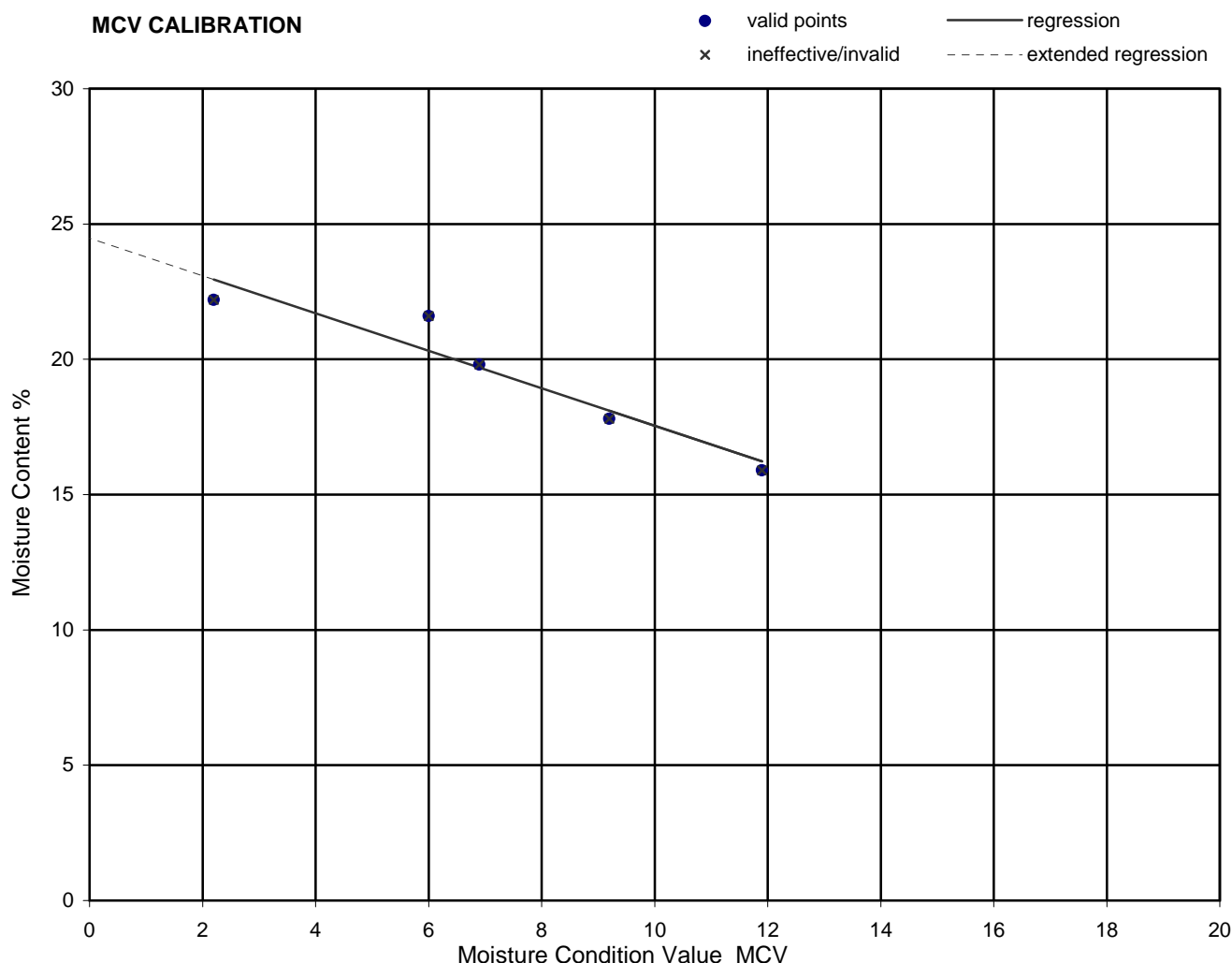
sheet 2 of 2

MOISTURE CONDITION VALUE (MCV) / MOISTURE CONTENT RELATIONSHIP

BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH2
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.50
			Samp No	3
			Type	B
			ID	ESGA1077-11201110100000000069
			Spec Ref	

MCV CALIBRATION



Characteristics of calibration line (determined using linear regression)

Intercept	24.5
Slope	-0.69
Sensitivity (Change in MCV per 1% moisture content)	1.44
Correlation (proximity of test points to regression line)	-0.96
Method of interpretation of MCV	Steepest straight line

The above characteristics are NOT covered by UKAS accreditation to BS1377

QA Ref

SLD 4, 5.5
Rev 72
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Figure

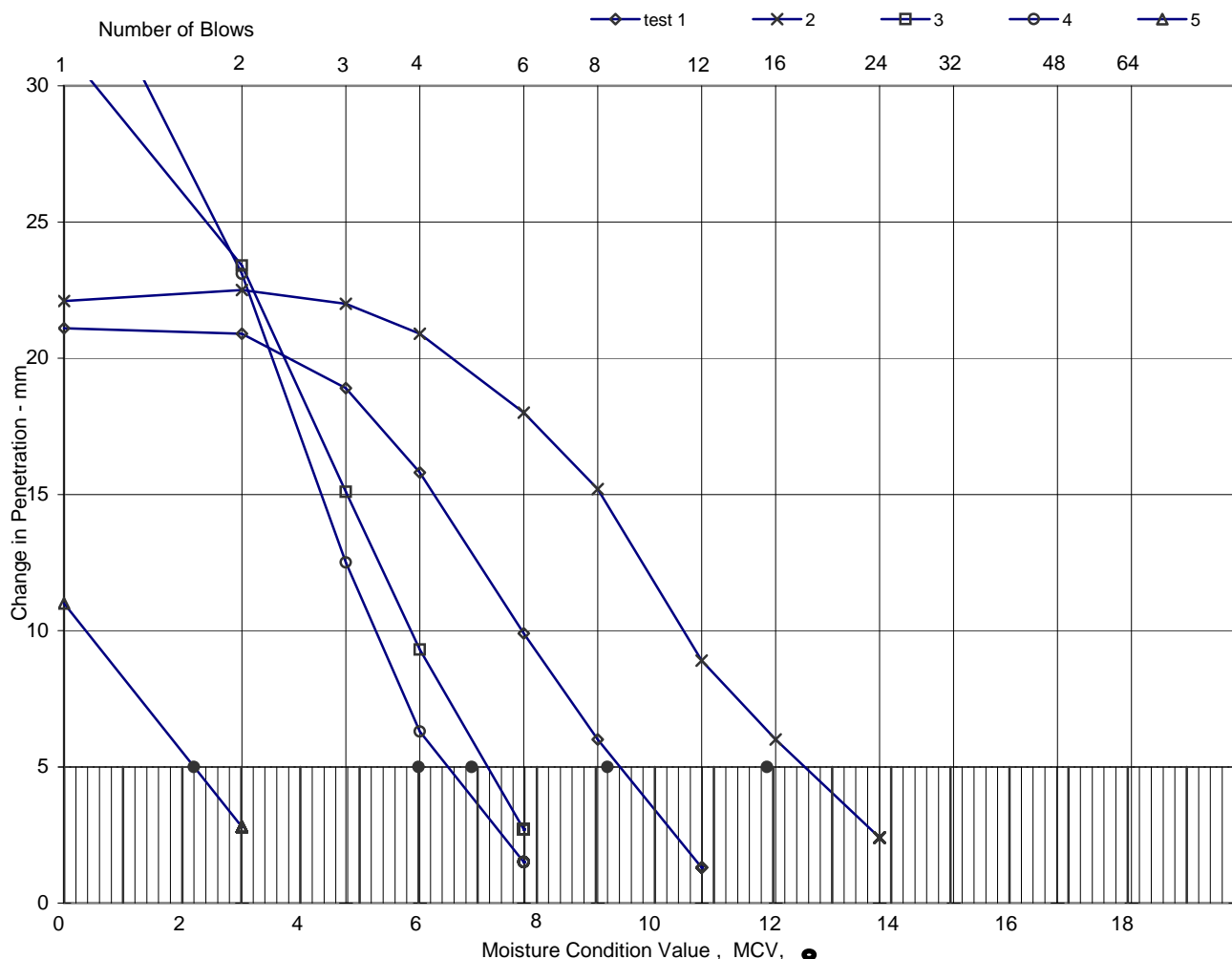
MCVREL 5

sheet 1 of 2

MOISTURE CONDITION VALUE (MCV) / MOISTURE CONTENT RELATIONSHIP

BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH2		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.50		
			Samp No	3	Type	B
			ID	ESGA1077-11201110100000000069		
			Spec Ref			



Test No	* ineffective / invalid point	1	2	3	4	5
Moisture Condition Value		9.2	11.9	6.9	6.0	2.2
Moisture Content	%	17.8	15.9	19.8	21.6	22.2
Bulk density after test	Mg/m ³	2.09	2.16	2.05	2.02	1.95
Dry density after test	Mg/m ³	1.77	1.86	1.71	1.66	1.60

Soil description	Brown slightly gravelly sandy silty CLAY.
Procedure / Preparation	BS1377:Part 1 and Part 4, clause 5.5
Remarks	

Initial moisture content <20mm	%	17.8
Material retained on 20mm sieve	%	0

Method of determining MCV
Steepest straight line

QA Ref

SLD 4, 5.5
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Figure

MCVREL 5

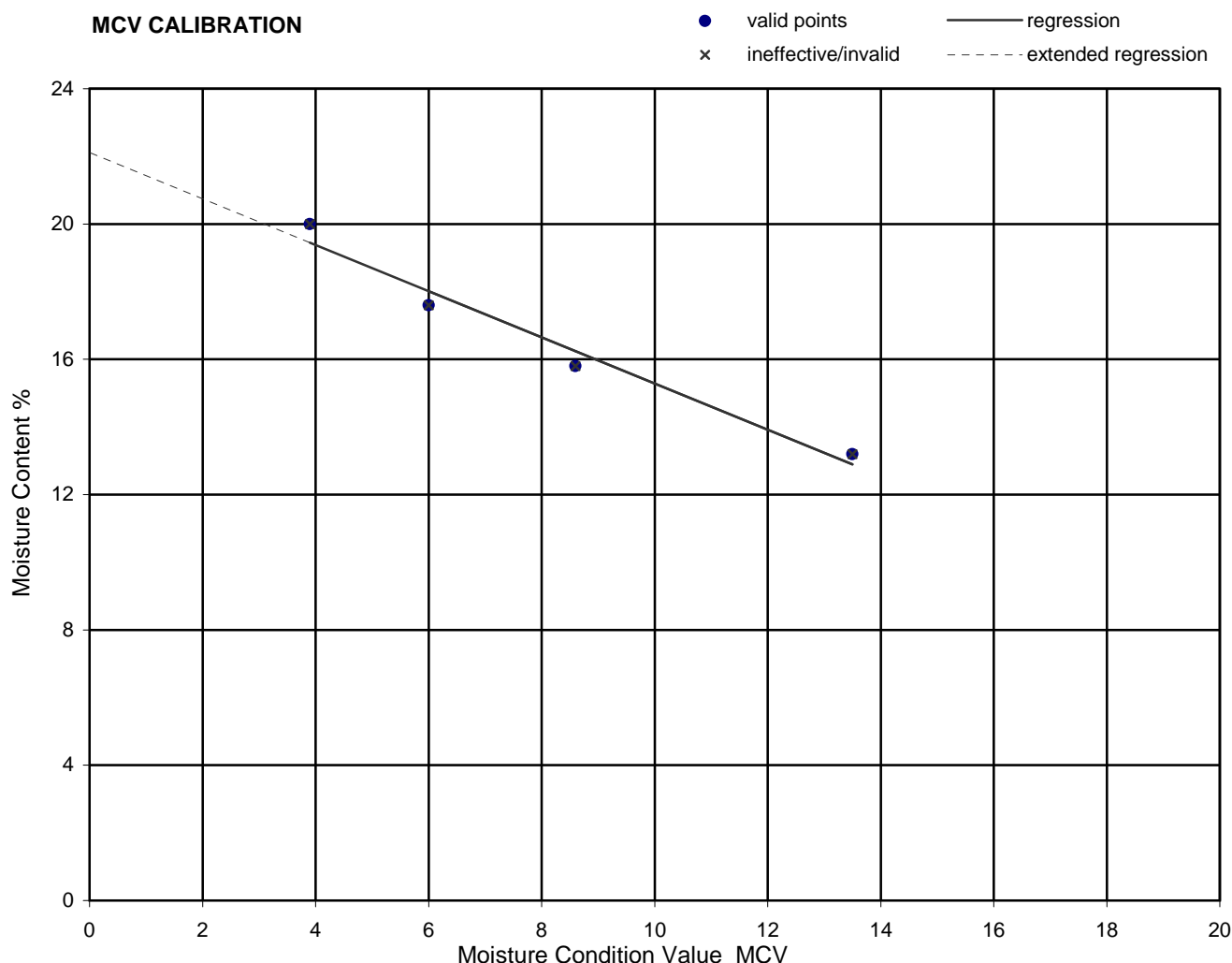
sheet 2 of 2

MOISTURE CONDITION VALUE (MCV) / MOISTURE CONTENT RELATIONSHIP

BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH2		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	2.00		
			Samp No	7	Type	B
			ID	ESGA1077-11201110100000000073		
			Spec Ref			

MCV CALIBRATION



Characteristics of calibration line (determined using linear regression)

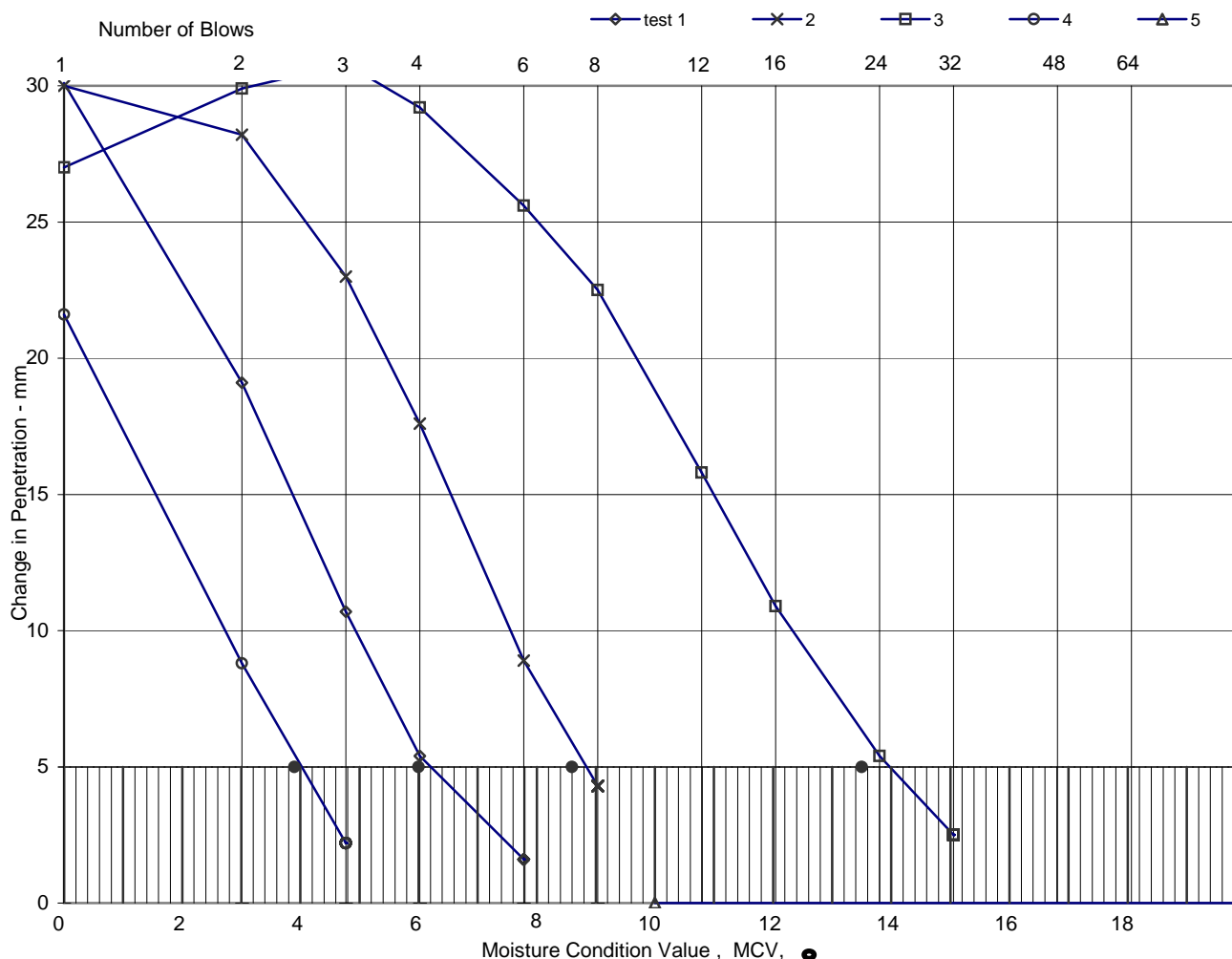
Intercept	22.1
Slope	-0.68
Sensitivity (Change in MCV per 1% moisture content)	1.46
Correlation (proximity of test points to regression line)	-0.98
Method of interpretation of MCV	Steepest straight line

The above characteristics are NOT covered by UKAS accreditation to BS1377

MOISTURE CONDITION VALUE (MCV) / MOISTURE CONTENT RELATIONSHIP

BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH2
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	2.00
			Samp No	7
			Type	B
			ID	ESGA1077-11201110100000000073
			Spec Ref	



Test No	* ineffective / invalid point	1	2	3	4	
Moisture Condition Value		6.0	8.6	13.5	3.9	
Moisture Content	%	17.6	15.8	13.2	20.0	
Bulk density after test	Mg/m ³	2.09	2.15	2.23	2.04	
Dry density after test	Mg/m ³	1.78	1.86	1.97	1.70	

Soil description	Brown slightly sandy slightly gravelly CLAY.
Procedure / Preparation	BS1377:Part 1 and Part 4, clause 5.5
Remarks	

Initial moisture content <20mm	%	17.6
Material retained on 20mm sieve	%	2

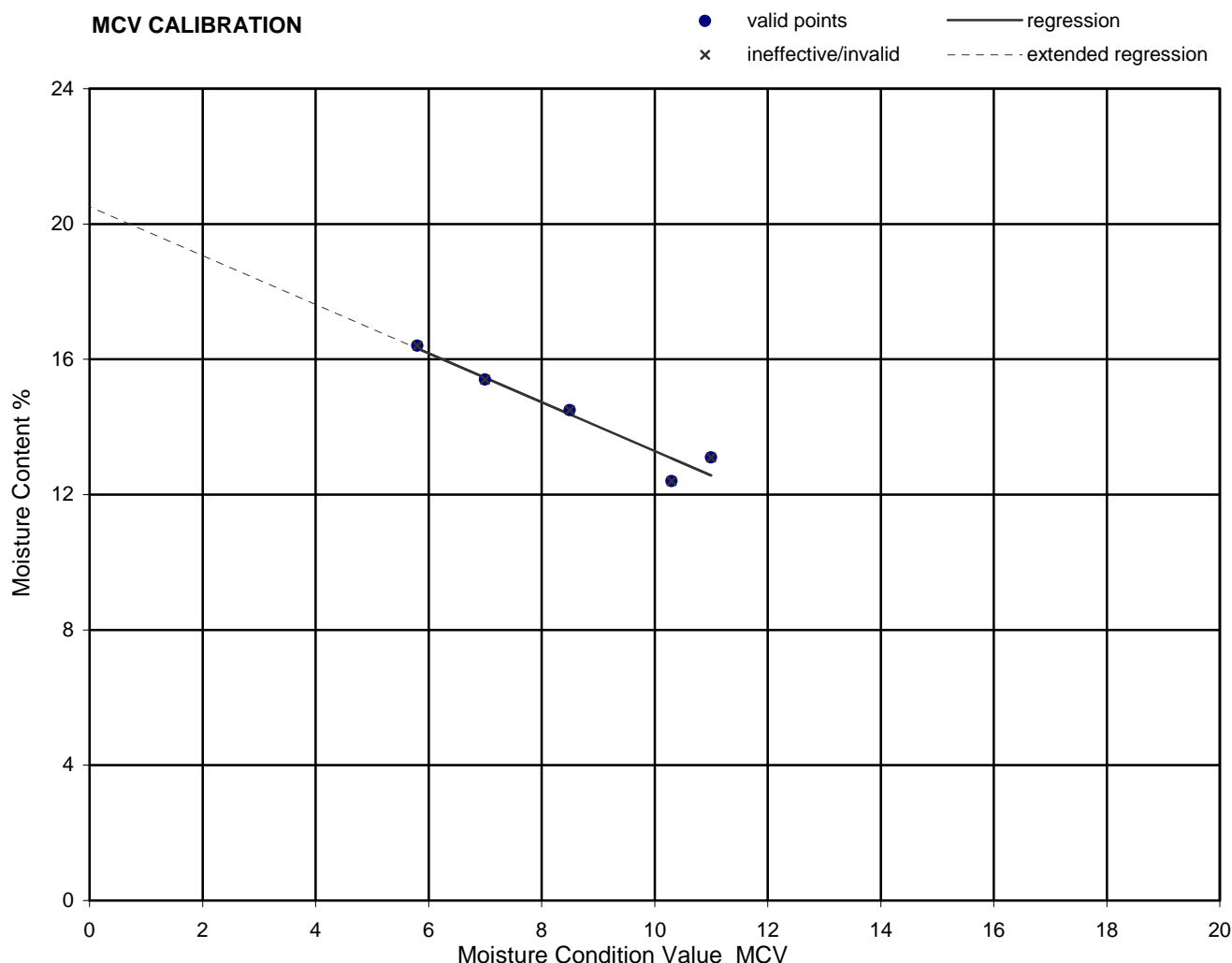
Method of determining MCV
Steepest straight line

MOISTURE CONDITION VALUE (MCV) / MOISTURE CONTENT RELATIONSHIP

BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH2	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	4.00	
			Samp No	11	Type B
			ID	ESGA1077-11201110100000000077	
			Spec Ref		

MCV CALIBRATION



Characteristics of calibration line (determined using linear regression)

Intercept	20.5
Slope	-0.72
Sensitivity (Change in MCV per 1% moisture content)	1.38
Correlation (proximity of test points to regression line)	-0.96
Method of interpretation of MCV	Steepest straight line

The above characteristics are NOT covered by UKAS accreditation to BS1377

QA Ref

SLD 4, 5.5
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Figure

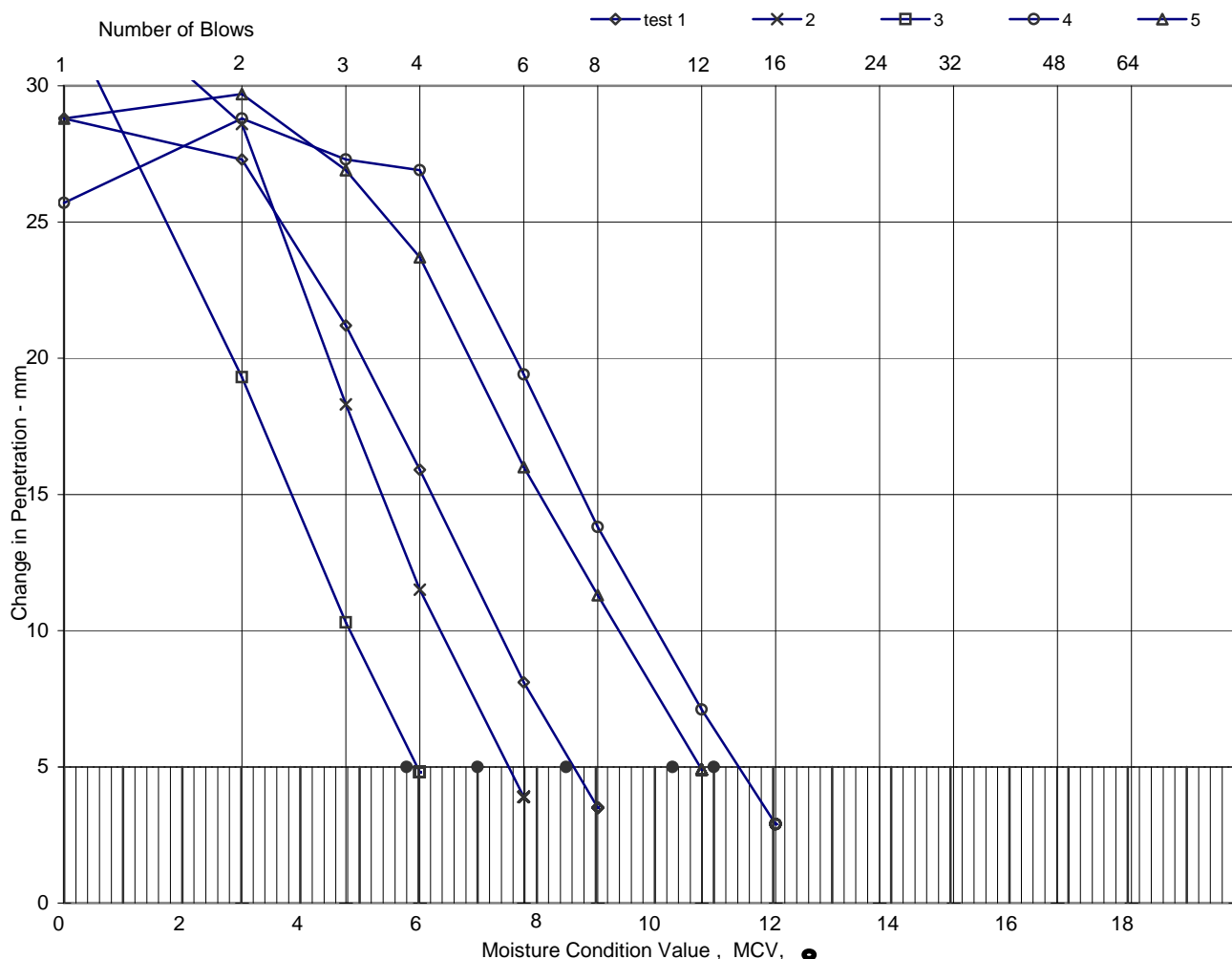
MCVREL 7

sheet 1 of 2

MOISTURE CONDITION VALUE (MCV) / MOISTURE CONTENT RELATIONSHIP

BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No		BH2	
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)		4.00	
			Samp No	11	Type	B
			ID	ESGA1077-11201110100000000077		
			Spec Ref			



Test No	* ineffective / invalid point	1	2	3	4	5
Moisture Condition Value		8.5	7.0	5.8	11.0	10.3
Moisture Content	%	14.5	15.4	16.4	13.1	12.4
Bulk density after test	Mg/m ³	2.21	2.17	2.15	2.24	2.23
Dry density after test	Mg/m ³	1.93	1.88	1.85	1.98	1.98

Soil description	Gryeish brown slightly sandy slightly gravelly CLAY.
Procedure / Preparation	BS1377:Part 1 and Part 4, clause 5.5
Remarks	

Initial moisture content <20mm	%	14.5
Material retained on 20mm sieve	%	1.7

Method of determining MCV
Steepest straight line

QA Ref

SLD 4, 5.5
Rev 72
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Figure

MCVREL 7

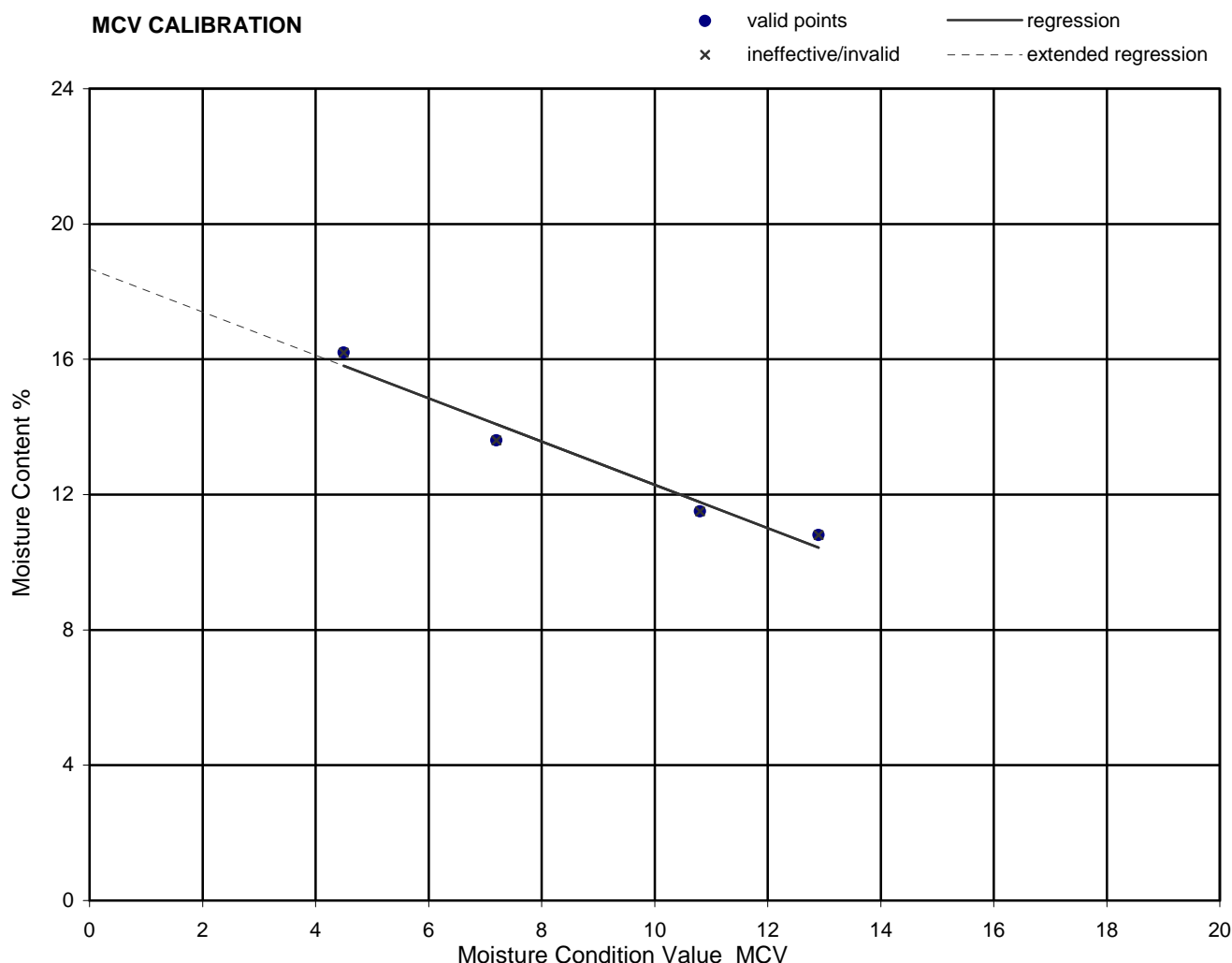
sheet 2 of 2

MOISTURE CONDITION VALUE (MCV) / MOISTURE CONTENT RELATIONSHIP

BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH2
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	6.00
			Samp No	15
			Type	B
			ID	ESGA1077-11201110100000000081
			Spec Ref	

MCV CALIBRATION



Characteristics of calibration line (determined using linear regression)

Intercept	18.7
Slope	-0.64
Sensitivity (Change in MCV per 1% moisture content)	1.56
Correlation (proximity of test points to regression line)	-0.98
Method of interpretation of MCV	Steepest straight line

The above characteristics are NOT covered by UKAS accreditation to BS1377

QA Ref

SLD 4, 5.5
Rev 72
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Figure

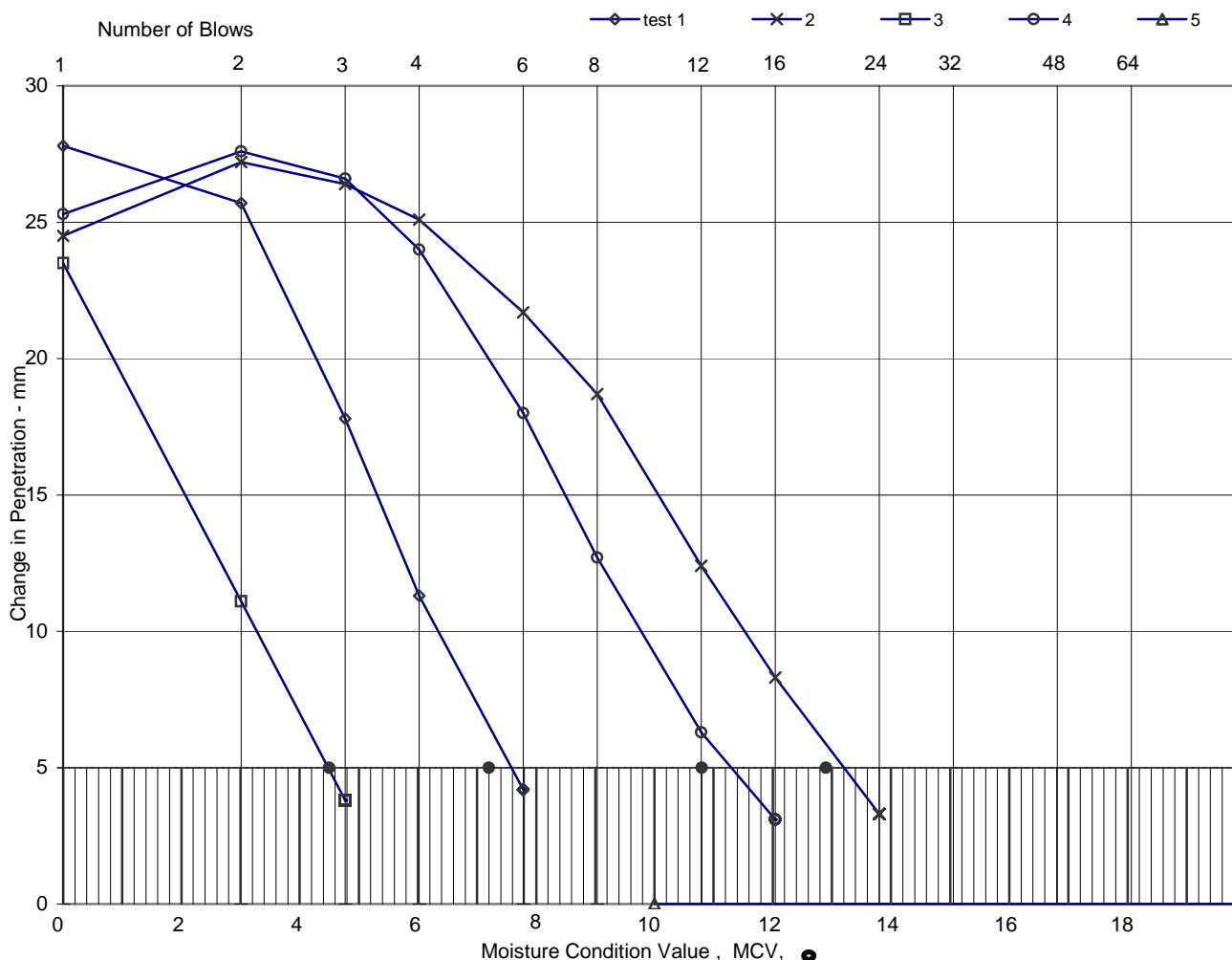
MCVREL 8

sheet 1 of 2

MOISTURE CONDITION VALUE (MCV) / MOISTURE CONTENT RELATIONSHIP

BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH2
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	6.00
			Samp No	15
			Type	B
			ID	ESGA1077-11201110100000000081
			Spec Ref	



Test No	* ineffective / invalid point	1	2	3	4	
Moisture Condition Value		7.2	12.9	4.5	10.8	
Moisture Content	%	13.6	10.8	16.2	11.5	
Bulk density after test	Mg/m ³	2.20	2.30	2.14	2.27	
Dry density after test	Mg/m ³	1.94	2.08	1.84	2.04	

Soil description	Brown slightly sandy slightly gravelly CLAY.
Procedure / Preparation	BS1377:Part 1 and Part 4, clause 5.5
Remarks	

Initial moisture content <20mm	%	13.6
Material retained on 20mm sieve	%	1.2

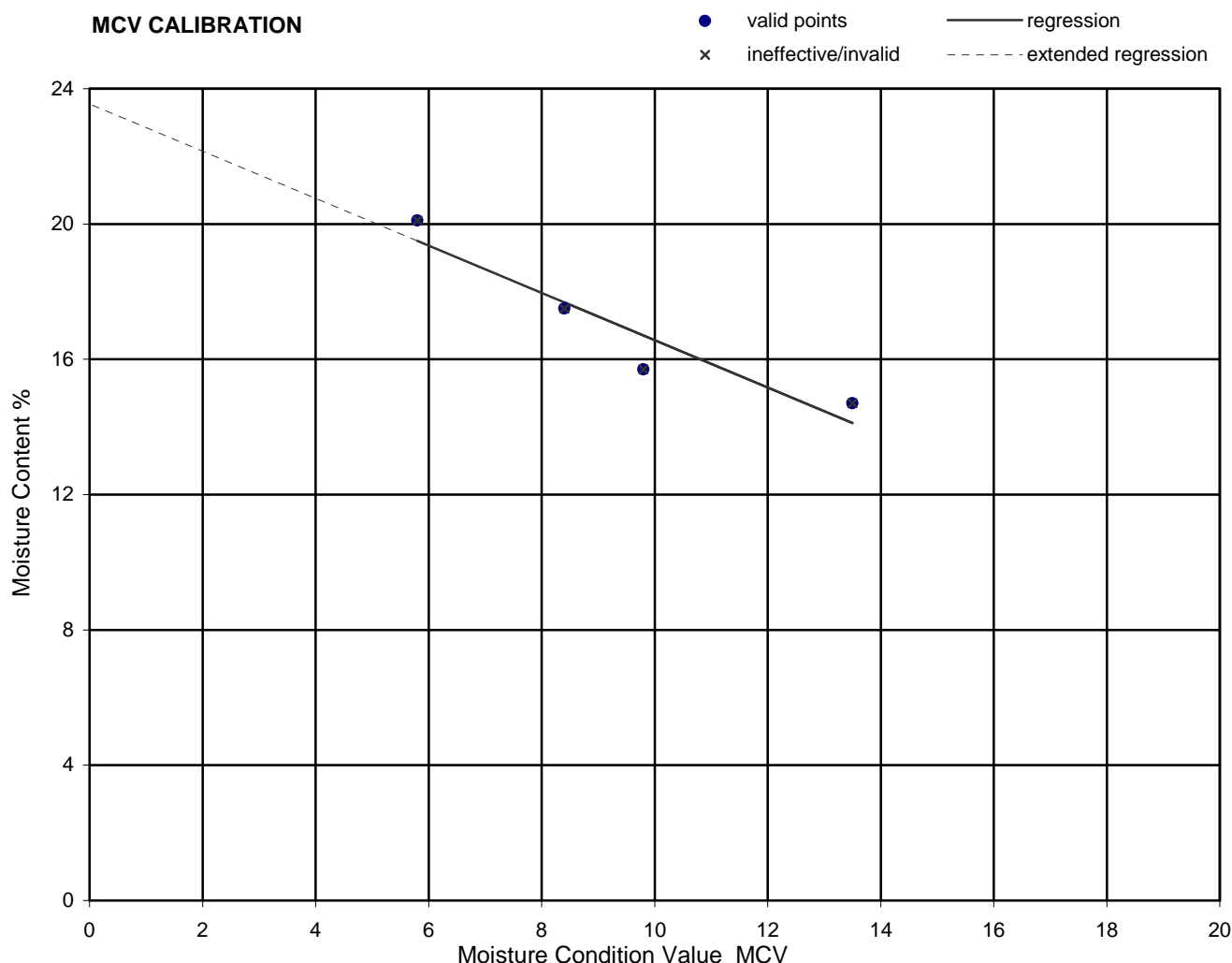
Method of determining MCV
Steepest straight line

MOISTURE CONDITION VALUE (MCV) / MOISTURE CONTENT RELATIONSHIP

BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH3
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.50
			Samp No	3
			Type	B
			ID	ESGA1077-11201110110000000128
			Spec Ref	

MCV CALIBRATION



Characteristics of calibration line (determined using linear regression)

Intercept	23.6
Slope	-0.70
Sensitivity (Change in MCV per 1% moisture content)	1.43
Correlation (proximity of test points to regression line)	-0.95
Method of interpretation of MCV	Steepest straight line

The above characteristics are NOT covered by UKAS accreditation to BS1377

QA Ref

SLD 4, 5.5
Rev 72
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Figure

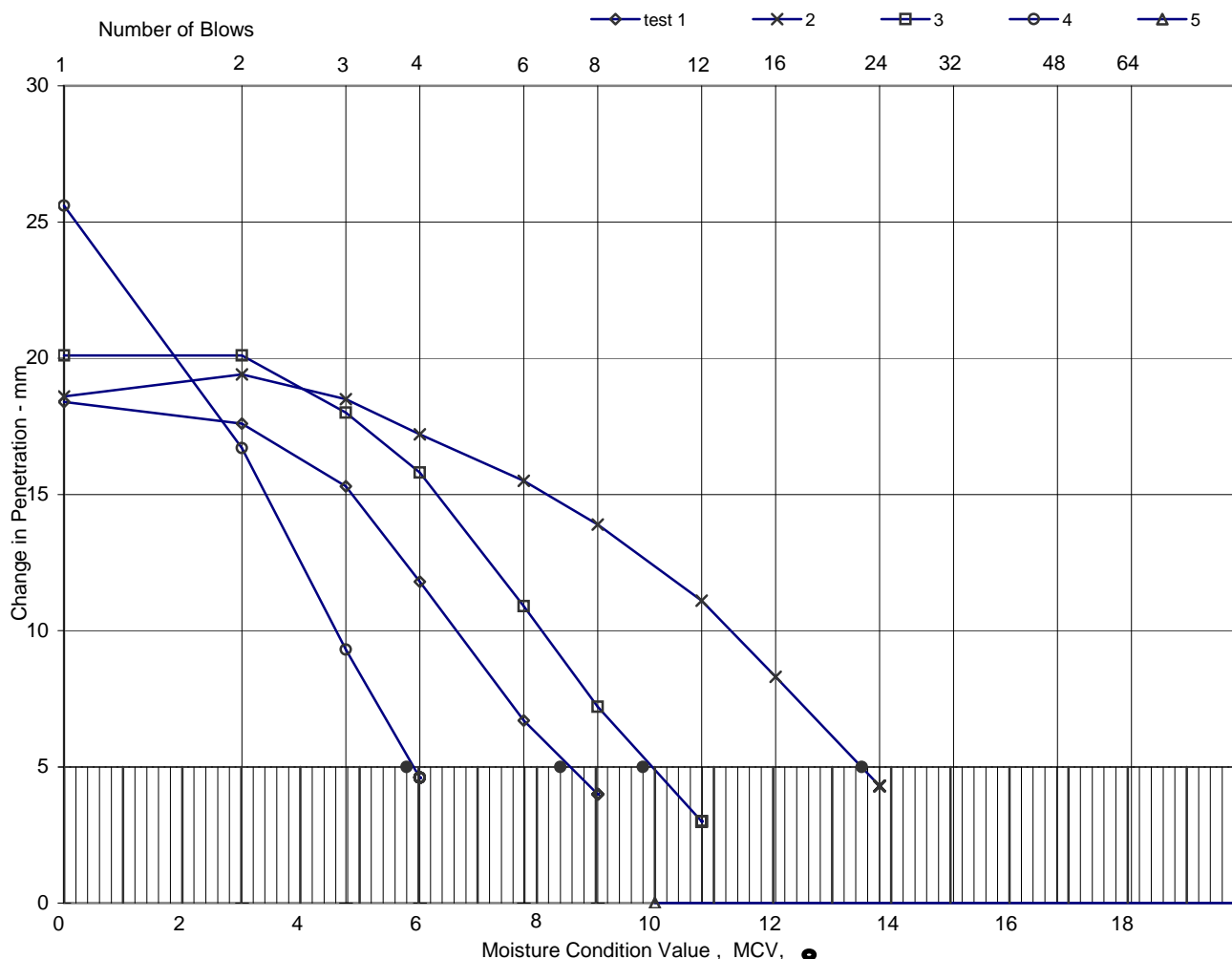
MCVREL 9

sheet 1 of 2

MOISTURE CONDITION VALUE (MCV) / MOISTURE CONTENT RELATIONSHIP

BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH3		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.50		
			Samp No	3	Type	B
			ID	ESGA1077-11201110110000000128		
			Spec Ref			



Test No	* ineffective / invalid point	1	2	3	4	
Moisture Condition Value		8.4	13.5	9.8	5.8	
Moisture Content	%	17.5	14.7	15.7	20.1	
Bulk density after test	Mg/m ³	2.05	2.16	2.10	2.02	
Dry density after test	Mg/m ³	1.74	1.88	1.82	1.68	

Soil description	Brown slightly gravelly sandy CLAY.
Procedure / Preparation	BS1377:Part 1 and Part 4, clause 5.5
Remarks	

Initial moisture content <20mm	%	17.5
Material retained on 20mm sieve	%	0

Method of determining MCV
Steepest straight line

QA Ref

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Figure

MCVREL 9

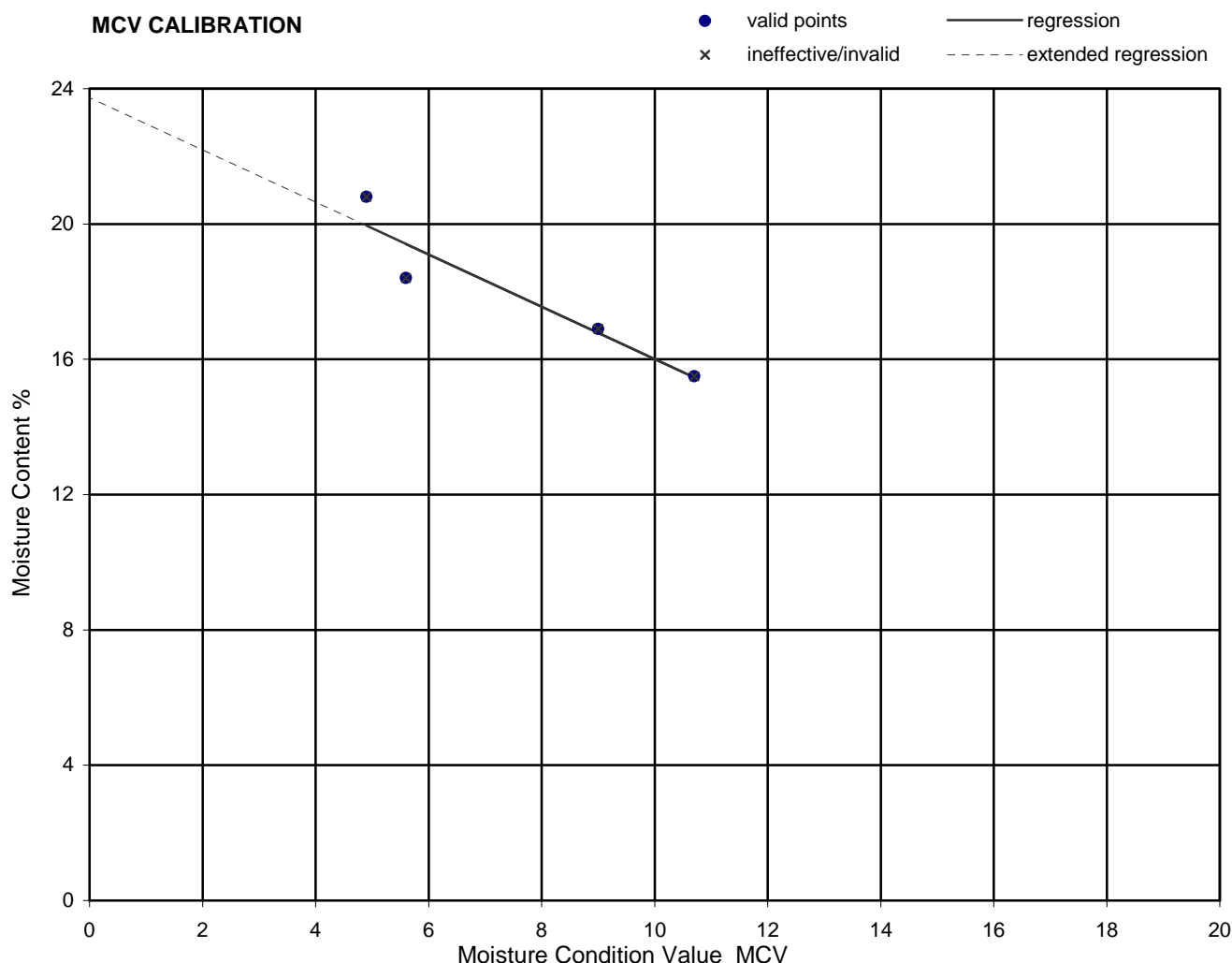
sheet 2 of 2

MOISTURE CONDITION VALUE (MCV) / MOISTURE CONTENT RELATIONSHIP

BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH3		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	1.85		
			Samp No	7	Type	B
			ID	ESGA1077-11201110110000000132		
			Spec Ref			

MCV CALIBRATION



Characteristics of calibration line (determined using linear regression)

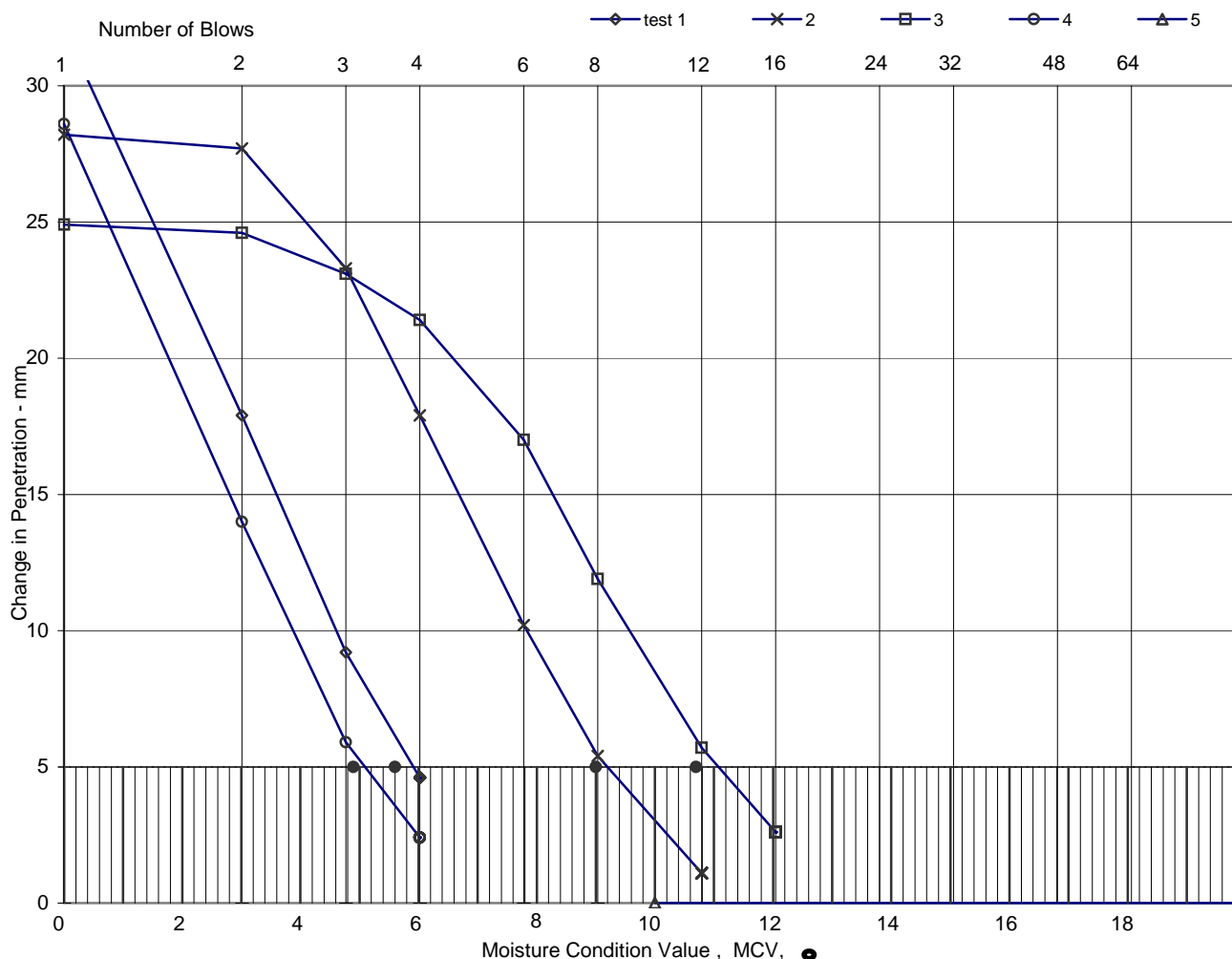
Intercept	23.7
Slope	-0.77
Sensitivity (Change in MCV per 1% moisture content)	1.29
Correlation (proximity of test points to regression line)	-0.94
Method of interpretation of MCV	Steepest straight line

The above characteristics are NOT covered by UKAS accreditation to BS1377

MOISTURE CONDITION VALUE (MCV) / MOISTURE CONTENT RELATIONSHIP

BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH3
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	1.85
			Samp No	7
			Type	B
			ID	ESGA1077-11201110110000000132
			Spec Ref	



Test No	* ineffective / invalid point	1	2	3	4	*
Moisture Condition Value		5.6	9.0	10.7	4.9	
Moisture Content	%	18.4	16.9	15.5	20.8	
Bulk density after test	Mg/m ³	2.07	2.13	2.16	2.04	
Dry density after test	Mg/m ³	1.75	1.82	1.87	1.69	

Soil description	Brown slightly sandy slightly gravelly CLAY.
Procedure / Preparation	BS1377:Part 1 and Part 4, clause 5.5
Remarks	

Initial moisture content <20mm	%	18.4
Material retained on 20mm sieve	%	1

Method of determining MCV
Steepest straight line

QA Ref

SLD 4, 5.5
Rev 72
Aug 11



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Figure

MCVREL 10

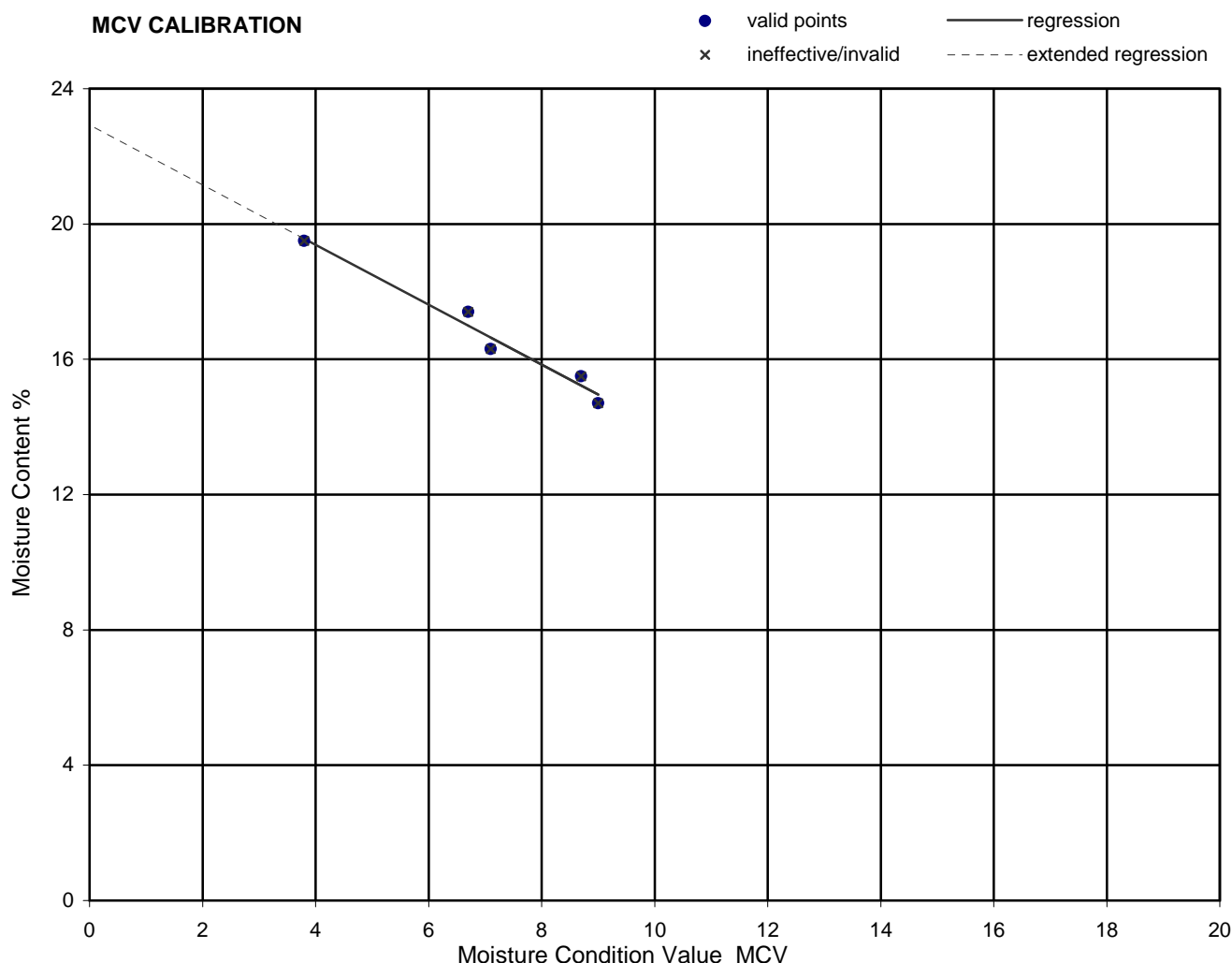
sheet 2 of 2

MOISTURE CONDITION VALUE (MCV) / MOISTURE CONTENT RELATIONSHIP

BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH3	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	3.15	
			Samp No	11	Type B
			ID	ESGA1077-11201110110000000136	
			Spec Ref		

MCV CALIBRATION



Characteristics of calibration line (determined using linear regression)

Intercept	22.9
Slope	-0.89
Sensitivity (Change in MCV per 1% moisture content)	1.13
Correlation (proximity of test points to regression line)	-0.98
Method of interpretation of MCV	Steepest straight line

The above characteristics are NOT covered by UKAS accreditation to BS1377

QA Ref

SLD 4, 5.5
Rev 72
Aug 11



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Figure

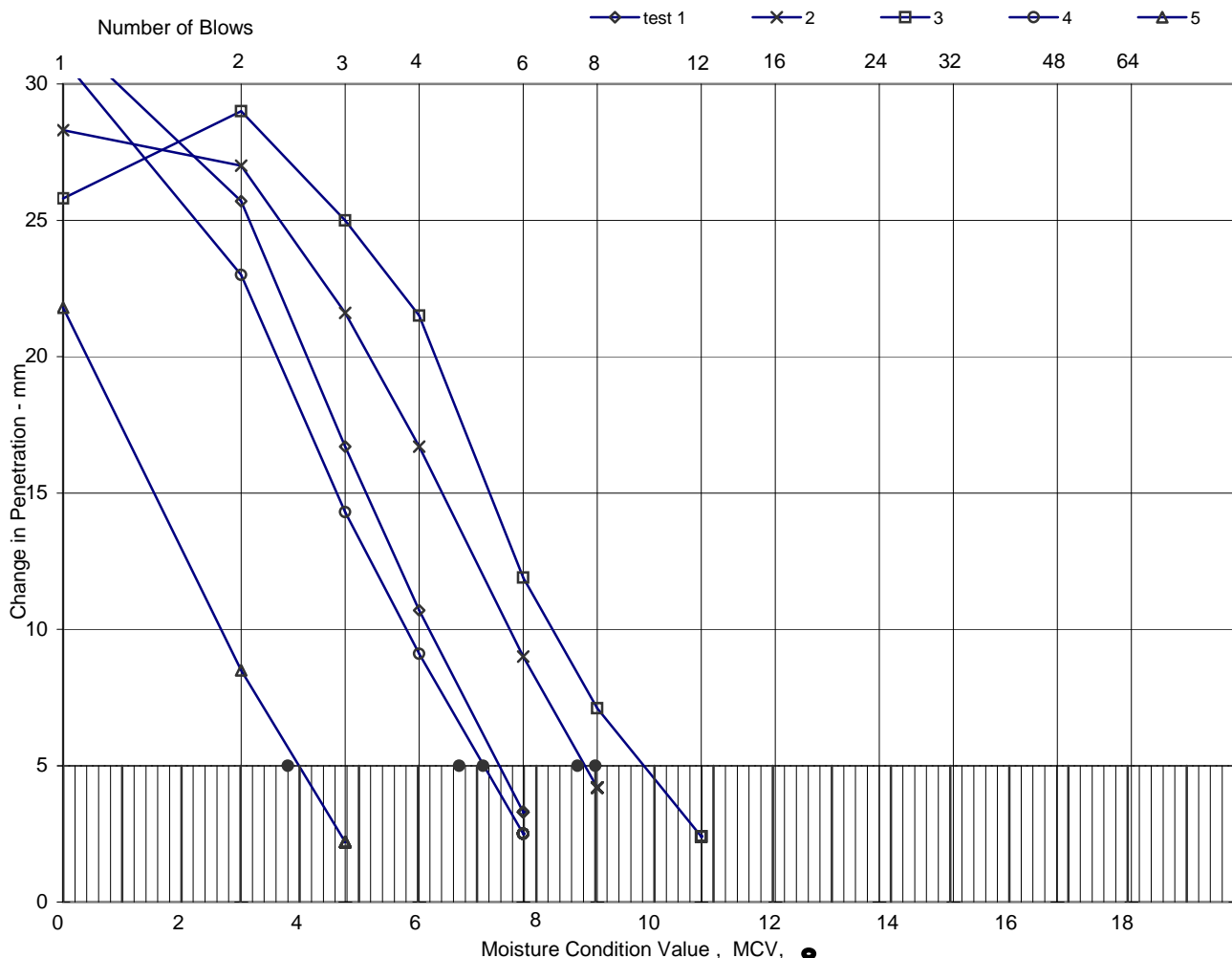
MCVREL 11

sheet 1 of 2

MOISTURE CONDITION VALUE (MCV) / MOISTURE CONTENT RELATIONSHIP

BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH3		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	3.15		
			Samp No	11	Type	B
			ID	ESGA1077-11201110110000000136		
			Spec Ref			



Test No	* ineffective / invalid point	1	2	3	4	5
Moisture Condition Value		7.1	8.7	9.0	6.7	3.8
Moisture Content	%	16.3	15.5	14.7	17.4	19.5
Bulk density after test	Mg/m ³	2.14	2.17	2.19	2.12	2.05
Dry density after test	Mg/m ³	1.84	1.88	1.91	1.81	1.72

Soil description	Reddish brown slightly sandy slightly gravelly CLAY.
Procedure / Preparation	BS1377:Part 1 and Part 4, clause 5.5
Remarks	

Initial moisture content <20mm	%	16.3
Material retained on 20mm sieve	%	0

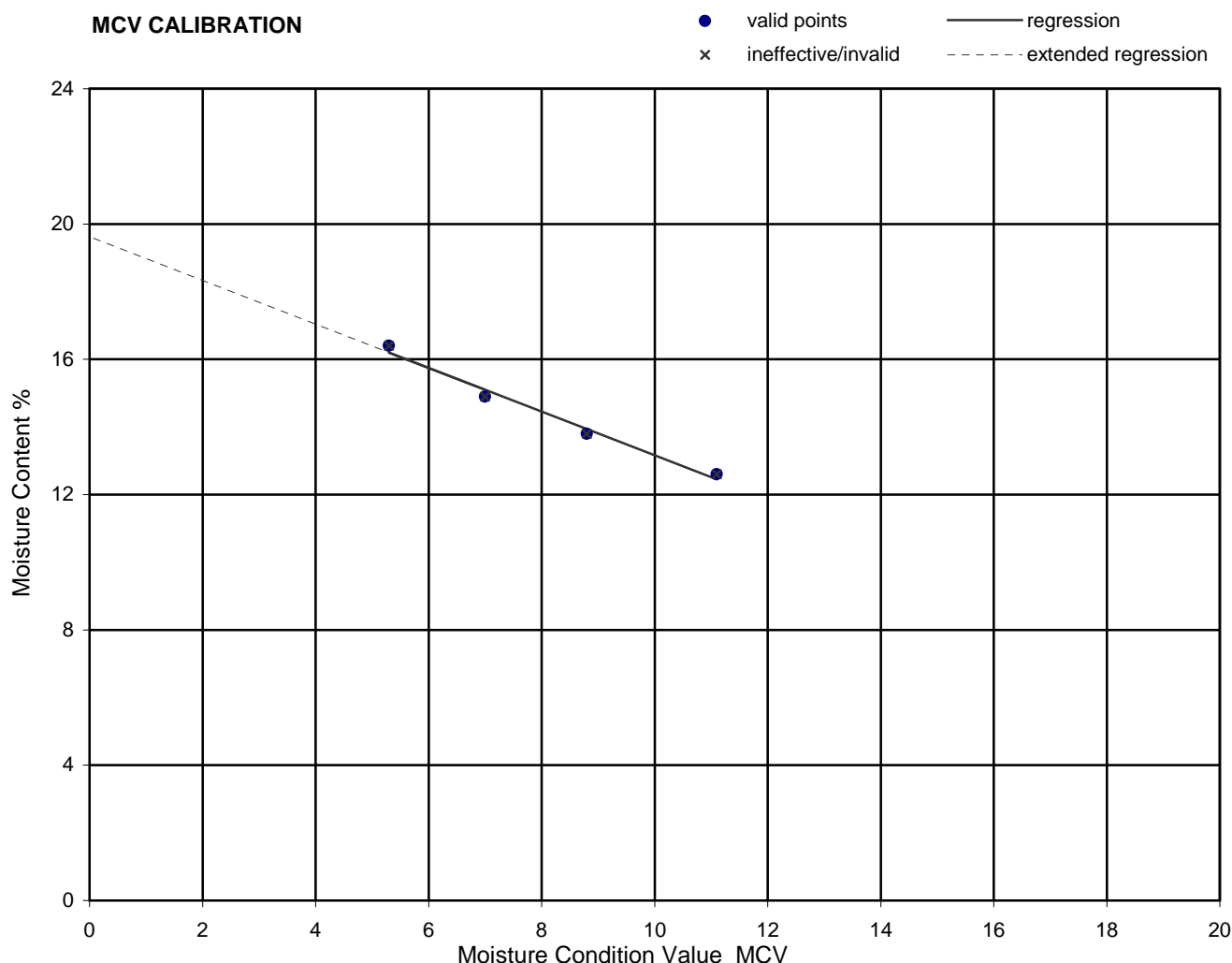
Method of determining MCV
Steepest straight line

MOISTURE CONDITION VALUE (MCV) / MOISTURE CONTENT RELATIONSHIP

BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH3
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	4.65
			Samp No	15
			Type	B
			ID	ESGA1077-11201110110000000140
			Spec Ref	

MCV CALIBRATION



Characteristics of calibration line (determined using linear regression)

Intercept	19.6
Slope	-0.65
Sensitivity (Change in MCV per 1% moisture content)	1.55
Correlation (proximity of test points to regression line)	-0.99
Method of interpretation of MCV	Steepest straight line

The above characteristics are NOT covered by UKAS accreditation to BS1377

QA Ref

SLD 4, 5.5
Rev 72
Aug 11



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Figure

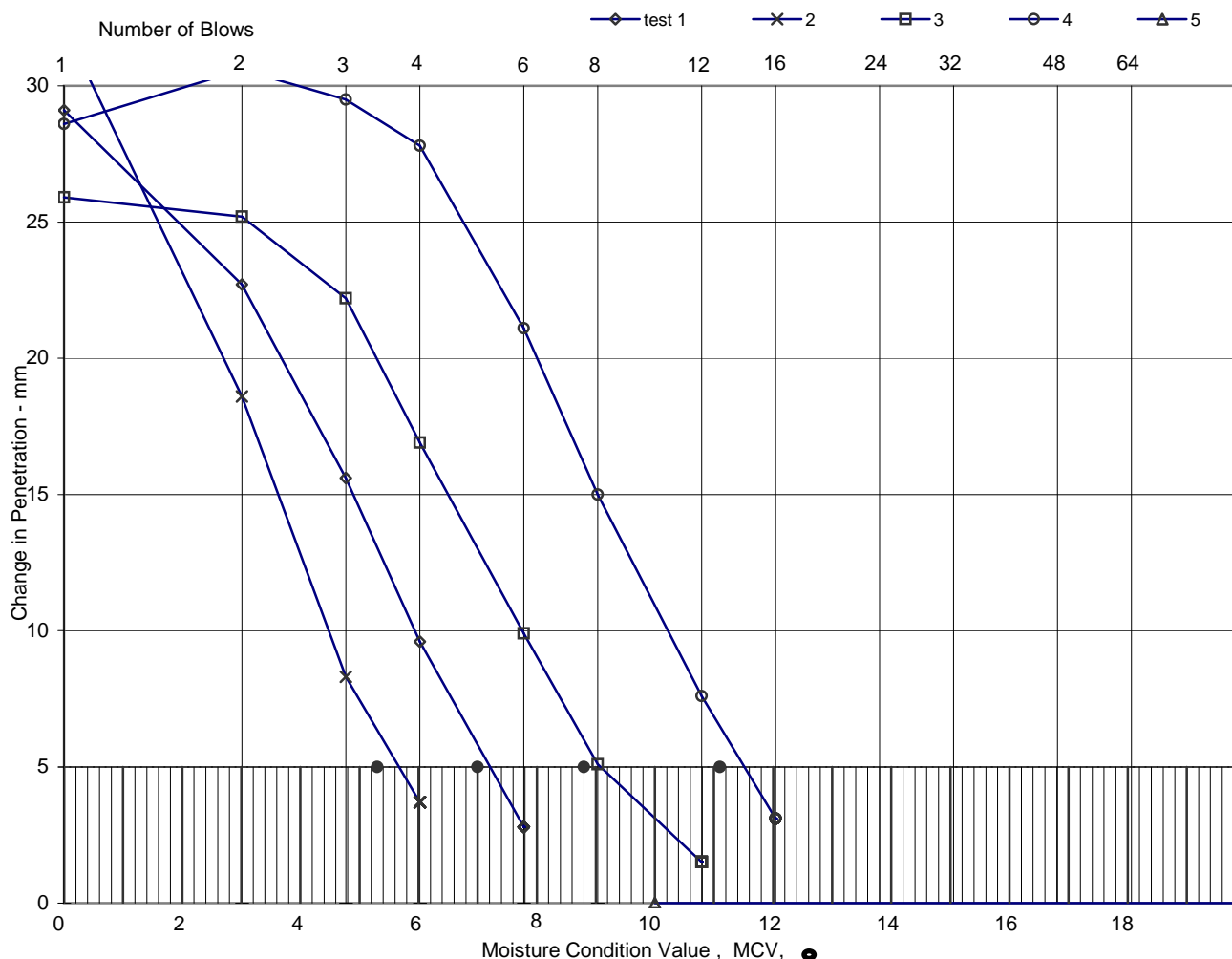
MCVREL 12

sheet 1 of 2

MOISTURE CONDITION VALUE (MCV) / MOISTURE CONTENT RELATIONSHIP

BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH3
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	4.65
			Samp No	15
			Type	B
			ID	ESGA1077-11201110110000000140
			Spec Ref	



Test No	* ineffective / invalid point	1	2	3	4	
Moisture Condition Value		7.0	5.3	8.8	11.1	
Moisture Content	%	14.9	16.4	13.8	12.6	
Bulk density after test	Mg/m ³	2.20	2.15	2.24	2.26	
Dry density after test	Mg/m ³	1.91	1.85	1.97	2.01	

Soil description	Brown slightly sandy slightly gravelly CLAY.
Procedure / Preparation	BS1377:Part 1 and Part 4, clause 5.5
Remarks	

Initial moisture content <20mm	%	14.9
Material retained on 20mm sieve	%	1

Method of determining MCV
Steepest straight line

QA Ref

SLD 4, 5.5
Rev 72
Aug 11



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Figure

MCVREL 12

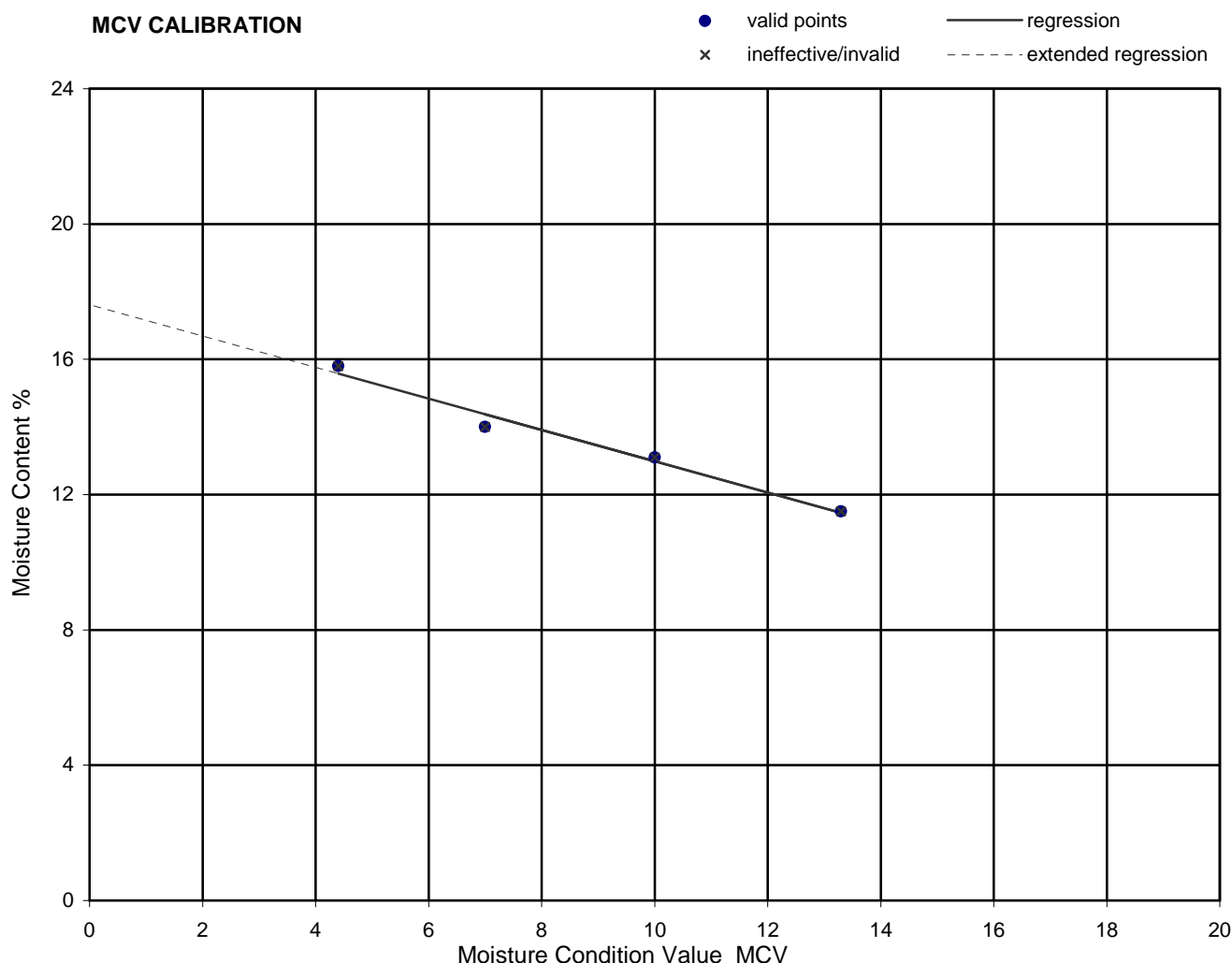
sheet 2 of 2

MOISTURE CONDITION VALUE (MCV) / MOISTURE CONTENT RELATIONSHIP

BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH3
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	6.15
			Samp No	19
			Type	B
			ID	ESGA1077-11201110110000000144
			Spec Ref	

MCV CALIBRATION



Characteristics of calibration line (determined using linear regression)

Intercept	17.6
Slope	-0.46
Sensitivity (Change in MCV per 1% moisture content)	2.16
Correlation (proximity of test points to regression line)	-0.99
Method of interpretation of MCV	Steepest straight line

The above characteristics are NOT covered by UKAS accreditation to BS1377

QA Ref

SLD 4, 5.5
Rev 72
Aug 11



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Figure

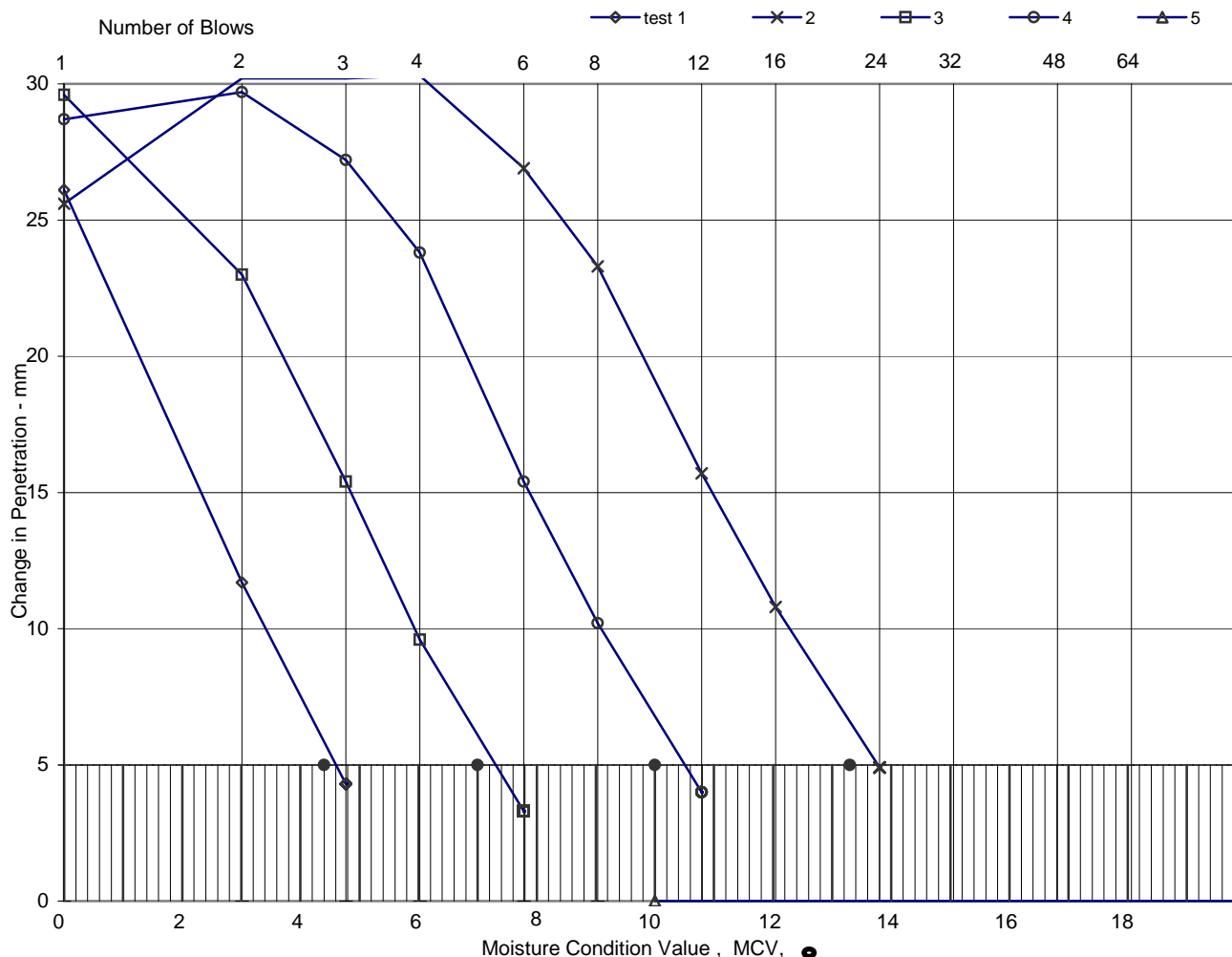
MCVREL 13

sheet 1 of 2

MOISTURE CONDITION VALUE (MCV) / MOISTURE CONTENT RELATIONSHIP

BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH3
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	6.15
			Samp No	19
			Type	B
			ID	ESGA1077-11201110110000000144
			Spec Ref	



Test No	* ineffective / invalid point	1	2	3	4	
Moisture Condition Value		4.4	13.3	7.0	10.0	
Moisture Content	%	15.8	11.5	14.0	13.1	
Bulk density after test	Mg/m ³	2.12	2.31	2.19	2.26	
Dry density after test	Mg/m ³	1.83	2.07	1.92	2.00	

Soil description	Brown slightly sandy slightly gravelly CLAY.
Procedure / Preparation	BS1377:Part 1 and Part 4, clause 5.5
Remarks	

Initial moisture content <20mm	%	15.8
Material retained on 20mm sieve	%	1.3

Method of determining MCV
Steepest straight line

QA Ref

SLD 4, 5.5
Rev 72
Aug 11



Printed:18/11/2011 17:35

Figure

MCVREL 13

sheet 2 of 2

CHEMICAL TESTS - SUMMARY OF RESULTS

Project No	Project Name
A1077-11	SANDESEND BOREHOLES, NORTH YORKSHIRE

Hole No.	Sample				Soil Description	Org %	LOI %	pH	Sulphate as SO ₄				SD1 options			CO ₂ %	Chloride, Cl		<2 mm %	Remarks
	No.	Depth (m)		type					Preparation/test *	2:1 water sol. g/L	ground water g/L	acid sol. %	TS %	Mg NO ₃ NH ₄ mg/L mg/L	water sol. %		acid sol. %			
		from	to																	
BH1	6	1.65	1.85	D	Brown slightly sandy slightly gravelly CLAY.			7.9	1+3	0.20								93		
BH1	22	8.45	8.65	D	Brown slightly gravelly CLAY.			7.9	1+3	0.20								88		
BH1	42	15.95	16.15	D	Brown slightly sandy gravelly CLAY.			7.5	1+3	0.25								57		
BH1	60	22.50	23.95	U	Stiff greyish brown slightly sandy slightly gravelly CLAY.			7.9	1+3	0.26								91		
BH2	8	3.00	3.45	U	Stiff greyish brown slightly sandy slightly gravelly CLAY.			8.0	1+3	0.14								89		
BH2	25	11.45	11.65	D	Brown slightly sandy slightly gravelly CLAY.			7.8	1+3	0.33								91		
BH2	36	15.90		W				7.7	2+3		0.86									
BH2	38	17.45	17.65	D	Brown slightly sandy slightly gravelly CLAY.			7.6	1+3	0.22								98		
BH2	39	17.90		W				7.7	2+3		0.68									
BH2	54	24.45	24.65	D	Brown slightly sandy slightly gravelly CLAY.			7.6	1+3	0.19								93		
BH3	1	0.10		D	Brown slightly gravelly SILT with occasional rootlets.			5.7	1+3	0.12								96		
BH3	9	2.95	3.95	D	Brown slightly sandy slightly gravelly CLAY.			7.9	1+3	0.16								97		
BH3	22	7.65	8.10	D	Brown slightly sandy slightly gravelly CLAY.			7.8	1+3	0.35								91		
BH3	35	12.50	12.95	D	Greyish brown slightly sandy CLAY.			7.9	1+3	0.06								100		
BH3	48	17.95	18.15	D	Greyish brown slightly sandy slightly gravelly CLAY.			7.7	1+3	0.21								94		
BH4	4	0.20	0.30	B	Light grey sandy gravelly SILT.			8.7	1+3	0.98								56		
BH4	8	0.80	1.10	B	Brown slightly gravelly sandy CLAY.			7.6	1+3	0.33								69		
BH4	21	4.50		D	Dark grey slightly gravelly CLAY. Gravel is mainly weak mudstone.			8.1	1+3	1.19								100		
BH5	8	1.20	1.65	D	Grey very gravelly slightly clayey SAND.			7.1	1+3	0.41								63		

BS 1377 : definitive method unless stated :	* Sulphate tests preparation / test methods :	BRE Special Digest SD1, dependent options :
Org Organic matter content (s-sulphides, c-chlorides identified)	1. BS 1377:Part 3:1990:clause 5.3	4. TRL447 - 1 water soluble sulphate
LOI Mass loss on ignition at 440°C	2. BS 1377:Part 3:1990:clause 5.4	5. TRL447 - 2 acid soluble sulphate
CO ₂ Carbonate content (rapid titration)	3. BS 1377:Part 3:1990:clause 5.5	6. BR279 - groundwater sulphate
Cl Chloride content	< 2mm material passing 2mm sieve	TS Total Sulphur to BR279 / EN ISO15178
		Mg Soluble Magnesium to BR279, colorimetric
		NO ₃ Soluble Nitrate to BR279, colorimetric
		NH ₄ qualitative

QA Ref	ESG Environmental Scientifics Group	Printed:01/12/2011 09:37	Table CHEM 1
SLR 3 Rev 95 Aug 11			

CHEMICAL TESTS - SUMMARY OF RESULTS

Project No	Project Name
A1077-11	SANDESEND BOREHOLES, NORTH YORKSHIRE

Hole No.	Sample				Soil Description	Org	LOI	pH	Sulphate as SO ₄				SD1 options			CO ₂	Chloride, Cl		<2 mm	Remarks
	No.	Depth (m)		type					Preparation/test *	2:1 water sol.	ground water	acid sol.	TS	Mg NO ₃ NH ₄	mg/L mg/L		water sol.	acid sol.		
		from	to																	
WS1	2	0.30	1.00	B	Brown SAND.			8.0	1+3	0.22								100		
WS2	2	0.30	1.00	B	Brownish grey gravelly SAND.			8.1	1+3	0.20								92		
WS2	7	2.80		D	Grey sandy clayey SILT.			7.3	1+3	0.26								100		
WS3	2	0.30	1.00	B	Brown gravelly SAND.			7.2	1+3	0.14								87		
WS4	2	0.40	0.90	B	Grey and brown very gravelly SAND.			7.6	1+3	0.15								68		
WS5	4	1.70		D	Brown slightly sandy slightly gravelly CLAY.			7.6	1+3	0.19								93		

BS 1377 : definitive method unless stated : * Sulphate tests preparation / test methods : BRE Special Digest SD1, dependent options :

Org Organic matter content 1. BS 1377:Part 3:1990:clause 5.3 4. TRL447 - 1 water soluble sulphate TS Total Sulphur to BR279 / EN ISO15178

(s-sulphides, c-chlorides identified) 2. BS 1377:Part 3:1990:clause 5.4 5. TRL447 - 2 acid soluble sulphate Mg Soluble Magnesium to BR279, colorimetric

LOI Mass loss on ignition at 440°C 3. BS 1377:Part 3:1990:clause 5.5 6. BR279 - groundwater sulphate NO3 Soluble Nitrate to BR279, colorimetric

CO₂ Carbonate content (rapid titration) NH₄ qualitative

Cl Chloride content < 2mm material passing 2mm sieve

QA Ref		Printed:01/12/2011 09:37	Table CHEM 2
SLR 3 Rev 95 Aug 11			

ENCLOSURE D
GEOENVIRONMENTAL LABORATORY TEST RESULTS

ESG Scientifics Reports Nos

EFS/119196
EXR/127351

TEST REPORT

SOIL SAMPLE ANALYSIS



Report No. EFS/119196 (Ver. 1)

Soil Mechanics
Askern Road
Carcroft
Doncaster
South Yorkshire
DN6 8DG

Site: Sandsend Boreholes, North Yorkshire

The 4 samples described in this report were registered for analysis by ESG on 01-Nov-2011. This report supersedes any versions previously issued by the laboratory.

The analysis was completed by: 14-Nov-2011

Tests where the accreditation is set to N or No, and any individual data items marked with a * are not UKAS accredited
Any opinions or interpretations expressed herein are outside the scope of any UKAS accreditation held by ESG.

The following tables are contained in this report:

Table 1 Main Analysis Results (Pages 2 to 4)
Table of PAH (MS-SIM) (80) Results (Pages 5 to 7)
Table of PCB Congener Results (Page 8)
Table of TPH Texas banding (std) (Page 9)
GC-FID Chromatograms (Pages 10 to 12)
Table of Asbestos Screening Results (Page 13)
Analytical and Deviating Sample Overview (Pages 14 to 15)
Table of Method Descriptions (Page 16)
Table of Report Notes (Page 17)

On behalf of
ESG :
Andrew Timms

A handwritten signature in black ink, appearing to read 'Andrew Timms'.

Operations Manager


Date of Issue: 14-Nov-2011

Tests marked 'A' have been subcontracted to another laboratory.

ESG accepts no responsibility for any sampling not carried out by our personnel.

[illegible]

[illegible]

Units :		µg/kg	µg/kg	mg/kg																
Method Codes :		BTEXHSA	BTEXHSA	PAHMSUS																
Method Reporting Limits :		10	10																	
UKAS Accredited :		Yes	Yes	Yes																
LAB ID Number CL/	Client Sample Description	m/p Xylenes	o Xylene	PAH (16) by GCMS																
					 <p>Bretby Business Park, Ashby Road</p> <p>Burton-on-Trent, Staffordshire, DE15 0YZ</p> <p>Tel +44 (0) 1283 554400</p> <p>Fax +44 (0) 1283 554422</p>		Client Name	Soil Mechanics							Soil Sample Analysis					
					Contact	Mr B Swallow														
Sandsend Boreholes, North Yorkshire							Date Printed	14-Nov-2011												
							Report Number	EFS/119196												
							Table Number	1												

Polycyclic Aromatic Hydrocarbons GC/MS (SIM)

Customer and Site Details:	Soil Mechanics: Sandsend Boreholes, North Yorkshire		
Sample Details:	BH1 ES 4A 1.00	Job Number:	S11_9196
LIMS ID Number:	CL1142188	Date Booked in:	01-Nov-11
QC Batch Number:	112027	Date Extracted:	11-Nov-11
Quantitation File:	Initial Calibration	Date Analysed:	14-Nov-11
Directory:	1411PAH.MS5\	Matrix:	Soil
Dilution:	1.0	Ext Method:	Ultrasonic

UKAS accredited?: Yes

Target Compounds	CAS #	R.T. (min)	Concentration mg/kg	% Fit
Naphthalene	91-20-3	-	< 0.08	-
Acenaphthylene	208-96-8	-	< 0.08	-
Acenaphthene	83-32-9	-	< 0.08	-
Fluorene	86-73-7	-	< 0.08	-
Phenanthrene	85-01-8	-	< 0.08	-
Anthracene	120-12-7	-	< 0.08	-
Fluoranthene	206-44-0	-	< 0.08	-
Pyrene	129-00-0	-	< 0.08	-
Benzo[a]anthracene	56-55-3	-	< 0.08	-
Chrysene	218-01-9	-	< 0.08	-
Benzo[b]fluoranthene	205-99-2	-	< 0.08	-
Benzo[k]fluoranthene	207-08-9	-	< 0.08	-
Benzo[a]pyrene	50-32-8	-	< 0.08	-
Indeno[1,2,3-cd]pyrene	193-39-5	-	< 0.08	-
Dibenzo[a,h]anthracene	53-70-3	-	< 0.08	-
Benzo[g,h,i]perylene	191-24-2	-	< 0.08	-
Total (USEPA16) PAHs	-	-	< 1.28	-

"M" denotes that % fit has been manually interpreted

Internal Standards	% Area
1,4-Dichlorobenzene-d4	NA
Naphthalene-d8	122
Acenaphthene-d10	122
Phenanthrene-d10	139
Chrysene-d12	148
Perylene-d12	150

Surrogates	% Rec
Nitrobenzene-d5	NA
2-Fluorobiphenyl	96
Terphenyl-d14	104

Concentrations are reported on a wet weight basis.

The Total PAH result is the sum of non-rounded individual PAH results and therefore may differ to the sum of the rounded individual PAH results printed above. By convention, where any one or more result is a "less than", the total is expressed as a "less than" and includes the "less than" concentration within the total.

Polycyclic Aromatic Hydrocarbons GC/MS (SIM)

Customer and Site Details:	Soil Mechanics: Sandsend Boreholes, North Yorkshire		
Sample Details:	BH3 ES 2A 0.30	Job Number:	S11_9196
LIMS ID Number:	CL1142189	Date Booked in:	01-Nov-11
QC Batch Number:	112027	Date Extracted:	11-Nov-11
Quantitation File:	Initial Calibration	Date Analysed:	14-Nov-11
Directory:	1411PAH.MS5\	Matrix:	Soil
Dilution:	1.0	Ext Method:	Ultrasonic

UKAS accredited?: Yes

Target Compounds	CAS #	R.T. (min)	Concentration mg/kg	% Fit
Naphthalene	91-20-3	-	< 0.08	-
Acenaphthylene	208-96-8	-	< 0.08	-
Acenaphthene	83-32-9	-	< 0.08	-
Fluorene	86-73-7	-	< 0.08	-
Phenanthrene	85-01-8	6.03	0.13	100
Anthracene	120-12-7	-	< 0.08	-
Fluoranthene	206-44-0	7.43	0.33	93
Pyrene	129-00-0	7.73	0.35	M
Benzo[a]anthracene	56-55-3	9.45	0.22	91
Chrysene	218-01-9	9.50	0.18	97
Benzo[b]fluoranthene	205-99-2	11.00	0.30	69
Benzo[k]fluoranthene	207-08-9	11.03	0.10	69
Benzo[a]pyrene	50-32-8	11.44	0.25	98
Indeno[1,2,3-cd]pyrene	193-39-5	12.85	0.16	97
Dibenzo[a,h]anthracene	53-70-3	-	< 0.08	-
Benzo[g,h,i]perylene	191-24-2	13.20	0.12	90
Total (USEPA16) PAHs	-	-	< 2.62	-

"M" denotes that % fit has been manually interpreted

Internal Standards	% Area
1,4-Dichlorobenzene-d4	NA
Naphthalene-d8	119
Acenaphthene-d10	119
Phenanthrene-d10	131
Chrysene-d12	137
Perylene-d12	140

Surrogates	% Rec
Nitrobenzene-d5	NA
2-Fluorobiphenyl	93
Terphenyl-d14	100

Concentrations are reported on a wet weight basis.

The Total PAH result is the sum of non-rounded individual PAH results and therefore may differ to the sum of the rounded individual PAH results printed above. By convention, where any one or more result is a "less than", the total is expressed as a "less than" and includes the "less than" concentration within the total.

Polycyclic Aromatic Hydrocarbons GC/MS (SIM)

Customer and Site Details:	Soil Mechanics: Sandsend Boreholes, North Yorkshire		
Sample Details:	BH3 ES 7A 2.00	Job Number:	S11_9196
LIMS ID Number:	CL1142190	Date Booked in:	01-Nov-11
QC Batch Number:	112027	Date Extracted:	11-Nov-11
Quantitation File:	Initial Calibration	Date Analysed:	14-Nov-11
Directory:	1411PAH.MS5\	Matrix:	Soil
Dilution:	1.0	Ext Method:	Ultrasonic

UKAS accredited?: Yes

Target Compounds	CAS #	R.T. (min)	Concentration mg/kg	% Fit
Naphthalene	91-20-3	-	< 0.08	-
Acenaphthylene	208-96-8	-	< 0.08	-
Acenaphthene	83-32-9	-	< 0.08	-
Fluorene	86-73-7	-	< 0.08	-
Phenanthrene	85-01-8	-	< 0.08	-
Anthracene	120-12-7	-	< 0.08	-
Fluoranthene	206-44-0	-	< 0.08	-
Pyrene	129-00-0	-	< 0.08	-
Benzo[a]anthracene	56-55-3	-	< 0.08	-
Chrysene	218-01-9	-	< 0.08	-
Benzo[b]fluoranthene	205-99-2	-	< 0.08	-
Benzo[k]fluoranthene	207-08-9	-	< 0.08	-
Benzo[a]pyrene	50-32-8	-	< 0.08	-
Indeno[1,2,3-cd]pyrene	193-39-5	-	< 0.08	-
Dibenzo[a,h]anthracene	53-70-3	-	< 0.08	-
Benzo[g,h,i]perylene	191-24-2	-	< 0.08	-
Total (USEPA16) PAHs	-	-	< 1.28	-

"M" denotes that % fit has been manually interpreted

Internal Standards	% Area
1,4-Dichlorobenzene-d4	NA
Naphthalene-d8	119
Acenaphthene-d10	120
Phenanthrene-d10	133
Chrysene-d12	139
Perylene-d12	140

Surrogates	% Rec
Nitrobenzene-d5	NA
2-Fluorobiphenyl	95
Terphenyl-d14	103

Concentrations are reported on a wet weight basis.

The Total PAH result is the sum of non-rounded individual PAH results and therefore may differ to the sum of the rounded individual PAH results printed above. By convention, where any one or more result is a "less than", the total is expressed as a "less than" and includes the "less than" concentration within the total.

Polychlorinated Biphenyls (congeners)

Customer and Site Details:	Soil Mechanics: Sandsend Boreholes, North Yorkshire
Job Number:	S11_9196
QC Batch Number:	111732
Directory:	1114PCB.GC8
Method:	Ultrasonic

Matrix: SOIL
Date Booked in: 01-Nov-11
Date Extracted: 12-Nov-11
Date Analysed: 14-Nov-11

*** This sample data is not UKAS accredited.**

[illegible]

Total Petroleum Hydrocarbons (TPH) Carbon Ranges

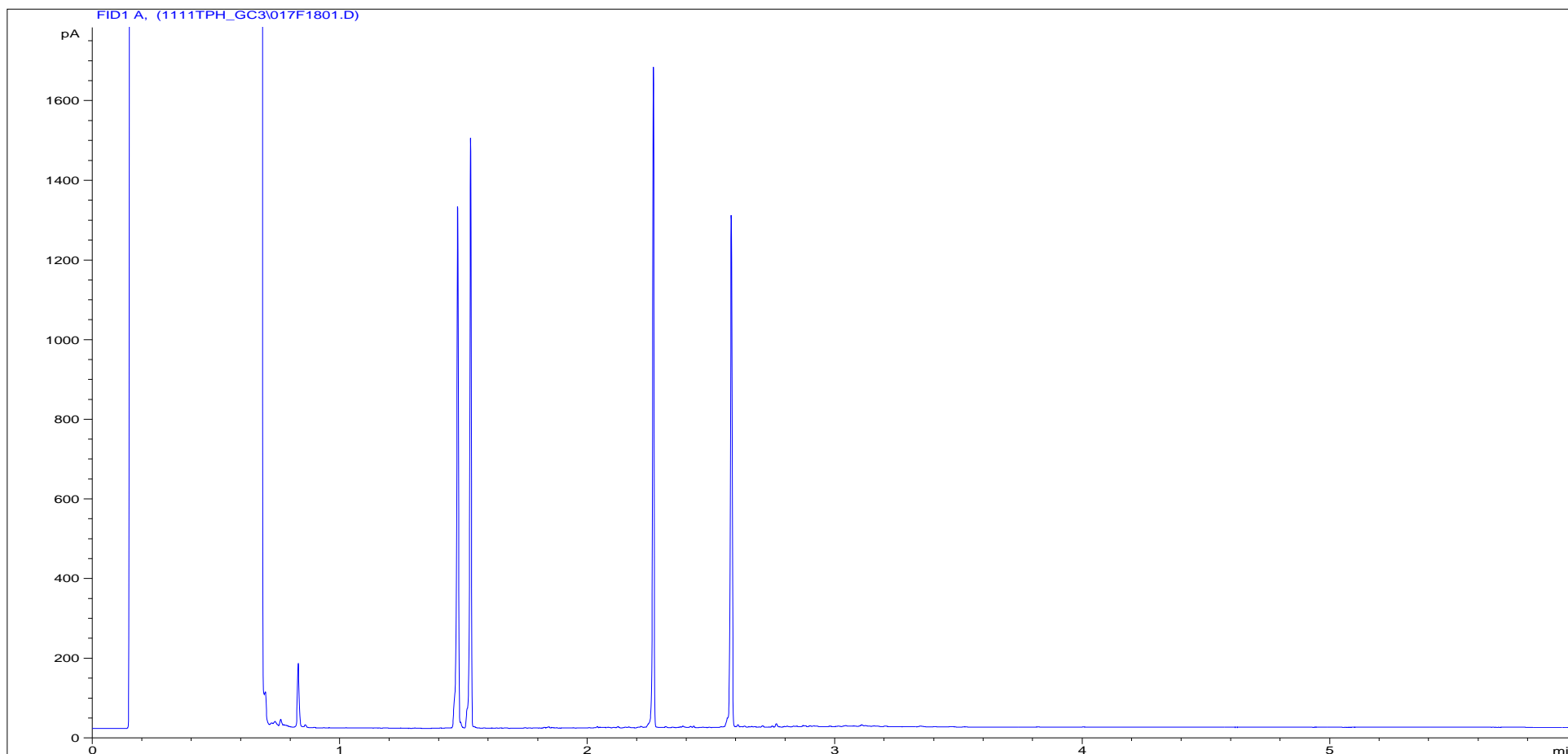
Customer and Site Details: Soil Mechanics : Sandsend Boreholes, North Yorkshire
Job Number: s11_9196
QC Batch Number: 2027
Directory: D:\TES\DATA\Y2011\NOVEMBER2011\1111TPH_GC3\019F2001.D
Method: Ultra Sonic

Matrix:	Soil
Date Booked in:	01-Nov-11
Date Extracted:	11-Nov-11
Date Analysed:	11-Nov-11

*** Sample data with an asterisk are not UKAS accredited.**

[illegible]

Petroleum Hydrocarbons (C8 to C40) by GC/FID

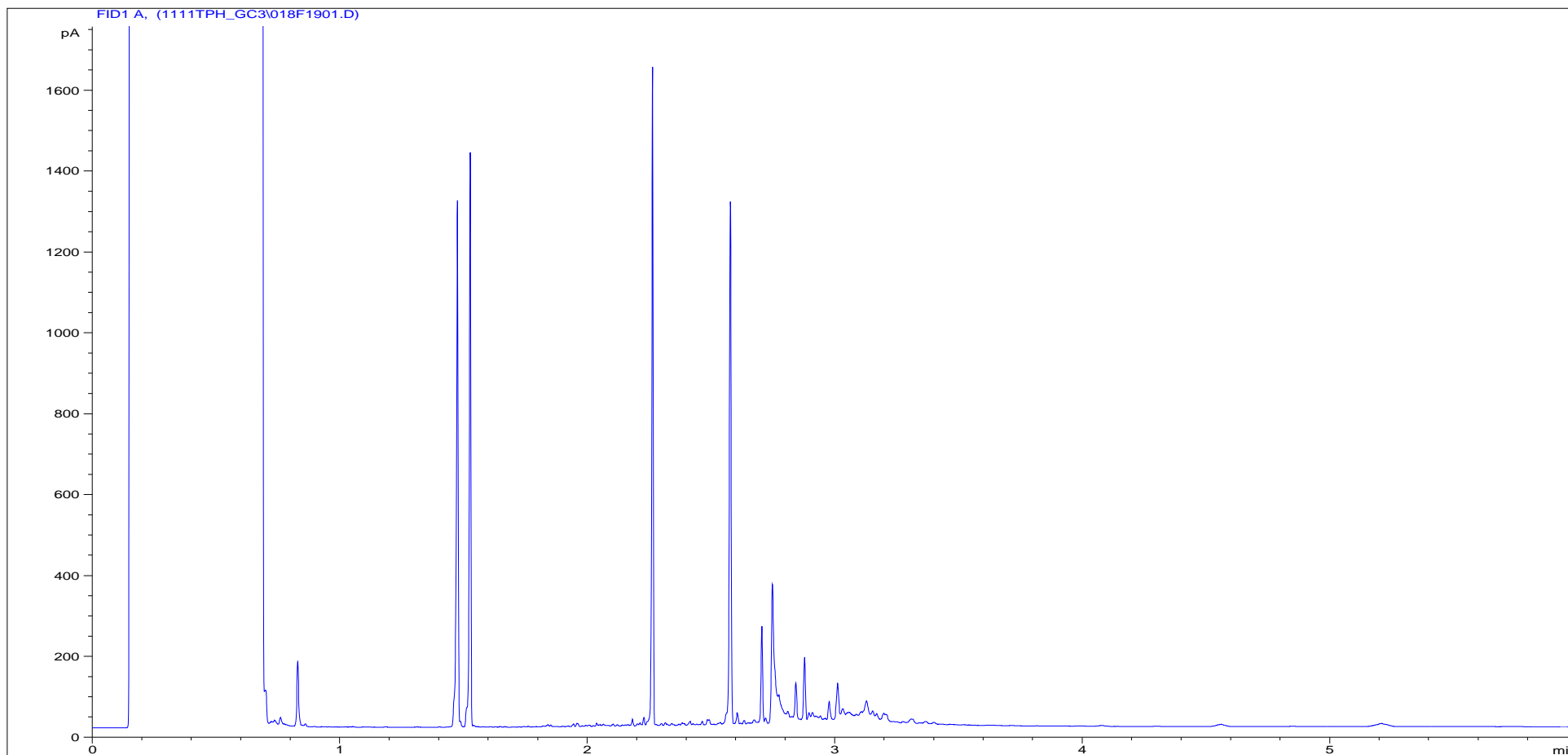


Sample ID: CL1142188
Multiplier: 8
Dilution: 1
Acquisition Method: 5UL_RUNF.M
Acquisition Date/Time: 11-Nov-11
Datafile: D:\TES\DATA\Y2011\NOVEMBER2011\1111TPH_GC3\017F1801.D

Job Number: s11_9196
Client: Soil Mechanics
Site: Sandsend Boreholes, North Yorkshire
Client Sample Ref: BH1 ES 4A 1.00

Where individual results are flagged see report notes for status.

Petroleum Hydrocarbons (C8 to C40) by GC/FID

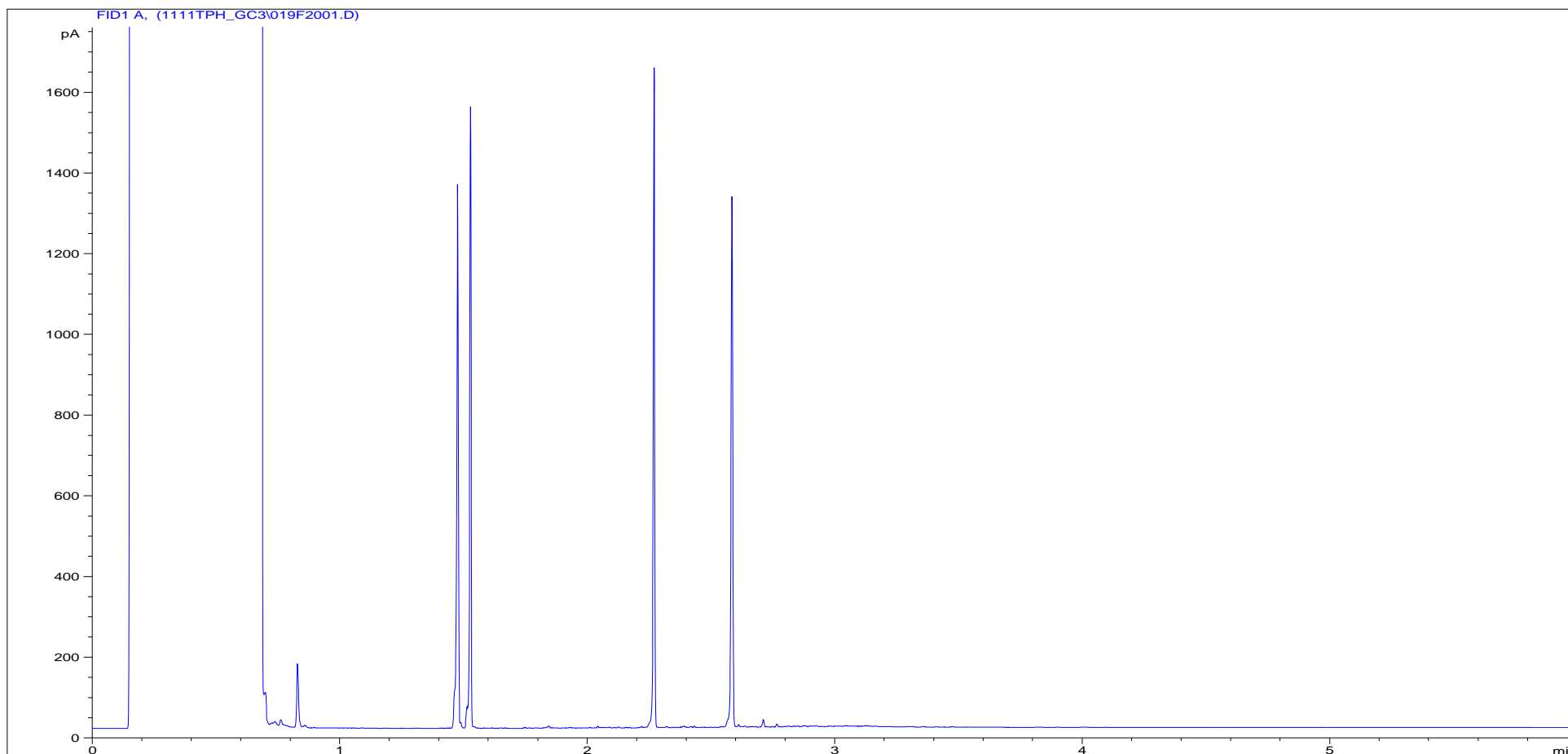


Sample ID: CL1142189
Multiplier: 8
Dilution: 1
Acquisition Method: 5UL_RUNF.M
Acquisition Date/Time: 11-Nov-11
Datafile: D:\TES\DATA\Y2011\NOVEMBER2011\1111TPH_GC3\018F1901.D

Job Number: s11_9196
Client: Soil Mechanics
Site: Sandsend Boreholes, North Yorkshire
Client Sample Ref: BH3 ES 2A 0.30

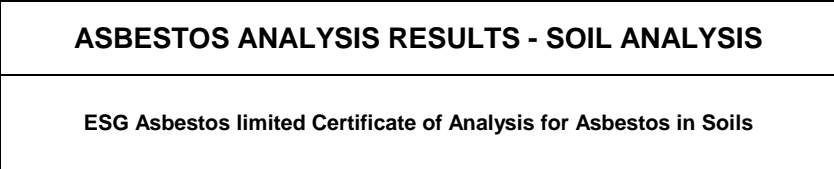
Where individual results are flagged see report notes for status.

Petroleum Hydrocarbons (C8 to C40) by GC/FID



Sample ID:	CL1142190	Job Number:	s11_9196
Multiplier:	8	Client:	Soil Mechanics
Dilution:	1	Site:	Sandsend Boreholes, North Yorkshire
Acquisition Method:	5UL_RUNF.M	Client Sample Ref:	BH3 ES 7A 2.00
Acquisition Date/Time:	11-Nov-11		
Datafile:	D:\TES\DATA\Y2011\NOVEMBER2011\1111TPH_GC3\019F2001.D		

Where individual results are flagged see report notes for status.




Detection limit of Method SCI-ASB-020 is 0.001%
Sampling has been carried out by client

ESG Environmental Chemistry
Etwall House, Bretby Business Park, Ashby Road, Burton upon Trent
Soil Mechanics
Sandsend Boreholes, North Yorkshire

Page 1 of 1	
Report No:	ANO-0488-2324
Report Date:	14/11/2011
Project Number:	S119196

[illegible]

Clients Request	NAIIS = No Asbestos Identified in Sample (Screens Only)	Name:	Andrew Elsby	Authorised Signatory: 
	NADIS = No Asbestos Detected in Sample (ID & Quant Only)	Position:	Regional Manager	

ures detailed in ESG Asbestos Limited in house method (SCI-ASB-020) based on HSE document MDHS 90 - Asbestos Contaminated Land - Draft 5 - November 1997 (withdrawn). Fibre d of transmitted/polarised light microscopy and centre stop dispersion staining (SCI-ASB-007), based on HSE's HSG 248. The analysis of fine fraction for asbestos content only includes fibres ess specified, to be amphiboles. All tests were carried out at ESG Asbestos Laboratory, Ashbourne House, Bretby Business Park, Ashby Road, Burton-upon-Trent, Staffordshire. DE15 0XD,

ntifics Group Limited (ESG), registered in England and Wales, registered company 04951688. Form ESG-SOIL-011 v04 Oct'11

S119196

Consignment No S24253
Date Logged 01-Nov-2011

	PCB-7 Congeners Analysis			
PCHRCOAM	PAH (16) by GCMS	✓		
PAMHSUS	SO ₄ --(H ₂ O sol) mg/l	✓		
ICPWS5	Magnesium.	✓		
	Iron	✓		
	Barium.	✓		
ICPSOIL	Aluminium	✓		
	Zinc (MS)	✓		
	Vanadium (MS)	✓		
	Selenium (MS)	✓		
	Nickel (MS)	✓		
	Mercury (MS)	✓		
	Manganese (MS)	✓		
	Lead (MS)	✓		
	Copper (MS)	✓		
	Cobalt (MS)	✓		
	Chromium (MS)	✓		
ICPMSS	Arsenic (MS)	✓		
ICPBOR	Boron (H ₂ O Soluble)	✓		
GROHSA	GRO	✓		
CustServ	Report B			
CEN LeachSW	CEN Leac(P) ₁			
BTEXHSA	MTBE (µg/kg)	✓		
	BTEX-HSA + MTBE analysis	✓		
MethodID	Sampled	D		
	Description			
ID Number				

	Analysis Required
	Analysis dependant upon trigger result - Note: due date may be affected if triggered
	No analysis scheduled
^	Analysis Subcontracted

EFS/119196 Ver. 1

Customer Soil Mechanics
 Site Sandsend Boreholes, North Yorkshire
 Report No S119196

Consignment No S24253
 Date Logged 01-Nov-2011

Report Due 14-Nov-2011

ID Number	Description	MethodID	Sampled	PHSOIL	SFAP1	Sub002a	TMSS	TPHFI005	WSLM59	Total Organic Carbon										
										TPH Carbon Banding.	TPH by GC/FID (AR)	Tot.Moisture @ 105C	^ Asbestos Screen	Cyanide(Total) (AR)	pH units (AR)					
Accredited to ISO17025										✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓
CL/1142187	BH1 0.10	D																		
CL/1142188	BH1 1.00	D																		
CL/1142189	BH3 0.30	D																		
CL/1142190	BH3 2.00	D																		

Note: For analysis where the Report Due date is greater than 7 days (PAH, Pesticides, PCB, Phenols, Herbicides) or 2 days (BOD) after the sampling date, although we will do our utmost to prioritise your samples, they may become deviant whilst being processed in the Laboratory.

In this instance, please contact the Laboratory immediately should you wish to discuss how you would like us to proceed. If you do not respond within 24 hours, we will proceed as originally requested.

Deviating Sample Key

- A The sample was received in an inappropriate container for this analysis
- B The sample was received without the correct preservation for this analysis
- C Headspace present in the sample container
- D The sampling date was not supplied so holding time may be compromised - applicable to all analysis
- E Sample processing did not commence within the appropriate holding time

Requested Analysis Key

- Analysis Required
- Analysis dependant upon trigger result - **Note: due date may be affected if triggered**
- No analysis scheduled
- ^ Analysis Subcontracted

Where individual results are flagged see report notes for status.

Method Descriptions

Matrix	MethodID	Analysis Basis	Method Description
Soil	BTEXHSA	As Received	Determination of Benzene, Toluene, Ethyl benzene and Xylenes (BTEX) by Headspace GCFID
Soil	GROHSA	As Received	Determination of Total Gasoline Range Organics Hydrocarbons (GRO) by Headspace GCFID
Soil	ICPBOR	Air Dried	Determination of Boron in soil samples by hot water extraction followed by ICPOES detection
Soil	ICPMSS	Air Dried	Determination of Metals in soil samples by aqua regia digestion followed by ICPMS
Soil	ICPSOIL	Air Dried	Determination of Metals in soil samples by aqua regia digestion followed by ICPOES detection
Soil	ICPWSS	Air Dried	Determination of Water Soluble Sulphate in soil samples by water extraction followed by ICPOES detection
Soil	PAHMSUS	As Received	Determination of Polycyclic Aromatic Hydrocarbons (PAH) by hexane/acetone extraction followed by GCMS detection
Soil	PCBUSECDAR	As Received	Determination of Polychlorinated Biphenyl (PCB) congeners/aroclor by hexane/acetone extraction followed by GCECD detection
Soil	PHSOIL	As Received	Determination of pH of 2.5:1 deionised water to soil extracts using pH probe.
Soil	SFAPI	As Received	Segmented flow analysis with colorimetric detection
Soil	SubCon*	*	Contact Laboratory for details of the methodology used by the sub-contractor.
Soil	TMSS	As Received	Determination of the Total Moisture content at 105°C by loss on oven drying gravimetric analysis
Soil	TPHFIDUS	As Received	Determination of hexane/acetone extractable Hydrocarbons in soil with GCFID detection.
Soil	WSLM59	Air Dried	Determination of Organic Carbon in soil using sulphurous Acid digestion followed by high temperature combustion and IR detection

Where individual results are flagged see report notes for status.

Report Notes

Generic Notes

Soil/Solid Analysis

Unless stated otherwise,

- Results expressed as mg/kg have been calculated on an air dried basis
- Sulphate analysis not conducted in accordance with BS1377
- Water Soluble Sulphate is on a 2:1 water:soil extract

Waters Analysis

Unless stated otherwise results are expressed as mg/l

Nil: Where "Nil" has been entered against Total Alkalinity or Total Acidity this indicates that a measurement was not required due to the inherent pH of the sample.

Oil analysis specific

Unless stated otherwise,

- Results are expressed as mg/kg
- SG is expressed as g/cm³@ 15°C

Gas (Tedlar bag) Analysis

Unless stated otherwise, results are expressed as ug/l

Asbestos Analysis

CH Denotes Chrysotile

CR Denotes Crocidolite

AM Denotes Amosite

NAIIS No Asbestos Identified in Sample

Symbol Reference

[^] Sub-contracted analysis. Note: The accreditation status is that assigned by the subcontract laboratory.

\$\$ Unable to analyse due to the nature of the sample

¶ Samples submitted for this analyte were not preserved on site in accordance with laboratory protocols.

This may have resulted in deterioration of the sample(s) during transit to the laboratory.

Consequently the reported data may not represent the concentration of the target analyte present in the sample at the time of sampling

¥ Results for guidance only due to possible interference

& Blank corrected result

I.S Insufficient sample to complete requested analysis

I.S(g) Insufficient sample to re-analyse, results for guidance only

Intf Unable to analyse due to interferences

N.D Not determined

N.Det Not detected

Req Analysis requested, see attached sheets for results

▮ Raised detection limit due to nature of the sample

* All accreditation has been removed by the laboratory for this result

‡ MCERTS accreditation has been removed for this result

Note: The Laboratory may only claim that data is accredited when all of the requirements of our Quality System have been met. Where these requirements have not been met the laboratory may elect to include the data in its final report and remove the accreditation from individual data items if it believes that the validity of the data has not been affected. If further details are required of the circumstances which have led to the removal of accreditation then please do not hesitate to contact the laboratory.

END OF REPORT

Where individual results are flagged see report notes for status.

TEST REPORT

CEN LEACHATE 2:1



Report No. EXR/127351 (Ver. 1)

Soil Mechanics
Askern Road
Carcroft
Doncaster
South Yorkshire
DN6 8DG

Site: Sandsend Boreholes, North Yorkshire

The 2 samples described in this report were registered for analysis by ESG on 01-Nov-2011. This report supersedes any versions previously issued by the laboratory.

The analysis was completed by: 16-Nov-2011

Tests where the accreditation is set to N or No, and any individual data items marked with a * are not UKAS accredited
Any opinions or interpretations expressed herein are outside the scope of any UKAS accreditation held by ESG.

The following tables are contained in this report:

Table 1 Main Analysis Results (Pages 2 to 4)
Table of PAH (MS-SIM) (10) Results (Page 5)
Table of PCB Congener Results (Page 6)
Table of TPH Texas banding (0.01) (Page 7)
GC-FID Chromatograms (Page 8)
Analytical and Deviating Sample Overview (Pages 9 to 10)
Table of Method Descriptions (Page 11)
Table of Report Notes (Page 12)

On behalf of
ESG :
Andrew Timms

A handwritten signature in black ink, appearing to read 'Andrew Timms', written over the printed name.

Operations Manager

Date of Issue: 16-Nov-2011

Tests marked 'A' have been subcontracted to another laboratory.

ESG accepts no responsibility for any sampling not carried out by our personnel.

[illegible]

[illegible]

[illegible]

Polycyclic Aromatic Hydrocarbons

GC/MS (SIM)

Customer and Site Details: Soil Mechanics: Sandsend Boreholes, North Yorkshire
Sample Details: BH1 1.00 **Job Number:** W12_7351
LIMS ID Number: EX1147751 **Date Booked in:** 01-Nov-11
QC Batch Number: 110983 **Date Extracted:** 15-Nov-11
Quantitation File: Initial Calibration **Date Analysed:** 15-Nov-11
Directory: 115PAH.MS17\ **Matrix:** Leachate
Dilution: 1.0 **Ext Method:** Bottle

UKAS accredited?: Yes

Target Compounds	CAS #	R.T. (min)	Concentration ug/l	% Fit
Naphthalene	91-20-3	3.36	0.012	94
Acenaphthylene	208-96-8	4.41	0.035	95
Acenaphthene	83-32-9	4.53	0.204	98
Fluorene	86-73-7	4.92	0.138	98
Phenanthrene	85-01-8	-	< 0.010	-
Anthracene	120-12-7	-	< 0.010	-
Fluoranthene	206-44-0	7.13	0.063	89
Pyrene	129-00-0	7.42	0.119	87
Benzo[a]anthracene	56-55-3	9.10	0.024	82
Chrysene	218-01-9	9.15	0.019	83
Benzo[b]fluoranthene	205-99-2	10.63	0.010	91
Benzo[k]fluoranthene	207-08-9	-	< 0.010	-
Benzo[a]pyrene	50-32-8	-	< 0.010	-
Indeno[1,2,3-cd]pyrene	193-39-5	-	< 0.010	-
Dibenzo[a,h]anthracene	53-70-3	-	< 0.010	-
Benzo[g,h,i]perylene	191-24-2	-	< 0.010	-
Total (USEPA16) PAHs	-	-	< 0.694	-

"M" denotes that % fit has been manually interpreted

Internal Standards	% Area
1,4-Dichlorobenzene-d4	NA
Naphthalene-d8	96
Acenaphthene-d10	95
Phenanthrene-d10	95
Chrysene-d12	99
Perylene-d12	97

Surrogates	% Rec
Nitrobenzene-d5	NA
2-Fluorobiphenyl	94
Terphenyl-d14	116

The Total PAH result is the sum of non-rounded individual PAH results and therefore may differ to the sum of the rounded individual PAH results printed above. By convention, where any one or more result is a "less than", the total is expressed as a "less than" and includes the "less than" concentration within the total.

Polychlorinated Biphenyls (congeners)

Customer and Site Details:	Soil Mechanics: Sandsend Boreholes, North Yorkshire
Job Number:	W12_7351
QC Batch Number:	110080
Directory:	1109PCB.GC8
Method:	Separating Funnel

Matrix: Leachate
Date Booked in: 01-Nov-11
Date Extracted: 09-Nov-11
Date Analysed: 10-Nov-11

*** This sample data is not UKAS accredited.**

[illegible]

Total Petroleum Hydrocarbons (TPH) Carbon Ranges

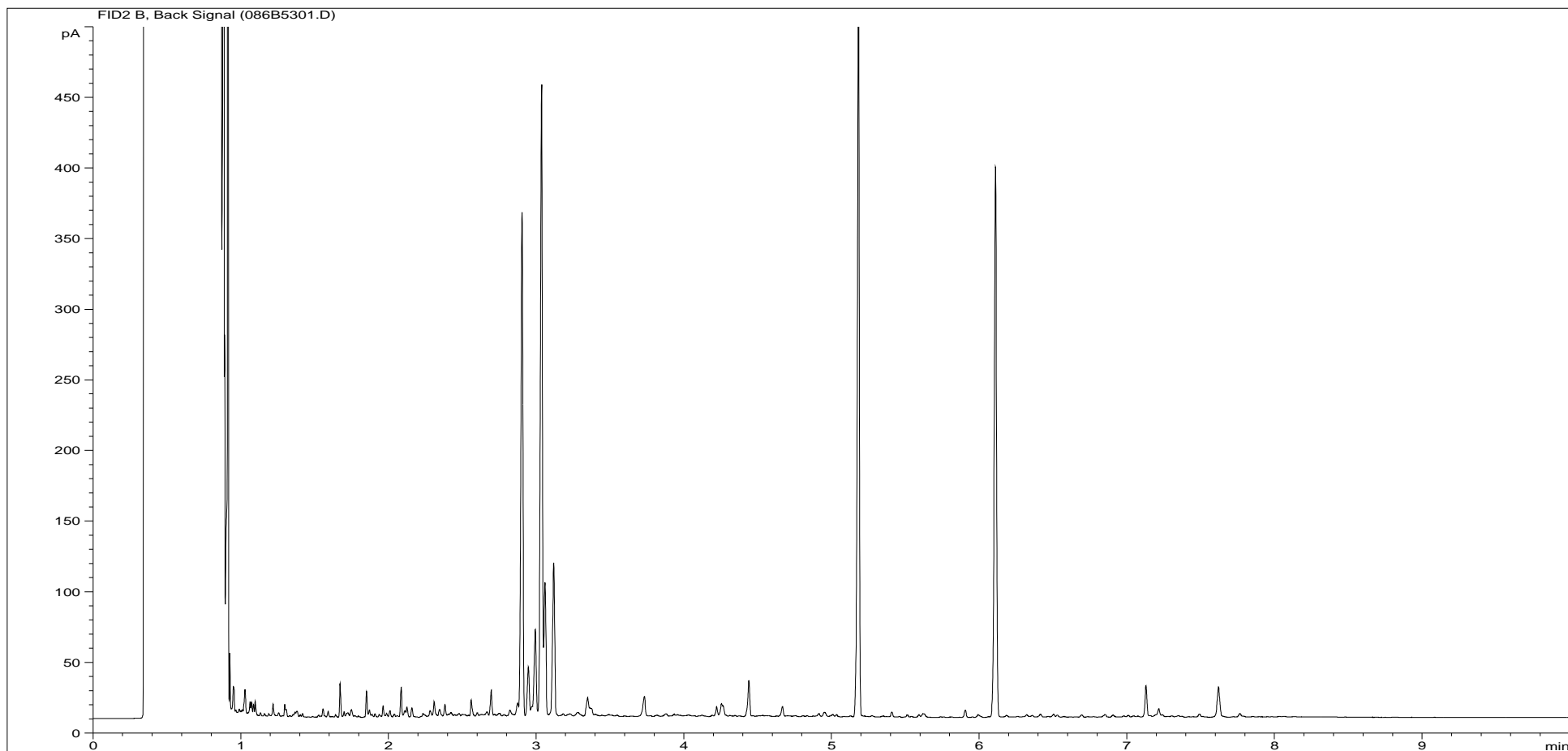
Customer and Site Details: Soil Mechanics : Sandsend Boreholes, North Yorkshire
Job Number: W12_7351
QC Batch Number: 110967
Directory: D:\TES\DATA\Y2011\110811TPH_GC16\110811 2011-11-08 09-00-42\086B5301.D
Method: Bottle

Matrix: Leachate
Date Booked in: 01-Nov-11
Date Extracted: 08-Nov-11
Date Analysed: 09-Nov-11, 01:50:04

*** Sample data with an asterisk are not UKAS accredited.**

[illegible]

Petroleum Hydrocarbons (C8 to C40) by GC/FID



Sample ID:	EX1147751	Job Number:	W12_7351
Multiplier:	0.005	Client:	Soil Mechanics
Dilution:	1	Site:	Sandsend Boreholes, North Yorkshire
Acquisition Method:	TPH_RUNF.M	Client Sample Ref:	BH1 1.00
Acquisition Date/Time:	09-Nov-11, 01:50:04		
Datafile:	D:\TES\DATA\Y2011\110811TPH_GC16\110811 2011-11-08 09-00-42\086B5301.D		

Where individual results are flagged see report notes for status.

Customer
Site
Report No

Soil Mechanics
Sandsend Boreholes, North Yorkshire
W127351

Consignment No S24253
Date Logged 01-Nov-2011

Report Due 14-Nov-2011

MethodID	Sampled	Description	ID Number	
BTEXHSA	BTEX-HSA + MTBE analysis			
CUSTSERV	Report B (CEN1)			
GROHSA	GRO-HSA			
ICPMSW	Nickel as Ni MS (Dissolved)			
	Chromium as Cr MS (Dissolved)			
	Cadmium as Cd MS (Dissolved)			
	Copper as Cu MS (Dissolved)			
	Lead as Pb MS (Dissolved)			
	Zinc as Zn MS (Dissolved)			
ICPMATVAR	Total Sulphur as SO4 (Diss) VAR	Accredited to ISO17025	EX/1147750	
	Cobalt as Co MS (Dissolved)			EX/1147751
	Vanadium as V MS (Dissolved)			
	Selenium as Se MS (Dissolved)			
	Mercury as Hg MS (Dissolved)			
	Arsenic as As MS (Dissolved)			
	Manganese as Mn MS (Dissolved)			
	Zinc as Zn MS (Dissolved)			
	Lead as Pb MS (Dissolved)			
	Copper as Cu MS (Dissolved)			
PAHMSW	PAH GC-MS (16)	Description	ID Number	
PCBONEC	PCB - 7 Congeners Analysis			
SFAPI	Cyanide (Total) as CN SFA			
LeachPrep	Leachate Prep			
	Boron as B (Dissolved) VAR			
	Aluminium as Al (Dissolved) VAR			
	Iron as Fe (Dissolved) VAR			
	Barium as Ba (Dissolved) VAR			
	Magnesium as Mg (Dissolved) VAR			
	Total Sulphur as SO4 (Diss) VAR			

Note: For analysis where the Report Due date is greater than 7 days (PAH, Pesticides, PCB, Phenols, Herbicides) or 2 days (BOD) after the sampling date, although we will do our utmost to prioritise your samples, they may become deviant whilst being processed in the Laboratory.

In this instance, please contact the Laboratory immediately should you wish to discuss how you would like us to proceed. If you do not respond within 24 hours, we will proceed as originally requested.

Deviating Sample Key	
A	The sample was received in an inappropriate container for this analysis
B	The sample was received without the correct preservation for this analysis
C	Headspace present in the sample container
D	The sampling date was not supplied so holding time may be compromised - applicable to all analysis
E	Sample processing did not commence within the appropriate holding time
Requested Analysis Key	
	Analysis Required
	Analysis dependant upon trigger result - Note: due date may be affected if triggered
	No analysis scheduled
^	Analysis Subcontracted

Customer Soil Mechanics
Site Sandsend Boreholes, North Yorkshire
Report No W127351

Consignment No S24253
Date Logged 01-Nov-2011

Report Due 14-Nov-2011

ID Number	Description	MethodID	TPHFID	WSLM13	WSLM3
		Sampled	TPH Carbon Banding	TPH GC (0.01)	Total Organic Carbon
Accredited to ISO17025		✓	✓	✓	✓
EX/1147750	BH1 0.10				
EX/1147751	BH1 1.00				

Note: For analysis where the Report Due date is greater than 7 days (PAH, Pesticides, PCB, Phenols, Herbicides) or 2 days (BOD) after the sampling date, although we will do our utmost to prioritise your samples, they may become deviant whilst being processed in the Laboratory.

In this instance, please contact the Laboratory immediately should you wish to discuss how you would like us to proceed. If you do not respond within 24 hours, we will proceed as originally requested.

Deviating Sample Key	
A	The sample was received in an inappropriate container for this analysis
B	The sample was received without the correct preservation for this analysis
C	Headspace present in the sample container
D	The sampling date was not supplied so holding time may be compromised - applicable to all analysis
E	Sample processing did not commence within the appropriate holding time
Requested Analysis Key	
	Analysis Required
	Analysis dependant upon trigger result - Note: due date may be affected if triggered
	No analysis scheduled
^	Analysis Subcontracted

Method Descriptions

Matrix	MethodID	Analysis Basis	Method Description
Water	BTEXHSA	As Received	Benzene, Toluene, Ethylbenzene, & Xylenes by headspace extraction GC/FID quantitation
Water	GROHSA	As Received	Determination of Total Gasoline Range Organics Hydrocarbons (GRO) by Headspace FID
Water	ICPMSW	As Received	Direct quantitative determination of Metals in water samples using ICPMS
Water	ICPWATVAR	As Received	Direct determination of Metals and Sulphate in water samples using ICPOES
Water	PAHMSW	As Received	Determination of PolyAromatic Hydrocarbons in water by pentane extraction GC/MS quantitation
Water	PCBCONEC	As Received	Determination of Polychlorinated Biphenyl (PCB) congeners by pentane extraction followed by GC/ECD detection
Water	SFAPI	As Received	Segmented flow analysis with colorimetric detection
Water	TPHFID	As Received	Determination of pentane extractable hydrocarbons in water by GC/FID
Water	WSLM13	As Received	Instrumental analysis using acid/persulphate digestion and dispersive IR detection
Water	WSLM3	As Received	Determination of the pH of water samples by pH probe

Where individual results are flagged see report notes for status.

Report Notes

Generic Notes

Soil/Solid Analysis

Unless stated otherwise,

- Results expressed as mg/kg have been calculated on an air dried basis
- Sulphate analysis not conducted in accordance with BS1377
- Water Soluble Sulphate is on a 2:1 water:soil extract

Waters Analysis

Unless stated otherwise results are expressed as mg/l

Nil: Where "Nil" has been entered against Total Alkalinity or Total Acidity this indicates that a measurement was not required due to the inherent pH of the sample.

Oil analysis specific

Unless stated otherwise,

- Results are expressed as mg/kg
- SG is expressed as g/cm³@ 15°C

Gas (Tedlar bag) Analysis

Unless stated otherwise, results are expressed as ug/l

Asbestos Analysis

CH Denotes Chrysotile

CR Denotes Crocidolite

AM Denotes Amosite

NAIIS No Asbestos Identified in Sample

Symbol Reference

^ Sub-contracted analysis. Note: The accreditation status is that assigned by the subcontract laboratory.

\$\$ Unable to analyse due to the nature of the sample

¶ Samples submitted for this analyte were not preserved on site in accordance with laboratory protocols.

This may have resulted in deterioration of the sample(s) during transit to the laboratory.

Consequently the reported data may not represent the concentration of the target analyte present in the sample at the time of sampling

¥ Results for guidance only due to possible interference

& Blank corrected result

I.S Insufficient sample to complete requested analysis

I.S(g) Insufficient sample to re-analyse, results for guidance only

Intf Unable to analyse due to interferences

N.D Not determined

N.Det Not detected

Req Analysis requested, see attached sheets for results

▮ Raised detection limit due to nature of the sample

* All accreditation has been removed by the laboratory for this result

‡ MCERTS accreditation has been removed for this result

Note: The Laboratory may only claim that data is accredited when all of the requirements of our Quality System have been met. Where these requirements have not been met the laboratory may elect to include the data in its final report and remove the accreditation from individual data items if it believes that the validity of the data has not been affected. If further details are required of the circumstances which have led to the removal of accreditation then please do not hesitate to contact the laboratory.

END OF REPORT

Where individual results are flagged see report notes for status.

ENCLOSURE E
PHOTOGRAPHS

Rotary Cores

Plates 1 to 8

ENCLOSURE F
DRAWINGS

Site Location Plan
Site Plan

F1
F2 and 3

Site Location Plan



Reproduced from the 1996 Ordnance Survey 1:50 000 scale Landranger map No 94 by permission of Ordnance Survey on behalf of The Controller of Her Majesty's Stationery Office, © Crown copyright, Environmental Scientifics Group Limited. All rights reserved. Licence Number 100006060

Notes: Scale 1:50 000	Project SANDSEND SLOPE STABILISATION, WHITBY, NORTH YORKSHIRE Project No. A1077-11 Carried out for Balfour Beatty Living Places	Figure F1
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PRELIMINARY / DRAFT

GENERAL NOTES

1.Reproduced from Ordnance Survey Data with the permission of The Controller of Her Majesty's Stationery Office, © Crown Copyright. Supplied by *****.

2.Reproduced from CLIENT NAME's Drawing No. ***** Rev No. ***.

3.Hole Locations to OSGB36 Co-ordinate Reference System.

4.All dimensions are in metres unless indicated otherwise.

5.All levels are in metres and related to OS Orthometric level datum.

Survey Control Co-ordinates			
Reference	East	North	Elevation
SANDS-REF	487211.203	512092.729	30.61

GI Investigation Co-ordinates (OSGB36 + OS Orthometric Hgt)			
Reference	East	North	Elevation
BH01	486765.856	512240.035	44.48
BH02	486709.848	512274.241	45.54
BH03	486650.134	512295.055	45.95
BH04	486819.851	512318.442	14.59
BH05	486705.875	512374.439	12.66
BH06	486759.825	512358.088	13.02
W501	487176.623	512177.532	4.43
W502	487089.805	512215.254	3.88
W503	486809.973	512351.996	3.98
W504	486649.562	512432.748	2.69
W505	486380.618	512549.270	3.72

LEGEND TO SYMBOLS

Borehole Location

Window Sample Location

Trial Pit Location

Dynamic Probe Location

Inspection Pit Location

Water Sample Location

TP**

TS

MC

AL

GT

LC

LC

Piezometer Response Zone

UUM=16,119,238

N=54

45/200

45/200

45 blows in test drive for 200mm penetration

Geology Codes:

TS Topsoil

MC Made Ground

AL Alluvium

GL Glacial Till

LC London Clay

LA Lambeth Group

Scale: 1:NNN

0

Xm

X

X

X

X

Rev	Drawn	Date	Apprv.	Date	Modification Details

AMENDMENTS

Title

Ground Investigation Survey

Project

Sandsend Whitby Nth Yorkshire

Client

CLIENT NAME OR LOGO

Soil Mechanics

Date	Drawn By	Apprv. By
12.10.11	HSL	DEF

Sheet Size	Scale	Project No
A1-1	1:1000	123456

Drawing No	Rev
123456/001	



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2.Reproduced from CLIENT NAME's Drawing No. ***** Rev No. ***.						
3.Hole Locations to OSGB36 Co-ordinate Reference System.						
4.All dimensions are in metres unless indicated otherwise.						
5.All levels are in metres and related to OS Orthometric level datum.						
Survey Control Co-ordinates						
Reference	East	North	Elevation	Description		
SANDS-REF	487211.203	512092.729	30.61	GPS Ref Stn		
GI Investigation Co-ordinates (OSGB36 + OS Orthometric Hts)						
Reference	East	North	Elevation	Description		
BH01	486763.854	512401.035	44.48	Borehole		
BH02	486769.848	512274.241	45.56	Borehole		
BH03	486820.134	512295.055	45.95	Borehole		
BH04	486819.851	513318.422	14.59	Borehole		
BH05	486765.875	512374.439	12.66	Borehole		
BH06	486739.825	512358.088	13.02	Borehole		
WS01	487176.223	512177.532	4.43	Window Sample		
WS02	487089.805	512215.234	3.86	Window Sample		
WS03	486889.973	512351.996	3.98	Window Sample		
WS04	486649.352	510453.748	2.89	Window Sample		
WS05	486380.618	512549.270	3.72	Window Sample		
LEGEND TO SYMBOLS						
<div><div><div>+</div><div>+</div><div>+</div><div>+</div><div>+</div><div>•</div></div><div>Borehole Location Window Sample Location Trial Pit Location Dynamic Probe Location Inspection Pit Location Water Sample Location</div></div>						
<div><div><div><div><div>TP**</div><div>TS</div><div>MG</div><div>AL</div><div>GT</div><div>LC</div></div><div><div>UUM=16,119,238</div><div>N=54</div><div>45/200</div></div></div><div><div>Piezometer Response Zone</div><div>Multistage Triaxial Test Results</div><div>SPT Results</div><div>Max Water Level in Piezometer/Standpipe</div><div>SPT Results (see notes)</div></div><div><div>SPT Results: 45/200</div><div>45 blows in test drive for 200mm penetration</div></div><div><div>Geology Codes:</div><div>TS Topsoil</div><div>MG Made Ground</div><div>AL Alluvium</div><div>GT Glacial Till</div><div>LC London Clay</div><div>LG Lambeth Group</div></div><div><div>Scale: 1:NNN</div><div><div>0</div><div>Xm</div><div>X</div><div>X</div><div>X</div><div>X</div></div></div></div></div>						
Rev	Drawn	Date	Approv.	Date	Modification Details	
AMENDMENTS						
Title						
Ground Investigation Survey						
Project						
Sandsend Whitby Nth Yorkshire						
Client						
CLIENT NAME OR LOGO						
Soil Mechanics						
Date		Drawn By		Approv. By		
12.10.11		HSL		DEF		
Sheet Size		Scale		Project No		
A1-2		1:1000		123456		
Drawing No				Rev		
123456/001						